CITY OF MANHATTAN BEACH COMMUNITY DEVELOPMENT DEPARTMENT

TO: Planning Commission

THROUGH: Richard Thompson, Director of Community Development

FROM: Angelica Ochoa, Assistant Planner

DATE: September 24, 2014

SUBJECT: Use Permit to Remodel an Existing Multi-tenant Commercial Building for

a New Pre-School (Daycare), Playground and Parking Lot at 1114-1126

22nd Street (Chalk School)

RECOMMENDATION

Staff recommends that the Planning Commission discuss and adopt attached Draft Resolution No. 14-11 (EXHIBIT A) approving the subject project with conditions.

PROJECT BACKGROUND

The applicant, Patrick Killen, project architect for Chalk School, applied for a Use Permit on June 6, 2014 to remodel and reuse the existing multi-tenant commercial building for a new day care use at 1114-1126 22nd Street. Specifically, the applicant is requesting construction of a new facility that will include 5 classrooms (6,371 square feet), a separate motor skills building (1,840 square feet) southwest of the site at the rear, a 7,363 square feet outdoor playground and 12 standard, 5 compact and 1 ADA accessible space for a total of 17 parking spaces. Per Section 10.16.020 of the Commercial Chapter of the City of Manhattan Beach Municipal Code, the subject application requires a use permit for day care use. Preschools are classified as day care in the Municipal Code Section 10.08.040 (D).

The Planning Commission, at its regular meeting of September 10, 2014, conducted a public hearing and discussed the subject project (Exhibit B). Some of the issues that were raised included maximum number of teachers per classroom, number of maximum students allowed per Department of Social Services, loading and unloading of students, sufficient parking for teachers and staff on-site, street parking on 22nd Street and Cedar Avenue, traffic impacts from the future expansion of the Manhattan Village Mall, and landscaping for buffering of noise impacts adjoining residential to the east.

The Planning Commission requested that staff return with a draft resolution addressing the issues discussed. The Planning Commission continued the matter to the meeting of September 24, 2014.

DISCUSSION

The Planning Commission heard public testimony and discussed the subject item at their regular meeting of September 24, 2014. Overall, the Planning Commission was in support of approving a new day care use at the subject site. The following issues were discussed at the meeting.

Parking and Number of Employees

The required number of spaces per Municipal Code Section 10.64.030 for day care use is 1 space per 7 children. Based on 118 students as proposed, the required number of parking spaces is 17 and 17 spaces are proposed. The Commissioners discussed whether the parking requirement for day care use included the number of employees. The zoning code parking requirement for day care use is based on the number of children but includes parking for all staff. According to the City Traffic Engineer, who attended the meeting, the preschool has staggered arrival and pick-up times and not all staff or all students will be present at the same time and for this reason it is expected that 17 parking spaces will be sufficient to handle the maximum number of students. The owner's representative confirmed that the staff works on staggered schedules and not all teachers will be on-site between the operating hours of 7am to 6pm.

Staff is limiting the maximum number of students to 118 based on the required parking per the Zoning Code of 17 spaces. Also, the Planning Commission felt that limiting the number of employees would restrict the operation of the business, and not allow flexibility on the number and ages of potential students or potentially sufficient teachers per classroom. Since the applicant is not certain of the age of students that will be licensed, by the Department of Social Services, the applicant is requesting to not restrict the number of employees. If the Department of Social Services (DSS) only permits 98 students based on the proposed outdoor play area of 7,363 square feet, then only 14 parking spaces would be required, and the proposed parking of 17 spaces would be met.

Traffic and Loading

Another issue that was discussed at the Planning Commission meeting, was loading and unloading of students and the traffic impact on the surrounding streets, especially to the residential uses to the east of the subject site on Cedar Avenue. The City Traffic Engineer emphasized that based on the results of the traffic study, there would be no significant traffic impacts from the proposed project. The net increase in vehicle trips is not significant because 22nd Street and Cedar Avenue currently operate at a level of service "A" with traffic volumes well below street capacity and the addition of a pre-school use would only add a small amount of vehicle traffic. The pickup and drop off of students would occur from only a two-way access driveway on Cedar Avenue and there would be staff present to ensure the quick flow of loading and unloading of students. In addition, the afternoon pick up of students would have minimal impact on congestion since 50% of the students attend only half day program until 12:30pm.

The Planning Commission discussed this issue and recommended to allow the opportunity and use of street parking on 22nd Street, west of Cedar Avenue for teachers and staff. This would make parking spaces available on-site for parents to use for the loading and unloading of students. The City Traffic Engineer agreed to remove the restriction of parking on-site for teachers and staff as required in the Traffic Operations and Management Plan (TOMP). Also, the Planning Commission suggested allowing the public parking spaces on the west side of

Cedar Avenue for loading and unloading of students to free up parking spaces for parents and appropriate signage will accommodate this through the TOMP.

Public Comments

At the public hearing, three neighbors expressed concerns regarding traffic, noise, loading and unloading of students, insufficient amount of parking on-site and traffic going into the surrounding residential neighborhood.

The Planning Commission felt that the City Traffic Engineer and the results from the traffic study addressed the traffic issues. Also, the project architect stated that other uses such as medical or retail would be more intensive and generate more traffic. In terms of noise, the project architect and school's representative stated that students would be let out to the playground at staggered times to reduce the amount of noise with the addition of landscaping for acoustical benefit. In addition, the issue of loading and unloading of students would be facilitated with school staff to ease congestion. The Planning Commission felt that allowing teachers and staff to park on 22^{nd} Street, west of Cedar Avenue and on the west side of Cedar Avenue would increase the amount of available parking on-site to alleviate cars entering and parking in the nearby residential neighborhoods.

Day Care (Chalk) Conditions

Since the applicant does not know the number of students the day care would be licensed per DSS requirements, staff felt the project should be approved for 118 students. Since 17 parking spaces complies with the required parking of 1 space for 7 children, 118 students would be the maximum allowed. The following conditions have been added to the Draft Resolution PC 14-11 (Exhibit A) as it pertains to the operation of the day care:

Resolution Conditions

- 1) Maximum number of students to be 118 per 17 parking spaces.
- 2) Hours of operation to be 7am to 6pm Monday through Friday.
- 3) Project to maintain a total of 17 parking spaces on-site.
- 4) Trash and recycling pick up to be conducted during non-peak times, not between 7-9am and 3-6pm.
- 5) Special activities or events will be limited to a maximum of 3 per year with prior City notification.
- 6) Submit annual Traffic Operations and Management Plan (TOMP) to be followed by faculty, staff, students and parents/guardians. The plan should include regular notification and reminders to all who frequent the site.

CONCLUSION

Overall, the Planning Commissions supported the project. They appreciated the modern and efficient design of the project and felt that it would fill an important need in the community.

They felt that the use was appropriate for the site and they could meet the findings to approve the Use Permit application. Staff recommends that the Planning Commission discuss and adopt the attached draft Resolution approving the subject Use Permit with conditions.

EXHIBITS:

- A. Draft Resolution PC 14-11
- B. PC Draft Minutes, staff report and related attachments from PC Meeting of 9/10/14

cc: Patrick Killen, Project Architect

'Draft' RESOLUTION PC NO. 14-11

RESOLUTION OF THE PLANNING COMMISSION OF THE CITY OF MANHATTAN BEACH APPROVING A USE PERMIT TO ALLOW A NEW DAY CARE AT 1114-1116 22nd STREET (CHALK PRE-SCHOOL)

THE PLANNING COMMISSION OF THE CITY OF MANHATTAN BEACH DOES HEREBY RESOLVE AS FOLLOWS:

<u>SECTION 1.</u> The Planning Commission of the City of Manhattan Beach hereby makes the following findings:

- A. The Planning Commission of the City of Manhattan Beach conducted a public hearing pursuant to applicable law on September 10, 2014, and the matter was continued to September 24, 2014 to consider an application for a Use Permit for the property legally described as 1114-1116 22nd Street, Portion of Lots 6 and 7, Section 19, Township 3 South Range 14 West in the City of Manhattan Beach.
- B. Said public hearing was advertised pursuant to applicable law, testimony was invited and received.
- C. The applicant is Patrick Killen, project architect for the subject site representing CHALK Pre-School.
- D. The applicant requests approval to allow a new day care pre-school use at 1114-1116 22nd Street. Specifically, the project proposal is to remodel and reuse 6,371 square feet of an existing one-story building and a 1,840 square foot separate building at the rear of the site, a 7,363 square feet outdoor playground, and a parking lot of 17 spaces on a 22,455 square feet lot. The proposed hours of operation will be 7am to 6pm Monday through Friday.
- E. The Planning Commission finds that the project will not individually or cumulatively have an adverse effect on wildlife resources, as defined in Section 711.2 of the Fish and Game Code.
- F. The subject project as proposed was found to be in accordance per CEQA (California Environmental Quality Act) Guidelines and is exempt as an in-Fill Development Project per Class 32, Section 15332.
- G. The site is located in Area District II and is zoned CG, (General Commercial). The subject site is currently developed with a one-story multi-tenant commercial building. There is an existing one-story building located at the southwest of the site in the existing rear parking lot that will be developed as day care use also. The surrounding area is developed with a mix of commercial, offices, bank, retail, restaurant, and residential uses to the north, east, west and south of the site. There are commercial uses to the North (across 22nd Street), Walgreens Pharmacy, further north is the Manhattan Village Mall. Single family residential uses are developed to the east (across Cedar Avenue) and northeast (across 22nd Street), as well as to the southeast. Commercial uses are developed directly to the west, UCLA Radiology Center and further west across Sepulveda Boulevard there are restaurants, offices, a jewelry store, car rental agency, hair salon and a variety of retail uses, to the south, there is Manhattan Bread and Bagel Center, Rubios, Citizens Bank and other offices.

EXHIBIT A PC MTG 9-24-14

- H. Pursuant to MBMC 10.84.60.A, findings are hereby made:
 - a. The proposed location of the use is near a concentration of a mix of local commercial serving businesses along Manhattan Beach Boulevard, is in accord with the objectives of the General Commercial Zoning District which seeks to provide sites for businesses serving the daily needs of nearby residential areas, incorporating standards that prevent significant adverse impacts on adjoining residential uses.
 - b. The project site is classified General Commercial in the General Plan, which is intended for a wide range of businesses/uses. The project is a new day care school and is consistent with the goals and policies of the Land Use and Infrastructure Element of the General Plan, including the following:
 - LU-2.1: Develop landscaping standards for commercial areas that unify and humanize each district.
 - LU-2.4: Support appropriate stormwater pollution mitigation measures.
 - LU-3.1: Continue to encourage quality design in all new construction.
 - LU-5.2: Work with all commercial property owners bordering residential areas to mitigate impacts and use appropriate landscaping and buffering of residential neighborhoods.
 - LU-6.2: Encourage a diverse mix of businesses that support the local tax base, are beneficial to residents, and support the economic needs of the community.
 - I-4.2: Carefully review commercial development proposals with regard to planned ingress/egress, and enforce restrictions as approved.
 - I-4.3: Encourage provision of on-site parking for employees.
 - I-4.4: Ensure that required parking and loading spaces are available and maintained for parking.
 - I-5: Reduce the adverse parking and traffic impacts that schools create on surrounding residential neighborhoods.
 - I-5.2: Work with the school district and private schools to improve pedestrian and bicycle safety around schools.
 - I-5.5: Work with the school district and private schools to address high traffic volumes during the morning and afternoon peak school hours, and improve drop-off and pick-up circulation.
 - c. The proposed use will comply with all applicable provisions of the Manhattan Beach Zoning Ordinance, including noticing and public hearing requirements.
 - d. The proposed use will not adversely impact nor be adversely impacted by nearby properties or create demands exceeding the capacity of public services and facilities that cannot be mitigated. Potential impacts may include but not necessarily be limited to: traffic, parking, and noise. The addition of a day care pre-school at 1114-1116 22nd Street will not exacerbate parking problems in the immediate area

because a new on-site parking lot of seventeen vehicles including loading and unloading spaces will be provided with the project. There is existing public parking on 22nd Street, on both sides of the street in front of the subject site, west of Cedar Avenue and also on the west side of Cedar Avenue, at the rear of the site, which will also be available for loading and unloading during peak hours. Also, the new day care pre-school will have minimal impact on traffic since the volume of traffic for the pre-school is a minimal percentage of the total traffic volume on 22nd Street and Cedar Avenue. The project will have minimal impact on noise since the building was designed to lessen any impacts on the surrounding residences, such as the type of material used on the outdoor playground for acoustical benefit and classrooms at the rear with walls and landscape buffer along the perimeter of the property.

<u>SECTION 2.</u> The Planning Commission of the City of Manhattan Beach hereby **APPROVES** the subject use permit subject to the following conditions:

Construction/Implementation

- 1. The project shall be built in substantial conformance with the plans and project description as submitted to and approved by the Planning Commission on September 16, 2014. Any substantial deviation from the approved plans and project description requires review and approval from the Director of Community Development and the determination if Planning Commission review and approval is required.
- 2. Landscaping shall be provided consistent with code requirements, including drought tolerant mature landscaping with box sized trees to buffer adjacent uses and shade the parking lot. Specifically, along the south, east and west property line landscaping shall be provided to provide a sound buffer to the adjacent residential properties and shade trees throughout the parking lot. A low water use irrigation system shall be installed and maintained in all new planting areas. Plans must be submitted for review and approval by the Community Development Department.
- 3. The applicant shall provide an on-site trash, recycling and mop sink area, subject to the approval from Public Works, Community Development, and Waste Management. The designated area shall be screened from public view and constructed within the building structure. Trash and recycling pick up to be conducted during non-peak times, not between 7-9:30am and 3-5:30pm and shall be accessible from 22nd Street.

Operational Conditions

- 4. The maximum number of students shall be 118. The intent of this provision is to minimize parking and traffic impacts resulting from the day care operation.
- 5. Special activities or events shall be limited to a maximum of 3 per year outside of regular operating hours, providing adequate off-site parking is provided in adjacent parking lots.
- 6. The applicant shall maintain 17 parking spaces based on the code required, 1 space per 7 children, with a maximum of 118 students.

Traffic Conditions

- 7. The applicant shall prepare and maintain a Traffic Operations and Management Plan (TOMP) as summarized in the Traffic Study to be followed by faculty, staff, students and parents/guardian. The TOMP shall be submitted to the Community Development Department at the beginning of each school enrollment period or more often at the City's discretion. The reports shall include information on parking operations, site access and circulation, and pre-school student drop-off/pick-up operation. The plan shall provide for regular notification and reminders to all who frequent the site of the parking, loading/unloading and other Use Permit requirements.
- 8. No bus, van or other school vehicle shall be stored on-site unless approved by the Community Development Director.
- 9. All parking areas shall be unreserved, open and available for employees and visitors during school hours. The public parking spaces on the west side of Cedar Avenue maybe used for loading and unloading of students and the parking spaces on both sides of 22nd Street, west of Cedar Avenue maybe used by teachers and staff. School-related vehicles should not park, drop-off or pick up students along 22nd Street or Cedar Avenue unless parking lot is full.
- 10. The project, including the parking lot shall comply with Standard Urban Stormwater Mitigation Plan as required by Building and Safety.

Procedural

- 11. In accordance with Section 10.84.090 of the Manhattan Beach Municipal Code, the subject use permit shall expire two years after the date of approval, unless implemented, or extended.
- 12. All provisions of the use permit are subject to review by the Community Development Department 6 months after occupancy and annually thereafter.
- 13. At any time in the future the Planning Commission or City Council may review the use permit or the purposes of revocation or modification. Modification may consist of conditions deemed reasonable to mitigate or alleviate impacts to adjacent land uses.
- 14. Pursuant to Public Resources Code section 21089 (b) and Fish and Game Code Section 711.4 (c) as applicable, the project is not operative, vested or final until required filing fees are paid.
- 15. The applicant must submit in writing to the City of Manhattan Beach acceptance of all conditions within 30 days of approval of Use Permit.
- 16. The applicant agrees as a condition of approval of this project to pay all reasonable legal and expert fees and expenses of the City of Manhattan Beach in defending any legal action brought against the City within 90 days after the city's final approval, other than one by the Applicant, challenging the approval of the project or any action or failure to act by the City relating to the environmental review process pursuant to the California Environmental Quality Act. In the event such a legal action is filed against the City, the City shall estimate it is expenses for the litigation. The Applicant shall

deposit said amount with the City or enter into an agreement with the City to pay such expenses as they become due.

SECTION 3. Pursuant to Government Code Section 65009 and Code of Civil Procedure Section 1094.6, any action or proceeding to attack, review, set aside, void or annul this decision, or concerning any of the proceedings, acts, or determinations taken, done or made prior to such decision or to determine the reasonableness, legality or validity of any condition attached to this decision shall not be maintained by any person unless the action or proceeding is commenced within 90 days of the date of this resolution and the City Council is served within 120 days of the date of this resolution. The City Clerk shall send a certified copy of this resolution to the applicant, and if any, the appellant at the address of said person set forth in the record of the proceedings and such mailing shall constitute the notice required by Code of Civil Procedure Section 1094.6.

> I hereby certify that the foregoing is a full, true, and correct copy of the Resolution as adopted by the Planning Commission at its regular meeting of September 24, 2014 and that said Resolution was adopted by the following vote:

AYES: NOES: ABSTAIN: ABSENT:

RICHARD THOMPSON

Planning Commission Secretary

ROSEMARY LACKOW

Recording Secretary

CITY OF MANHATTAN BEACH [DRAFT] PLANNING COMMISION MINUTES OF REGULAR MEETING SEPTEMBER 10, 2014

A Regular Meeting of the Planning Commission of the City of Manhattan Beach, California, was held on the 10th day of September, 2014, at the hour of 6:30 p.m., in the City Council Chambers, at 1400 Highland Avenue, in said City.

1. ROLL CALL

Present: Andreani, Bordokas, Conaway, Hersman, Chairperson Ortmann

Absent: None

Staff Present: Richard Thompson, Community Development Director

Angelica Ochoa, Associate Planner Erik Zandvliet, Traffic Engineer Rosemary Lackow, Recording Secretary

2. **AUDIENCE PARTICIPATION - None**

3. APPROVAL OF MINUTES – July 9, 2014

A motion was MADE and SECONDED (Hersman/Bordokas) to **APPROVE** the minutes of July 9, 2014, as submitted.

AYES: Conaway, Hersman Chairperson Ortmann

NOES: None ABSENT: None

ABSTAIN: Andreani, Bordokas

4. PUBLIC HEARING

09/10/14-2. Consideration of a Use Permit to Remodel an Existing Multi-Tenant

Commercial Building for a New Pre-School (Daycare), Playground and Parking

Lot at 1114-1126 22nd Street (Chalk Preschool)

Director Thompson made introductory remarks and introduced Associate Planner Ochoa who gave an oral report using a slide presentation. Ms. Ochoa summarized the staff recommendation that the Commission conduct the public hearing, accept public testimony, discuss and provide direction to staff on the subject proposal. Her slide presentation covered the site plan and map including surrounding streets and uses, the details of the project site and proposal, existing conditions including current/past commercial uses, driveways, site photos, public and City department comments, recommended conditions if approved, and findings that need to be made for a Use Permit.

Chairperson Ortmann invited the Commissioners to direct questions to staff.

In response to questions from Commissioner Andreani, Associate Planner Ochoa noted that: California State DSS (Department of Social Services) establishes the number of students that can attend based on the amount of outdoor play area provided (1 student per 75 square feet outdoor play area) and based on this, attendance is capped at 98 children. Ms. Ochoa advised Commissioner Andreani that questions regarding the preschool operations (e.g. food preparation and expected number of part time vs. full time students expected) would best be addressed by the applicant and questions regarding the traffic and parking lot circulation should be directed to the City's Traffic Engineer.

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EXHIBIT B PC MTG 9-24-14 In response to an inquiry from Commissioner Conaway, Ms. Ochoa indicated that she did not know the disposition of the businesses that are being replaced by the school except that she had notice recently that the businesses have vacated the site.

Chairperson Ortmann invited the Traffic Engineer to make a presentation to the Planning Commission.

Erik Zandvliet, Traffic Engineer for the City gave the Commission an overview of the project and summary of the findings of the Traffic Impact Study prepared for the project which includes several operational recommendations to be contained in a TOMP (Traffic Operations and Management Plan), and this plan would be incorporated into the conditions of approval.

Mr. Zandvliet noted that he agreed with the report's overall conclusion that there would be no significant traffic impacts. **Mr. Zandvliet** also explained how drop-off and pick-up would occur, with access solely from a two-way driveway on Cedar and the use of a staff monitor(s) to ensure flow in and out of parking spaces and on and off the site, with no loading or unloading recommended off-site on adjacent streets. **Mr. Zandvliet** emphasized that a preschool unlike elementary schools has staggered arrival and pick-up times is why it is expected that the area on site including the amount of parking spaces, will be sufficient to accommodate 98 students. **Mr. Zandvliet** stated that he does not expect there to be undue amount of cut-through traffic in the adjoining residential neighborhood to the east nor does he think it is necessary to impose parking restrictions on the adjoining streets. Should unanticipated problems arise, additional measures can be implemented administratively.

In response to Commissioner Bordokas **Mr. Zandvliet** noted that while it is not typical for a Use Permit to restrict commercial parking in front of a business for its customers, some restrictions or conditions on adjoining street use have been imposed in use permits for some schools. There was a brief discussion as to a condition proposed by staff that the school be prohibited from teacher parking and student unloading on the 22nd Street frontage adjoining the site. Development Director Thompson explained that location is very important and he believes the proposed condition to limit parking for loading/unloading students only on private property was borrowed from a prior application by Chalk for a school proposed on Manhattan Beach Boulevard which is a much busier street. **Mr. Zandvliet** summarized that this type of condition is at the discretion of the Commission, and from a traffic engineering perspective it is possible to allow use of 22nd Street west of Cedar Avenue for parking.

In response to a question from Commissioner Andreani, **Mr. Zandvliet** stated that he believes that 17 spaces should be sufficient even with 10 staff on the premises because not all staff members, nor all 98 students are expected to be present simultaneously during the peak use times. **Mr. Zandvliet** clarified that the zoning code for preschool parking takes into account the staff parking needs. He also stated that the net increase in vehicle trips is not considered to be significant because the streets adjoining the school site (Cedar Avenue and 22nd Street) currently operate at a level of service "A", far below their capacity and the number of vehicles amount being added is relatively small.

In response to a question from Commissioner Andreani regarding possible impact on Cedar after Manhattan Village is developed, **Mr. Zandvliet** acknowledged there could be some impact, depending on whether traffic exiting the mall would be allowed to cross Marine.

In response to a question from Commissioner Hersman regarding loss of revenue from the existing businesses being vacated, Director Thompson noted that the loss of City revenues was not studied in this case, but it is not expected to be significant due to the relatively small size of the existing businesses that would be replaced by the school.

In response to a question from Commissioner Conway, **Mr. Zandvliet** indicated that although the intersection of 18th Street and Cedar Avenue was not analyzed in the traffic study, it is his professional opinion it will not be significantly impacted and will remain at its current high level of performance because it is well under its capacity.

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In response to a question from Chairperson Ortmann **Mr. Zandvliet** explained that the traffic study's conclusion that there would be no net additional adverse traffic impact is based on the assumption that all of the commercial spaces on the site would be occupied.

There being no other questions from the Commissioners, Chairperson Ortmann invited the applicant to address the Commission.

Pat Killen, project architect explained the background and planning process for the proposed school, including the site was being considered for medical office uses which would require 60 parking spaces. **Mr. Killen** summarized the site layout including the play area, access and parking, and proposed improvements to existing structures (main classroom building and motor skills development space). **Mr. Killen** stated that the applicant hosted a neighborhood meeting on August 28th. He noted that Chalk has substantial experience as they operate five other similar schools. Two local schools (Westwood/Venice) average a 50/50 breakdown in the amount of students that will be part time and full time. **Mr. Killen** also explained that there will be a small kitchen but it will not be used for preparing but rather the distribution of lunches brought from home. He concluded by stating that, by comparison with the Manhattan Beach Boulevard location for the previous Chalk proposal, this is a much better site.

In response to an inquiry from Commissioner Andreani, **Mr. Killen** summarized the neighborhood meeting: 6 residents attended and he felt that their concerns regarding noise and traffic were addressed. He felt that it helped for the neighbors to understand the proposed operations, including how the parents and staff would be involved, with staff closely monitoring and supervising the arrival and departure of students during peak times. Regarding noise **Mr. Killen** had explained to the neighbors that only one class at a time will be using the outdoor area. **Mr. Killen** responded to Commissioner Andreani that "special events" while possible are not programmed for the school, that the playground surface (but not the parking lot) will be permeable and there will be a low-impact drainage system (already required through the city's "Green Building Code") and drip irrigation.

In response to Commissioner Conway, **Mr. Killen** explained the location of the school office on the east side of the site. He emphasized that the play areas will be located adjacent to all of the classrooms. Access will be from the office, then to classrooms and play areas. Children will be kept away from the streets. The frontage along 22nd Street will not have an entrance but will be greatly enhanced aesthetically so as to engage the building façade with the street, and not appear as a back of building. Commissioner Conaway encouraged **Mr. Killen** to investigate using more landscaping especially on the east side for acoustical benefit.

Mr. Killen clarified to Commissioner Hersman that the play areas will be completely fenced and will not be accessible to the parking lot.

Angela Johnson, representing Chalk as the proposed school's Director, responded to questions. **Ms. Johnson** explained the operation in more detail, and that the number of teachers will be dependent on the number of children present in each classroom, but that their goal is a 10:1 ratio of children to teacher. **Ms. Johnson** explained that the school uses a key fob program so that they know in the case of an emergency, where each child is and there will always be someone on a full-time basis to staff.

In response to a question from Commissioner Bordokas, **Ms. Johnson** explained that the process whereby the state DSS certifies the school in advance to opening its doors.

In response to a question from Commissioner Andreani, **Ms. Johnson** stated that both Chalk schools in Venice and Westwood have close to a 50/50 ratio of part vs. full-time students and this was the basis for their proposal for Manhattan Beach. Regarding the protocol for children to bring their own food, **Ms. Johnson** noted that the school provides snacks but they have found that parents prefer to provide food because it is common for children to have a special diet and allergies.

Ms. Johnson confirmed to Commissioner Hersman that Chalk does not require potty training as a condition of admittance and that 10 children in a classroom, depending on the age, may only need one teacher. With 2-year olds the ratio of children to teacher is generally less than older children.

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AUDIENCE PARTICIPATION

Chairperson Ortmann invited the public to address the Planning Commission.

John Dumond, 1135 22nd Street for 17 years, is concerned mainly about on-site parking and possible impacts to traffic due to congestion in the on-site parking lot for the school. He noted that there is no parking on the north side of 22nd Street and on the east side of Cedar. Specific concerns are that 17 parking spaces provided on site will be needed mainly for the staff which he estimates will be 13 (10 teachers, 2 per classroom plus 3 non-teacher staff) which will leave only about 5 parking spaces on and off the site that will be available for the parents to park and drop off or pick up their children. He is concerned that the process will involve up to 50 people at a time needing to drop off children and with spaces already occupied by staff this could result in congestion with cars backing up into the street. **Mr. Dumond** also noted concern that there will be special events as noted on their website that mentions a carnival to be held soon at their Venice location.

Chairperson Ortmann asked **Mr. Dumond** whether he thought this type of use could, as a tradeoff for the neighborhood, serve as a good transition between the commercial uses on Sepulveda and the residences to the east, as opposed to continuing to have uses similar to the five businesses that have existed on the site. **Mr. Dumond** responded that the pre-existing uses are now gone, and at this time he feels a medical use is a more viable replacement use. He would prefer a medical office use because he doesn't think it would cause parking congestion like the school. In addition **Mr. Dumond** stated that he believes that noise from the outdoor play areas will be for at least 8 hours a day because each classroom will be using the play area throughout the day and he confirmed to Chairperson Ortmann that the Schooner's bar was an existing use when he bought his property.

Claudia Elliot, resident on 22^{nd} Street, east of Cedar requested and received clarification that the students are proposed to be dropped off in the parking lot and not in 22^{nd} Street. She is concerned that with parking already limited on 22^{nd} Street that if there is additional parking in her block coming from the school, she will lose a parking space typically available in front of her house that she needs for her disabled husband. She is also concerned that 21^{st} Street would be used as a loop to get back to Cedar. Chairperson Ortmann responded that staff does not believe that such traffic intrusion will occur, however staff will be monitoring the school and adjustments, if needed would be made.

Monica Griffin, 20th Street east of Cedar expressed concern regarding parking and traffic and in particular that Cedar would be used as a bypass to avoid the intersection at Marine and Sepulveda. Because the drop off could be occurring in the peak "rush" hours, she asked that the City carefully consider the traffic study. She asked if other traffic studies have been done for other preschools in the City.

Director Thompson noted that the City has a lot of experience with preschools including the drop off and pick up process, and noted it is common for adjustments to be made for operations. However, staff doesn't expect there to be a need for all of the teachers to be there at the same time and believes this site is a good location, and staff is comfortable in assuming that at no time will all 98 parents show up at the same time at the school.

There being no other speakers: Chairperson Ortmann closed the public hearing and invited the applicant to address the Commission and respond to public comments.

Pat Killen addressed street parking and noted that two additional spaces on adjoining streets (one on 22nd and one on Cedar) will be available for public parking. Mr. Killen stated his opinion that regarding potential optional commercial uses for the site, a medical office use would be more intensive (requiring 60 parking spaces for this site) and he believes a retail use would generate considerably more traffic. Therefore he believes that the proposal by comparison to other commercial uses, would be a good transitional use. Furthermore, the City of Manhattan Beach has very high development standards for preschool parking compared to other "high end" cities. Mr. Killen confirmed that Chalk will be doing a ground lease and concluded by stating that he believes that Manhattan Beach is lucky to get this new preschool development because cost of land and land requirements for schools are usually disincentives.

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Angela Johnson clarified that the staff works on staggered schedules and not all the teachers will be onsite the entire school day, from 7 am to 6 p.m. **Ms. Johnson** also stated that not all teachers drive to the school and some parents bring their child in a stroller, not a car. For example, at 7 am there will likely be only 1 or 2 classrooms in use with 2 or 3 staff members. Regarding play area noise, Ms. Johnson explained that the children will be in the play area only 2.5 hours in the am and 2.5 hours in the pm.

COMMISSION DISCUSSION

There being no more persons wishing to speak, Chair Ortmann closed the public hearing and invited the Planning Commission to discuss the project.

Commissioner Bordokas stated that she is excited about the proposal, noting there is a real shortage for preschools in Manhattan Beach. Many children have to be taken to schools outside of the City.

Commissioner Conaway stated his support for the project and believes that this location is superior to the earlier Chalk application on Manhattan Beach Boulevard and believes the preschool is sorely needed in the community. He doesn't think the relatively small incremental traffic being generated will create problems although he is sympathetic with neighbors who are concerned. He agrees with Commissioner Bordokas that the proposed condition would limit the school's use of the adjacent streets is too restrictive given that he believes there is opportunity for parking along 22nd Street, and suggests that condition be struck. Commissioner Conaway does believe that special events are a concern and suggested that the TOMP specifically address potential impacts such as parking by event attendees are to be handled. Regarding play area noise, he encourages the school to consider ways to increase the acoustic design of the site to prevent sound from bouncing off the lot and traveling east.

Commissioner Hersman stated she is in agreement with Commissioners Conway and Bordokas. She believes that the south side of 22^{nd} Street west of Cedar is an appropriate place for the teachers to park and will be beneficial for the neighbors because it will free up space on the site for the parents to use. She questioned having a limit on the amount of staff allowed. Director Thompson stated that staff would reexamine whether the proposed restrictions on both street parking and a cap on staff should be imposed on the project. Commissioner Hersman also endorsed the idea of providing more landscaping to buffer sound.

Commissioner Andreani noted that she is sorry to see long term local business go away, but agrees that the Chalk school is much needed and will be a good transitional use for the site, and is more desirable than a retail or medical office. Commissioner Andreani has concerns about the number of students which she would like to see limited to 98. She would like to see staff look into allowing one or two parking spaces on adjoining streets for drop off and pick up. Commissioner Andreani's other concerns include how Cedar Way may be impacted by improvements to be done at Manhattan Village Shopping Center (i.e. should plan for that) and that special events be appropriately limited. She applauds the overall design, use of green building codes landscaping to address noise.

Chairperson Ortmann stated he agrees with comments made by his fellow commissioners and added that he believes that this is a reasonable and appropriate transitional use for the neighborhood. He shares concerns about traffic and parking but is also encouraged by the confidence that staff has that the traffic and parking will not be a problem.

Development Director Thompson indicated staff has noted the concerns and direction of the Commission and will return with a draft resolution of approval for consideration and adoption.

Commissioner Andreani stated her appreciation for the neighbors including Mr. Dumond in giving their input and encouraged the neighbors to also attend the next meeting when a draft Resolution will be presented to the Commission for action.

Director Thompson indicated that this matter will be returned to the Commission on September 24th.

[Draft] Planning Commission Meeting Minutes of September 10, 2014 Page 5 of 6

6. DIRECTOR'S ITEMS

Development Director Thompson provided the following status reports:

- 900 Club: The City Council conducted an appeal hearing and took action, directing that staff prepare a resolution that supports the PC decision with a couple minor changes.
- Shade Hotel: The City Council conducted a hearing and there were a few changes in the conditions, including a requirement for an automatic public hearing before the City Council (not Planning Commission) within one year.
- Manhattan Village Shopping Center: The City Council has supported the project subject to some additional requirements. The applicant has not yet responded to those conditions. Good news is that Apple will be opening a new store in the mall this weekend.

7. PLANNING COMMISSION ITEMS

Commissioner Conaway asked and Director Thompson responded that the Plache Minor Exception appeal was approved by Council.

Chairperson Ortmann: 1) Status of Mobility Plan: Director Thompson reported it is scheduled to be reviewed by the City Council at a study session on September 23rd. After that, it will go to the Planning Commission. 2) Downtown RFP Status: Director Thompson reported that an RFP has been released for a Downtown Specific Plan, and meanwhile there is a moratorium that prohibits any change in use downtown.

Commissioner Hersman: Status of Pier fishing: Director Thompson summarized that after the City Council passed a moratorium on fishing, the City was informed that other state agencies had jurisdiction on fishing, and subsequently the City has been working with those agencies. Fishing has been restored on the pier. The City is monitoring the situation.

8. TENTATIVE AGENDA – September 24, 2014

Director Thompson indicated that there are no items scheduled so far with the exception of the adoption of a resolution for the subject preschool.

9. ADJOURNMENT

The meeting was adjourned at 8:30 pm to Wednesday, September 24, 2014, in the City Council Chambers, City Hall, 1400 Highland Avenue.

ROSEMARY LACKOW
Recording Secretary

RICHARD THOMPSON

ATTEST:

Community Development Director

CITY OF MANHATTAN BEACH DEPARTMENT OF COMMUNITY DEVELOPMENT

TO:

Planning Commission

FROM:

Richard Thompson, Director of Community Development

BY:

Angelica Ochoa, Associate Planner

DATE:

August 28, 2014

SUBJECT:

Consideration of a Use Permit to Remodel an Existing Multi-tenant

Commercial Building for a New Pre-School (Daycare), Playground and

Parking Lot at 1114-1126 22nd Street (Chalk Preschool)

RECOMMENDATION

Staff recommends that the Planning Commission CONDUCT the PUBLIC HEARING, DISCUSS and PROVIDE DIRECTION to staff.

APPLICANT

Patrick Killen, Studio9one2 Architecture, c/o CHALK Preschool 930 Manhattan Beach Boulevard Manhattan Beach, CA 90266

PROJECT OVERVIEW

LOCATION

Location

1114-1126 22nd Street (Exhibit A)

Legal Description

Portion of Lots 6 and 7, Section 19, Township 3 South

Range 14 West

Area District

II

LAND USE

General Plan

General Commercial

Zoning (Exhibit B)

CG, General Commercial

Land Use: Existing Proposed

> Multi-tenant Commercial Pre-school (Day Care) Building Playground/Parking Lot

Neighboring Land Uses/Zoning

Commercial to the North (across 22nd Street), Walgreens Pharmacy, further north is the Manhattan Village Mall, Single Family Residential uses are developed to the east (across Cedar Avenue) and northeast (across 22nd Street), Commercial uses directly to the west is the UCLA Radiology Center and further west across Sepulveda Boulevard there are restaurants, offices, a jewelry store, car rental agency, hair salon and a variety of retail uses; Single Family residential uses to the southeast and commercial uses are developed to the south, including the Manhattan Bread and Bagel Center, Rubios, Citizens Bank and other offices.

PROJECT DETAILS

Parcel Size: 22,455 sf

Building Area: Existing Proposed Allowed

6,371 sf (Multi-tenant) 6,371 sf (Day Care) 22,455 sf 1,840 sf (Office -1.840 sf (Day Care)

(Separate building)

TOTAL 8, 212 sf **8,212 sf** (same as existing)

Parking: Existing Proposed Required

> 23 standard spaces 12 standard spaces 1 space per 1 handicap space 5 compact spaces 7 children 1 handicap space 17 spaces total

Hours of Operation: **Existing** Proposed Allowed

M-F Varies M-F 7am to 6pm By use permit

Employees: Proposed Allowed

> 5 full time By use permit

1-3 part-time teachers 1-2 part-time office

Students: **Proposed** Allowed

> 118 By use permit

Landscaping: Proposed Required

358 sq. ft. 241 sq. ft.

> (5% of 5172 sq.ft. parking area)

5 trees

ENVIRONMENTAL DETERMINATION

In accordance with the provisions of the California Environmental Quality Act (CEQA) as amended by the City of Manhattan Beach CEQA Guidelines, the Community Development Department found that the subject project is exempt as an in-fill Development Project, Class 32, Section 15332.

BACKGROUND (Exhibit B)

The project site is located at 1114-1126 22nd Street, on the corner of 22nd Street and Cedar Avenue in the General Commercial (CG) District, Area District II. It is currently developed as an existing multi-tenant one-story 6,371 square feet building consisting of 5 tenants, a printing shop, 2 restaurants, Stir Fry and Magic Skewer, 1 restaurant/bar, Schooner's, and a hair salon with a surface parking lot with 1 driveway on 22nd Street and two driveways on Cedar Avenue. There is an existing one story 1,840 square feet building located at the southwest of the site in the existing parking lot that is office use and will also be developed as day care use. The surrounding area is developed with a mix of commercial, offices, a pharmacy, a bank, retail, medical centers, restaurants, and residential uses to the east (directly across Cedar Avenue), northeast (across 22nd Street), and further west of the site, across Sepulveda behind commercial uses.

The applicant's architect, Patrick Killen of Studio9one2 Architecture, filed a Use Permit application on June 6, 2014 to remodel and reuse the existing building for a new day care use (CHALK PRESCHOOL). Per Section 10.16.020 of the Commercial Chapter of the City of Manhattan Beach Municipal Code, the subject application requires a use permit for a change in the existing commercial uses to day care. Preschools are classified as day care in the Municipal Code Section 10.08.040 (D). A Use Permit application for a new pre-school at 1030 Manhattan Beach Boulevard was filed by the same applicant in July 2011 and was approved by the Planning Commission on December 14, 2011 and by the City Council on February 21, 2012. However, the project was not built by the owner due to financing issues.

CHALK Pre-School currently has five other locations, including Westwood, Venice, and three in Chicago. The owners of CHALK Pre-School would like to expand their school to the City of Manhattan Beach and offer a pre-school program in a new facility, with a large playground and an on-site parking area. The pre-school program will offer classes and activities to students ranging in ages 2 through 5 with operating hours from 7am to 6pm.

DISCUSSION

Project Proposal

The subject applicant is proposing to remodel both of the existing buildings with a new pre-school use. Specifically, the multi-tenant building will be remodeled and consist of 5 classrooms and a large entry with a check-in and office area, a 7,363 square feet outdoor playground and 12 standard, 5 compact and 1 ADA accessible space for a total of 17 parking spaces. The existing building at the southwest of the site will be remodeled to be used for gross motor skills development. The playground will be developed in the

middle of the site between the main day care facility, the parking area and the motor skills building. The existing driveway on 22nd Street is being removed and will create one new public parking space on the street. The existing driveway on Cedar Avenue closer to the corner of 22nd Street will be removed which will add a second new public parking space, and the second driveway will be widened to create a two-way access to the site.

The proposed hours of operation will be from 7am to 6pm to allow for early drop off and pick up of children. A full-time 7:30-5:30pm program is offered as well as a half-day program from 8:30am to 12:30pm. The expected peak times for drop off will be between 7:30am to 9:15am and peak pick up times between 3pm to 6pm. According to the applicant, it is anticipated that about 50% of the students will attend only half day until 12:30pm and the remainder a full day program. The applicant feels this will eliminate the crowding of the parking area due to the spreading of times students attend classes. Special events or activities, such as an open house, a maximum of three times annually are proposed. Also, no food service will be provided on-site. The students will be required to bring their own food for consumption. For this reason, no food deliveries will be provided at the subject site.

Employees and Students

The total number of proposed full time employees will be five, plus one to three part-time teachers and one to two part time office staff (owners) for a total maximum of ten employees. The owners have other locations to manage and they expect to be at the Manhattan Beach pre-school a maximum of only once or twice a week for a few hours each time. When the school has activities or classes that require outside instructors, the arrival and departure times will vary during non-peak times to avoid parking and traffic congestion.

The total number of proposed children for the subject site is 118. The required parking per Section 10.64.030 of the Zoning Code for day care use is 1 parking space per 7 children. Based on 118 children, the total required parking is 17 spaces, which is what is proposed. The California Department of Social Services (DSS) restricts the number of children to 1 child per 35 square feet of classroom space. Based on 4,126 square feet of classroom space, the maximum total number of children is 118. Also, 75 square feet of outdoor area for each child is required by DSS. Based on the proposed 7,360 square feet of outdoor play area, only 98 children would be allowed. Per the parking chapter of the Zoning Code, 98 children would require only 14 parking spaces. Therefore, the proposed parking spaces of 17 would satisfy the parking requirement. However, if DSS changes their licensing requirements, and the applicant satisfies those requirements, up to 118 children would be allowed and 17 parking spaces would be the required as proposed.

Parking and Access

In terms of parking, the site will provide a total of 17 parking spaces for employees and visitors. The parking lot will be accessible from Cedar Avenue and will allow egress and ingress from one driveway. The pick-up of trash and recycling for the site will be accessed off of 22nd Street for curb-side pick up during non-peak times to avoid traffic

congestion. The new trash enclosure will be located at the northwest corner of the site per the proposed plans.

Use Permit

The Planning Commission must make the following findings in accordance with Section 10.84.060 for the use permit, if the project is approved:

- 1. The proposed location of the use is in accord with the objectives of this title and the purposes of the district in which the site is located;
- 2. The proposed location of the use and the proposed conditions under which it would be operated or maintained will be consistent with the General Plan; will not be detrimental to the public health, safety or welfare of persons residing or working on the proposed project site or in or adjacent to the neighborhood of such use; and will not be detrimental to the public heath, safety or welfare of persons residing or working on the proposed project site or in adjacent to the neighborhood of such use; and will not be detrimental to properties or improvements in the vicinity or to the general welfare of the city;
- 3. The proposed use will comply with the provisions of this title, including any specific condition required for the proposed use in the district in which it would be located; and
- 4. The proposed use will not adversely impact or be adversely impacted by nearby properties. Potential impacts are related but not necessarily limited to: traffic, parking noise, vibration, odors, resident security and personal safety, and aesthetics, or create demands exceeding the capacity of public services and facilities which cannot be mitigated.

The Planning Commission, as part of approving the use permit for the subject project, in accordance with Section 10.84.070 can impose reasonable conditions as necessary to:

- A. Achieve the general purposes of this ordinance or the specific purposes of the zoning district in which the site is located, or to make it consistent with the General Plan:
- B. Protect the public health, safety, and general welfare, or
- C. Ensure operation and maintenance of the use in a manner compatible with existing and potential uses on adjoining properties or in the surrounding area.
- D. Provide for periodic review of the use to determine compliance with conditions imposed, and Municipal Code requirements.

Public Input

The subject project was noticed to residents within 500 feet and was published in the Beach Reporter on August 28, 2014. Staff has not received any public comments. The applicant's representative and architect had a neighborhood meeting on August 28th at the subject site. Six neighbors attended the meeting and issues, such as parking, traffic, noise and other issues related to the proposed project were discussed. According to the architect, the neighbors concerns and questions were satisfied. A set of plans was left

with one of the neighbors to share with the rest of the neighborhood and for those who did not attend.

City Traffic Engineer/Traffic Impact Study (Exhibit C)

A traffic impact study was prepared by Linscott, Law & Greenspan, Engineers on July 10, 2014, as requested by the City Traffic Engineer, after reviewing the proposed project and estimated that it would generate more than 50 trips per day. As a result, the traffic study concluded that a net increase of 87 vehicle trips during the weekday AM peak hour and 65 vehicle trips during the weekday PM peak hour and that the proposed project would not result in significant traffic impacts and no traffic mitigation measures would be required.

The City Traffic Engineer agreed that based on the results of the Traffic Impact Study, there are no significant traffic impacts and had the following conditions to further address any impacts:

- 1. The applicant shall prepare and maintain a Traffic Operations and Management Plan (TOMP) as summarized in the Traffic Impact Study to be followed by faculty, staff, students and parents/guardians. The TOMP shall include, but not be limited to, the following requirements:
 - a. School staff shall be directed to arrive at the on-site parking lot prior to commencement of student drop-off operations and park within designated spaces.
 - b. One to two staff members or volunteers will be positioned within the site parking lot to direct parent/guardian drop-off and pick-up operations and assist during the morning drop-off and afternoon pick-up peak periods.
 - c. Staff or volunteers shall wear safety gear including reflective vests at all times when performing traffic control operations within the parking lot.
 - d. Parents and guardians shall park their vehicles on-site for short-term parking and then escort their pre-school child/children to the appropriate building entrance.
 - e. School-related vehicles (e.g., parents/guardians dropping off students, etc.) will also be directed to travel to the site via Sepulveda Boulevard, Cedar Avenue, and 22nd Street so as to result in a greater disbursement of trips.
 - f. Upon entering the project site, parents and guardians will be encouraged to have their student(s) ready to exit and enter the vehicle safely and efficiently.
 - g. The parking lot gate will remain open during student drop-off and pick-up times.
 - h. School-related vehicles will be directed to not park, drop-off, or pick-up students anywhere along 22nd Street or Cedar Avenue.
 - i. The TOMP should include information on parking operations, site access and circulation, and pre-school student drop-off/pick-up operations. The goal of maintaining and reinforcing the TOMP is to facilitate site access and circulation to/from the site, minimize impacts to the neighborhood

- surrounding the site, and efficiently manage parking facilities provided on the site.
- j. The parking and student drop-off/pick-up operations contained in the TOMP shall be included in Chalk pre-school policies. These school policies should be communicated to faculty, staff, students and parents/guardians at the beginning of the school year and be reinforced throughout the school year.

The project meets all Zoning Code parking and loading requirements.

Other Departments Input (Exhibit D)

The Fire Department, Building and Safety and Public Works/Engineering Divisions had no specific comments or conditions for the project. Additionally, the City Traffic Engineer suggested conditions of approval as described below.

CONCLUSION

Staff feels that the Planning Commission should review and take public input on the proposal.

If the project is approved, the Use Permit should include conditions in the Resolution to address any issues of concern. The following is a list of possible conditions that staff suggests the Planning Commission consider:

Resolution Conditions

- 1) Maximum number of students to be 98 per Department of Social Services required play area of 7,360 square feet.
- 2) If applicant satisfies all requirements, including those of the Department of Social Services, then a maximum of 118 students may be enrolled.
- 3) Maximum number of employees to be 10, 5 full time, 1-3 part-time teachers and 1-2 part-time office staff
- 4) No on-site food service or food service delivery allowed.
- 5) Hours of operation to be 7am to 6pm Monday through Friday.
- 6) Project to maintain a total of 17 parking spaces on-site.
- 7) No large delivery trucks, only standard UPS and FedEx single unit size, not to exceed 30 feet in length.
- 8) Trash and recycling pick up to be conducted during non-peak times, not between 7-9am and 3-6pm.
- 9) Special activities or events will be limited to a maximum of 3 per year.
- 10) All school employees and visitors shall be required to park on-site. No employee parking shall be allowed on residential streets and shall be a violation of the Use Permit.
- 11) Submit annual TOMP parking management plan/program and other conditions required by the City Traffic Engineer including parent, staff, and vendor parking information. The plan should include regular notification and reminders to all who frequent the site.

ALTERNATIVES

The Planning Commission may:

- 1. **DENY** the project subject to public testimony received, based upon appropriate findings, and **DIRECT** Staff to return a draft Resolution.
- 2. **APPROVE** the project subject to public testimony received, based upon appropriate findings, and **DIRECT** Staff to return a draft Resolution.

Attachments:

- A. Vicinity/Aerial Map
- B. Applicant's Application/Project Proposal
- C. Traffic Impact Study dated July 10, 2014
- D. Department Comments
- E. Plans

cc: Patrick Killen, Studio9one2 Architecture, Project Architect

Manhattan Beach





Legend

Addresses

- City Boundary
 Streets
- Parcels

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- Red: Band_1
 Green: Band_2
- Blue: Band_3
- L3_6440_1783a.tif
- Red: Band_1
 Green: Band_2
- Blue: Band_3
- Basemap Labels

EXHIBIT A PC MTG 9-10-14

Scale:1: 1,116

This map is a user generated static output from the "MB GIS Info" Intranet mapping site and is for general reference only. Data layers that appear on this map may or may not be accurate, current, or otherwise reliable.

Notes

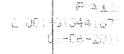
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MASTER APPLICATION FORM

CITY OF MANHATTAN BEACH COMMUNITY DEVELOPMENT DEPARTMENT

Office Use Only

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1114 22 Nº STREET Project Address		Pag Check Submitted.		
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PORTION OF LOTS 6+7 SECTION 19 TOW	INSHIP 3 SOUTH, R	Pauge 14 WEST		
Legal Description				
	GC	AD /I		
General Plan Designation	Zoning Designation	Area District		
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Applicant(s)/Appellant(s) Information PATRICK J KILLEN STUDIO Name	90NE Z ARCHI	7 <i>EC</i> [urf		
930 MANHATTAN BEACH BLVD	MANHATTAN B	GACH CA 902GG		
Mailing Address				
ARCHITECT		on the same		
Applicant(s)/Appellant(s) Relationship to Property	/ > -			
PATRICK KILLEN (310) 376-9171 pjk C5tvd: o9one Z.com Contact Person (include relation to applicant/appellant) Phone number / e-mail				
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SAME AS ABOVE Address:		**************************************		
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Complete Project Description- inclu- pages if necessary)				
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¹ An Application for a Coastal Development Perm application for any other permit or approvals re Beach Municipal Code. (Continued on revers	nt snan be made prio quired for the projec e)	r to, or concurrent with, an or by the City PichNTG 9-10-14		

Page 11 of 164

PC MTG 9-10-14

OWNER'S AFFIDAVIT

COUNTY OF LOS ANGELES	
I/We Mind Pot Simmer depose and say) that I am/we are the owner(s) of	being duly sworn,
the foregoing statements and answers herein column are in all respects frue and correct to the best of m	ntained and the information herewith submitted //our knowledge and belief(s).
Signature of Property Owner(s) – (Not Owner in Escrow	or Lessee)
MARA Pat SIMMENS Print Name	The state of the s
A760 MAIIACAC LANDING D-Mailing Address	Procell OHIO 43065
<u>L14-330-0839</u> Telephone	
Subscribed and sworn to before me, this 29^{th} day of Mhy , $20/4$	
in and for the County of FRANKLIN	John W. Echenrode
State of <u>O H10</u>	
沙运放大业大大公共企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业	Notary Public
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Below are the fees typically associated with the shown on this sheet may apply - refer to curn Department for assistance.) Fees are subject to all	ant City Fee Resolution (contact the Planning
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Variance Filing Fee:	\$ 5,160 🖾
Minor Exception Filling Fee (without notice): Filling Fee (with notice):	\$ 1,775
Subdivision Certificate of Compliance:	2,020 🖼
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Environmental Review (contact Planning Division to Environmental Assessment (no Initial Study Environmental Assessment (if Initial Study Fish and Game/CEQA Exemption County C	prepared): \$ 215
Public Notification Fee applies to all project covers the city's costs of envelopes, postagmailing of public notices. Add this to filing f	e and handling the

G-REARRING DIVISION Forms-Checking-County Handout/Manus Application Form 2011.doc -- Revised 12-13-12

²Make a separate \$75 check payable to LA County Clerk, (DO NOT PUT DATE ON CHECK)

ARCHITECTURE

Patrick J. Killen A.I.A.

June 4, 2014

Chalk Preschool Project 1114 22nd Street Manhattan Beach, CA

PROJECT DESCRIPTION

CHALK Preschool of Manhattan Beach is proposed at 1114 22nd Street in Manhattan Beach, California, This incredible addition to the Manhattan Beach community is a 8,212 square foot sunny facility containing five classrooms surrounding a 7,360 square foot playground. This secured campus is sure to create endless opportunities for on-site explorations and adventures. The school will provide a necessary part time and full time preschool option for the area's many surrounding families having children ages 2-5 years old, with extended hours ranging from 7:00 am - 6:00 pm Monday thru Friday. On site there will be 17 parking spaces for easy drop off and pick up. Drop off peak times will occur between 7:00 am to 9:00 am, with pick up times more varied. Approximately 25% of the children will only attend school for half a day and will be picked up around noon. The remainder will be picked up at times ranging from 3:00 pm to closing, thereby spreading out the car trips entering the parking lot. The school will employ one teacher per classroom full-time. The business owners manage and will also spend time at the facility but as they have six facilities to run, they will only be present once or twice a week for part of those days. The staff count will be 6 full-time and 1 or 2 part time. As do most other local schools, CHALK Preschool will, at times, have special classes and/ or activities for the children. These classes are often taught by outside instructors who typically arrive after the drop-off times and leave prior to the afternoon pick-up times, and as such have little impact on traffic volumes. Roughly 3 times a year, the school will host open house type events where parents are invited to visit the school.

There are presently 5 other CHALK locations in Southern California and Chicago. None of these locations offer food service. All food consumed by the students will be brought to school by the individual. Other supplies used by the school including office and art supplies, are typically purchased by staff at local stores. No large trucks are anticipated to access the site with the possible exception of the intermittent UPS or FedEx deliveries. Trash is currently picked up along the west side of Cedar Avenue. The trash area for this project is designed to allow for curb-side pick up which takes less than 5 minutes to perform. Studio 9one2 will work with Waste Management to ensure the pick up time does not occur during peak traffic hours.

The California Building Code sets the number of occupants at 1 person per 20 square feet of classroom for this building type. Based on this number, the building could have a potential occupancy of 210. The California Department of Social Services, however, restricts the number of children in each classroom to 1 child per 35 square feet. At 4,126 square feet of classroom space, the number of children for which the site could be licensed would be 118. The parking for this development was based on this number (118 at 1 space per 7 = 17 parking spaces required). Moreover, Social Services also requires 75 square feet of outdoor area for each child. With the play area size proposed, it is easily seen that the facility will be licensed for fewer children than the number that could be allowed based on the parking provided (7,360 square feet of play area / 75 square feet per child nets 98 students. Per Manhattan Beach Municipal Code, 98 students would only require 14 parking spaces. In this respect, the facility could be providing more parking than is required by Manhattan Beach Municipal Code.

930 C MANHATTAN BEACH BLVD. MANHATTAN BEACH CA 90266 TEL. 310 / 376-9171 FAX . 310 / 376-1822

ARCHITECTURE

Patrick J. Killen A.I.A.

Per the Owners of CHALK, the only full-time staff would be the 6 teachers, one in each classroom. If at times the ratio of child to teacher exceeds the State allowed 12 children to 1 adult supervisor, then the school will have part-time teacher aids to bring the ratio into compliance. The number of part-time teacher aids will fluctuate between 1 and 3. With all preschools, the number of children present at any one time is dependent on a number of factors. The number of children enrolled is the largest of these, but also, what type of enrollment to which each child subscribes. Because it is not required education, there is a tremendous amount of flexibility associated with preschools. Some will be full-time Monday through Friday. Others may be full-time but only a few days of the week. Full-time hours are from 7:30 am to 5:30 pm. Still others will be half-day, and some half day students also will not attend every day. Half-day hours are from 8:30 am to 12:30 pm. Currently, CHALK finds that roughly 50% of there students are full-day and 50% are half day. The school will not be able to determine the required staff until two conditions have occurred. First, they will need a license from the DSS which stipulates the maximum number of children allowed. Second, they will need to know an actual number of enrolled students and what hours those students will attend the school. As mentioned previously, office staff will consist solely of the owners and they will be at the location once or twice a week for a few hours each time. There will be no other staff. Based on their experience with the other two locations they presently operate in the Los Angeles area, the owners believe that these staff levels meet the state requirements and they do not anticipate a larger group.

The playground area totals 7,360 square feet. Per the DSS requirement of 75 square feet of outdoor play area per licensed child, that would limit the State License to 98 children. While there has been limited flexibility in the outdoor play area requirements with other preschools, that still would not allow a licensed number of 118 children.

Per Manhattan Beach Municipal Code, we have provided the amount of parking required for 118 children to attend the school. While we do not anticipate that the school will be licensed for that many students, we have, however, provided the parking to allow it. CHALK also varies its allowed drop-off times for the full-day and half-day students. Full-time students may be dropped off between the hours of 7:30 to 8:30 am, while half-day student drop-off is from 8:30 to 9:15 am. With their other facilities, CHALK finds that the average drop-off time for each child to be about 6 minutes. Each parking space could then account for 10 drop-offs per hour. If the school were licensed for 118 students, that would mean roughly 60 kids would need dropped off during each drop-off period (50% each for full-day and half-day). Even accounting for staff use of allotted parking, the remaining number of spaces should be more than adequate to allow a smooth drop-off period. Pick up times are more varied throughout the afternoon and so there tends to be little parking congestion at these times.

930 C MANHATTAN BEACH BLVD. MANHATTAN BEACH CA 90266 TEL. 310 / 376-9171 FAX . 310 / 376-1822

ARCHITECTURE

Patrick J. Killen A.I.A.

DATE:

June 5, 2014

TO:

CITY OF MANHATTAN BEACH

COMMUNITY DEVELOPMENT DEPARTMENT ATTN: ERIC HAALAND, ASSOCIATE PLANNER

FROM:

Studio9One2, Howard Crabtree, Architect

SUBJECT: 1114 - 22nd STREET

CUP for CHALK PRESCHOOL

Applicant: Patrick J. Killen, Architect - Studio 9one2 Architecture

The following documents are submitted in conjunction with the application for Conditional Use Permit

- 1. Master Application with attached description
- 2. Environmental Information Form
- 3. Narrative & response to findings SUBMIT UNDER SEPARATE COVER HOWARD
- 4. Grant deed
- 5. Radius Map Package
- 6. 2 Sets Of Full Scale Architectural Drawings & 3 sets reduced drawings
- 7. \$ \$ \$ 9595 __check payable to MB as filing fee
- & undated \$75.00 check payable to LA County Clerk

Please let me know if anything further is required with regard to the preliminary review.



ENVIRONMENTAL INFORMATION FORM

(to be completed by applicant)

CITY OF MANHATTAN BEACH COMMUNITY DEVELOPMENT DEPARTMENT

Date Filed: 6 · 5 · 14	
APPLICANT INFORMATION	
Name: PATRICK J. KILLEN	Contact Person: Howard CARBNASS
Address: 930 MANHATTAN BEACH BC	Address: 930 MANHATTA BCH BG. M.B. 7020
Phone number: 318 920 - 9171	Phone number: 310 376-9171
Relationship to property: Apcurect	Association to applicant: Emasses
PROJECT LOCATION AND LAND USE Project Address: ///4 22 ** STEEST	MANHATTAN BEACH
Assessor's Parcel Number:	
Legal Description: PORTION OF COTS G	+7 SECTION 19 TOWNSHIP 3 SOUTH RANGE 14WEST
Area District, Zoning, General Plan Design	+7 SECTION 19 TOWNSHIP 3 SOUTH RAMGE 14WEST
Surrounding Land Uses:	
North RETAIL (CVS)	West RESIDENTIAL
South COMMERCIAL OFFICE	West <u>RESIDENTIAL</u> East <u>MEDICAL OFFICE</u>
Existing Land Use:COMMERCIAL (RESTAURANT + BAR)
PROJECT DESCRIPTION Type of Project: Commercial X Residential, indicate type of condominium, etc.) and number of	development (i.e.: single family anartment
seats, square footage of kitchen s	n (neighborhood, citywide, or regional), type of cion, number of employees, number of fixed seating, sales, and storage areas: CHALK PRESCHOOL SOHOL OPERATIONS MON TO FAL. FROM 744-6 pm
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Number of Floor	s/Stories:	<u> </u>	ONE	مسين		
Percent Lot Cov	erage:	37%	37%	40000		
Off-Street Parkir	ng:	28 (NOW. CONF)	17	17	11	
Vehicle Loading	Space:	NA	N/R	w/R		
Open Space/Lar		0	6,184	241	0	

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X	Water quality impacts (surface or ground), or affect drainage patters?					
X	An increase in existing noise levels?					
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Explain all "Yes"	' responses (<i>atta</i>	ch additional s	sheets or atta	chments as n	ecessary):	

CERTIFICATION	N· I hereby certif	iv that the stat	tomonto filium:	-1		
CERTIFICATION : I hereby certify that the statements furnished above and in attached exhibits present the data and information required for this initial evaluation to the best of my oblitic and that the						
of my ability, and that the facts, statements, and information presented are true and						
correct to the best of my knowledge and belief.						
Signature:		Pr		patack Riccion Shack Presch		
Date Prepared:	6.2.2014		oparca i or. <u>c</u>			
Revised 7/97						



TRAFFIC IMPACT STUDY

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

City of Manhattan Beach, California July 10, 2014

Prepared for:

Studio 9one2 Architecture

930 Manhattan Beach Boulevard Manhattan Beach, California 90266

LLG Ref. 1-14-4083-1



EXHIBIT C PC MTG 9-10-14

Under the Supervision of:

Clare m. Fook - goeger

Clare M. Look-Jaeger, P.E. Principal

Linscott, Law & Greenspan, Engineers

600 S. Lake Avenue Suite 500 Pasadena, CA 91106

626.796.2322 т 626.792.0941 Page 19 of 164 www.llge**pages(15-70-9**-10-14

PC MTG 9-24-14 Page 34 of 179

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TRAFFIC IMPACT STUDY

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

City of Manhattan Beach, California July 10, 2014

1.0 Introduction

This traffic impact study addresses the potential traffic impacts associated with the proposed Chalk Pre-School Manhattan Beach project ("proposed project"). The project applicant proposes to remodel and re-use the existing buildings on-site for the operation of a private pre-school located at 1114 22nd Street in the City of Manhattan Beach, California. The proposed project site is situated at the southwest corner of Cedar Avenue and 22nd Street. The proposed Chalk Pre-School Manhattan Beach project site location and general vicinity are shown in *Figure 1-1*.

This report documents the findings and recommendations of a traffic impact analysis prepared by Linscott, Law & Greenspan, Engineers (LLG Engineers) to determine the potential impacts associated with the proposed Chalk Pre-School Manhattan Beach project. The traffic analysis evaluates the existing operating conditions at a total of eight (8) study locations, including six (6) study intersections and two (2) study street segments within the project vicinity, estimates the trip generation potential of the proposed project, and forecasts future operating conditions without and with the proposed project. Where necessary, intersection improvements and/or mitigation measures are identified. The Scope of Work for this traffic study report has been prepared in consultation with City of Manhattan Beach staff and the City's consultant Traffic Engineer.

This traffic report satisfies the traffic impact study requirements of the City of Manhattan Beach and is consistent with the 2010 Congestion Management Program for Los Angeles County. The project site has been visited and an inventory of adjacent area roadways and intersections was performed. Existing peak hour traffic information has been collected at the six key study intersections on a typical weekday for use in the preparation of intersection Level of Service calculations. The Intersection Capacity Utilization method was used to determine volume-to-capacity ratios and corresponding Levels of Service for the signalized study intersections while the analysis method from the *Highway Capacity Manual*² (HCM2010) was utilized to determine intersection delay values and corresponding Levels of Service for the two unsignalized study intersections.

Information concerning cumulative projects (planned and/or approved) in the vicinity of the proposed project has been researched at the Cities of Manhattan Beach, Hermosa Beach and El Segundo. Based on this research, a total of ten (10) related projects have been included in the traffic

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PC MTG 9-10-14

¹ 2010 Congestion Management Program for Los Angeles County, Los Angeles County Metropolitan Transportation Authority, 2010.

² Highway Capacity Manual, Transportation Research Board, National Research Council, Washington D.C., 2010.

VICINITY MAP

FIGURE 1-1

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STUDY STREET SEGMENT

LINSCOTT, LAW & GREENSPAN, engineers

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impact study. These ten (10) planned and/or approved related projects were therefore considered in the cumulative traffic analysis for this project.

This traffic report analyzes existing and future weekday AM peak hour and PM peak hour traffic conditions for a future-term (Year 2015) traffic setting upon completion of the proposed Chalk Pre-School Manhattan Beach project. Peak hour traffic forecasts for the Year 2015 horizon year have been projected by increasing existing traffic volumes by an annual growth rate of one percent (1.0%) per year and adding traffic volumes generated by the ten (10) related projects. In addition, the planned project parking supply is compared with the City of Manhattan Beach Off-street Parking Code requirement.

1.1 Study Area

A total of eight (8) locations, including six study intersections and two study street segments have been identified for evaluation during the weekday morning and afternoon peak hours based upon coordination with the City of Manhattan Beach consultant Traffic Engineer. These study locations provide local access to the study area and define the extent of the boundaries for this traffic impact investigation. Further discussion of the existing street system and study area is provided in Section 4.0 herein.

The general location of the project in relation to the study locations and surrounding street system is presented in *Figure 1-1*. The traffic analysis study area is generally comprised of those locations which have the greatest potential to experience significant traffic impacts due to the proposed project as defined by the Lead Agency. In the traffic engineering practice, the study area generally includes those intersections that are:

- a. Immediately adjacent or in close proximity to the project site;
- b. In the vicinity of the project site that are documented to have current or projected future adverse operational issues; and
- c. In the vicinity of the project site that are forecast to experience a relatively greater percentage of project-related vehicular turning movements.

The locations selected for analysis were based on the above criteria, proposed Chalk Pre-School Manhattan Beach project peak hour vehicle trip generation, anticipated distribution of project vehicular trips and existing intersection/corridor operations. As mentioned previously, a total of eight study locations, including six study intersections and two study street segments define the extent of the boundaries for this traffic impact investigation.

2.0 PROJECT DESCRIPTION

2.1 Site Location

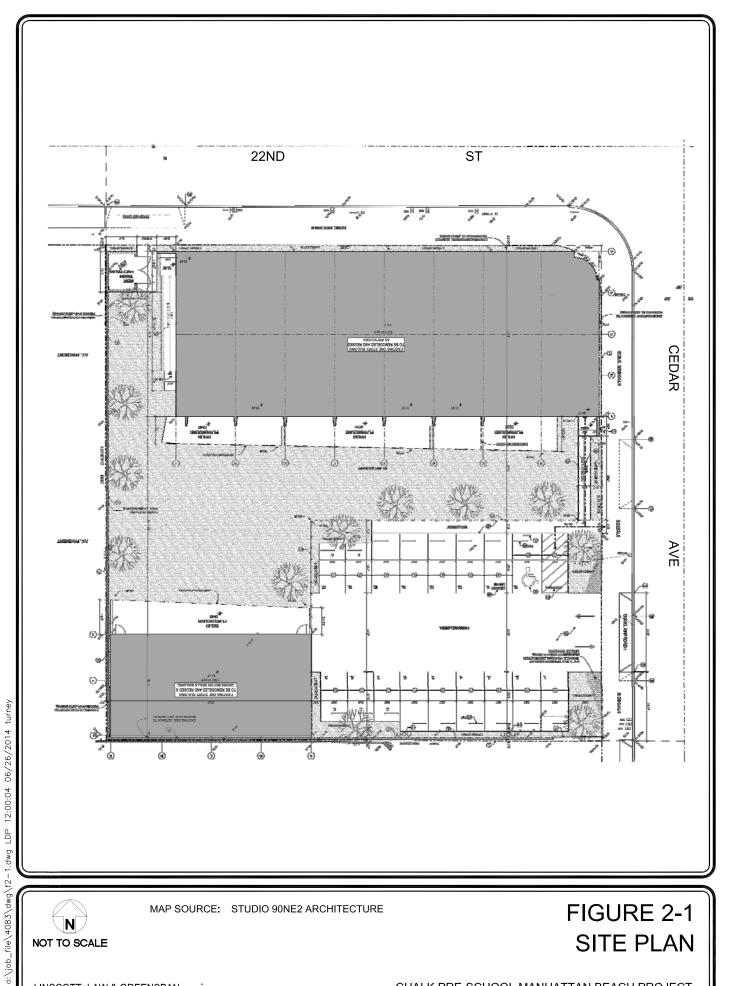
The proposed development project is located at 1114 22nd Street in the City of Manhattan Beach, California. The project site is situated along the south side of 22nd Street west of Cedar Avenue. The project site is bounded by surface parking lots for existing retail and office development to the south and west, 22nd Street to the north, and Cedar Avenue and residential uses to the east. Additionally, the project site is located in the Eastside section of Manhattan Beach, which is primarily comprised of distinct single-family residences with numerous public schools bounded generally by Marine Avenue, Sepulveda Boulevard, Aviation Boulevard and Artesia Boulevard. The proposed project site location and general vicinity are shown in *Figure 1–1*.

2.2 Project Description

The project applicant seeks to obtain entitlements to adaptively re-use the existing buildings located at 1114 22nd Street to operate a private pre-school facility for up to 119 students. Two existing buildings totaling 8,232 square feet of building area is currently utilized as a food sales, retail and personal service type uses. Both buildings will be renovated to accommodate occupancy by the proposed Chalk Pre-School facility. The existing building (6,372 square feet) at the northeast corner of the project site will function as the main pre-school building and a smaller 1,860 square-foot building at the southwest corner of the site will offer a separate motor skills building for the facility. The Chalk Pre-School facility will also include playground areas between the two buildings.

The Chalk Pre-School facility will provide a necessary part-time and full-time preschool option to surrounding families having children ages 2 to 5 years old, with extended hours ranging from 7:00 AM to 6:00 PM, Monday through Friday. Drop-off peak times are expected to occur between 7:00 AM and 9:00 AM, with pick-up times being more varied. Based on information provided by the Applicant, approximately 25 percent of the children will only attend school for half of the day and be picked up around 12:00 Noon. The remainder will be picked up at times ranging from 3:00 PM to closing. The staff count is expected at six full-time and one or two part-time members. If special outside instructors are needed, these instructors would arrive after the drop-off time and depart prior to the afternoon pick-up period. Therefore, while there can be many instances where not all 119 preschool students are present, all 119 students have been assumed to be on-site for analysis purposes.

It is anticipated that the adaptive re-use of the existing buildings at 1114 22nd Street by the Chalk Pre-School facility will commence in year 2015. The site plan for the proposed project is illustrated in *Figure 2-1*. A discussion of the project's site access and general pre-school traffic procedures is provided in Section 3.0 herein.



NOT TO SCALE

MAP SOURCE: STUDIO 90NE2 ARCHITECTURE

FIGURE 2-1 SITE PLAN

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT LINSCOTT, LAW & GREENSPAN, engineers

2.3 Project Parking

This section summarizes the review of the parking requirements for the proposed project according to the City of Manhattan Beach Municipal Code requirements and the planned project parking supply. It is anticipated that the proposed project will provide Code required parking as determined by the City of Manhattan Beach.

2.3.1 City of Manhattan Beach Code Parking Requirements

The City of Manhattan Beach off-street parking requirements for general daycare facilities are set forth in Section 10.34.060, *Off-Street Parking and Loading Spaces Required*, of the Municipal Code. Through application of the Municipal Code parking regulations, the following parking requirement would be calculated for the proposed project:

• Daycare, General – One (1.0) space per seven (7) children; maximum enrollment based on maximum occupancy load.

Based on project description information provided by the project applicant and strict application of Code to the maximum enrollment of 119 children, a total of 17 spaces would be required for the project site as summarized below:

• General Daycare Facility: 119 children x 1.0 space/7.0 children = 17 spaces
Total City Code Required Project Parking = 17 spaces

2.3.2 Proposed Project Parking Supply

As indicated in *Figure 2-1*, surface parking will be provided within the project site in a surface parking lot fronting Cedar Avenue between the two buildings. A total of 17 parking spaces is planned to be provided within the project site, with approximately 11 standard spaces, five (5) compact spaces, and one (1) handicap accessible space. As the proposed on-site parking supply totals 17 spaces, the proposed Chalk Pre-School Manhattan Beach project would provide the number of required spaces under the provisions of the Municipal Code.

3.0 SITE ACCESS AND CIRCULATION

The proposed site access scheme for the Chalk Pre-School Manhattan Beach project is displayed in *Figure 2-1*. A description of the proposed site access and circulation scheme is provided in the following subsections.

3.1 Vehicular Project Site Access

Vehicular access to the site is currently provided via a total of three existing site driveways: two driveways on Cedar Avenue along the easterly property frontage and one driveway on 22^{nd} Street along the northerly property frontage. The existing 22^{nd} Street driveway and the northerly Cedar Avenue driveway will both be closed pursuant to City standards with new concrete curb, gutter and sidewalk. The existing southerly Cedar Avenue driveway will be slightly relocated and retained. A description of the project site driveway for the Chalk Pre-School Manhattan Beach project is provided in the following paragraph.

• Proposed Cedar Avenue Project Driveway:

The Cedar Avenue project driveway is an existing driveway that will be slightly relocated and retained. This project driveway is located along the west side of Cedar Avenue near the southeasterly property frontage and is planned to accommodate full access (i.e., right-turn and left-turn ingress and egress turning movements). The Cedar Avenue project driveway will provide access to the on-site surface parking lot.

3.2 General Pre-School Traffic Procedures

The project site and vicinity were reviewed to identify the preferred drop-off and pick-up circulation scheme to accommodate access for pre-school drop-off and pick-up operations. As part of the proposed project, a circulation scheme will be utilized by pre-school parents/guardians in which drop-off/pick-up operations will be conducted in the proposed surface parking lot. The goal is to better accommodate parents/guardians dropping-off and picking-up pre-school students, while at the same time providing a safe environment for the students. Listed below is a summary of the recommendations regarding the Chalk Pre-School Manhattan Beach project's general traffic operations:

- School staff will be directed to arrive at the on-site parking lot prior to commencement of student drop-off operations and park within designated spaces.
- One to two staff members, or volunteers, will be positioned within the site parking lot to direct traffic operations during the morning drop-off and afternoon pick-up peak periods. The staff will be positioned to clearly observe parent/guardian drop-off and pick-up operations and assist in the unloading and loading operations. Staff may also direct parents/guardians into and out of the available parking spaces.
- Parents and guardians would be able to park their vehicles on-site for short-term parking and then can escort their pre-school child/children to the appropriate building entrance. Based on

LINSCOTT, LAW & GREENSPAN, engineers

LLG Ref. 1-14-4083-1 Chalk Pre-School Manhattan Beach Project information provided by Chalk representatives through experience with other operational Chalk pre-school facilities, the average drop-off time for each child is roughly six (6) minutes. As such, each parking space could accommodate 10 drop-offs per hour.

- School-related vehicles (e.g., parents/guardians dropping off students, etc.) will also be directed to travel to the site via Sepulveda Boulevard, Cedar Avenue, and 22nd Street so as to result in a greater disbursement of trips.
- Upon entering the project site, parents and guardians will be encouraged to have their student(s) ready to exit and enter the vehicle safely and efficiently.
- The parking lot gate will remain open during student drop-off and pick-up times.
- School-related vehicles will be directed to not park, drop-off, or pick-up students anywhere along 22nd Street or Cedar Avenue.
- It is recommended that staff, or volunteers, wear safety gear including reflective vests, hats and gloves at all times when performing traffic control operations within the parking lot.
- It is recommended that a Traffic Management Plan (TMP) be maintained that details the above operational protocol for faculty, staff, students and parents/guardians. The TMP should include information on parking operations, site access and circulation, and pre-school student drop-off/pick-up operations. The goal of maintaining and re-enforcing the TMP is to facilitate site access and circulation to/from the site, minimize impacts to the neighborhood surrounding the site, and efficiently manage parking facilities provided on the site.
- It is also recommended that the parking and student drop-off/pick-up operations contained in the TMP be included in Chalk pre-school policies. These school policies should be communicated to faculty, staff, students and parents/guardians at the beginning of the school year and be reinforced throughout the school year.

3.3 Pedestrian Access

The proposed project site has been designed to encourage pedestrian activity and walking as a transportation mode³. As indicated in *Figure 2-1*, pedestrian walkways are planned throughout the site, as well as connected to the adjacent sidewalks, in a manner that promotes walkability. Walkability is a term for the extent to which walking is readily available as a safe, connected, accessible and pleasant mode of transport. There are five basic requirements that are widely accepted as key aspects of the walkability of urban areas that should be satisfied. The underlying principle is that pedestrians should not be delayed, diverted, or placed in danger. The five primary characteristics of walkability are as follows:

³ For example, refer to http://www.walkscore.com/, which generates a walkability score of approximately 63 (Somewhat Walkable – most errands can be accomplished on foot) out of 100 for the project site. Walk Score calculates the walkability of an address by locating nearby stores, restaurants, schools, parks, etc. Walk Score measures how easy it is to live a car-lite lifestyle—not how pretty the area is for walking.

- Connectivity: People can walk from one place to another without encountering major obstacles, obstructions, or loss of connectivity.
- Convivial: Pedestrian routes are friendly and attractive, and are perceived as such by pedestrians.
- Conspicuous: Suitable levels of lighting, visibility and surveillance over its entire length, with high quality delineation and signage.
- Comfortable: High quality and well-maintained footpaths of suitable widths, attractive landscaping and architecture, shelter and rest spaces, and a suitable allocation of roadspace to pedestrians.
- Convenient: Walking is a realistic travel choice, partly because of the impact of the other criteria set forth above, but also because walking routes are of a suitable length as a result of land use planning with minimal delays.

A review of the project site plan and pedestrian walkways indicates that these five primary characteristics are accommodated as part of the proposed project. The project site is adjacent to and accessible from nearby residential neighborhoods and retail, restaurant and other commercial land use opportunities along the Sepulveda Boulevard corridor. The pedestrian walkways within the site will be appropriately landscaped and adorned to provide a friendly walking environment. Additionally, the walkways and connections with the external environment will be well lit.

Pedestrian project access to the site will be provided along the Cedar Avenue and 22nd Street property frontages and via new walkways. Pedestrian circulation around the periphery of the project site will be accommodated by the public sidewalks with planned walkway connections to the site's building entrances.

4.0 EXISTING STREET SYSTEM

4.1 Study Intersections

Immediate access to the project site is provided via Cedar Avenue and 22nd Street. The following six study intersections were selected for analysis in consultation with City staff in order to determine potential impacts related to the proposed project:

- 1. Sepulveda Boulevard/Marine Avenue
- 2. Sepulveda Boulevard/22nd Street^[a]
- 3. Sepulveda Boulevard/18th Street
- 4. Cedar Avenue/Marine Avenue
- 5. Cedar Avenue/22nd Street^[b]
- 6. Meadows Avenue/Marine Avenue

Four of the study intersections are presently controlled by traffic signals, while the remaining two study intersections are currently stop-sign controlled (No. 2, Sepulveda Boulevard/22nd Street and No. 5, Cedar Avenue/22nd Street). The existing roadway configurations and intersection controls at the six study intersections are displayed in *Figure 4-1*.

4.2 Study Street Segments

The following two study street segment locations were identified for analysis by City staff for inclusion into the street segment analysis:

- 1. 22nd Street between Sepulveda Boulevard and Cedar Avenue
- 2. Cedar Avenue between 22nd Street and 21st Avenue

The existing travel lanes and posted speed limits on the study street segments are discussed further in Section 4.4 herein.

4.3 Roadway Classifications

The City of Manhattan Beach utilizes the roadway categories recognized by regional, state and federal transportation agencies. There are four categories in the roadway hierarchy, ranging from freeways with the highest capacity to two-lane undivided roadways with the lowest capacity. The roadway categories are summarized as follows:

[[]a] Two-way stop-sign controlled intersection.

[[]b] All-way stop-sign controlled intersection.

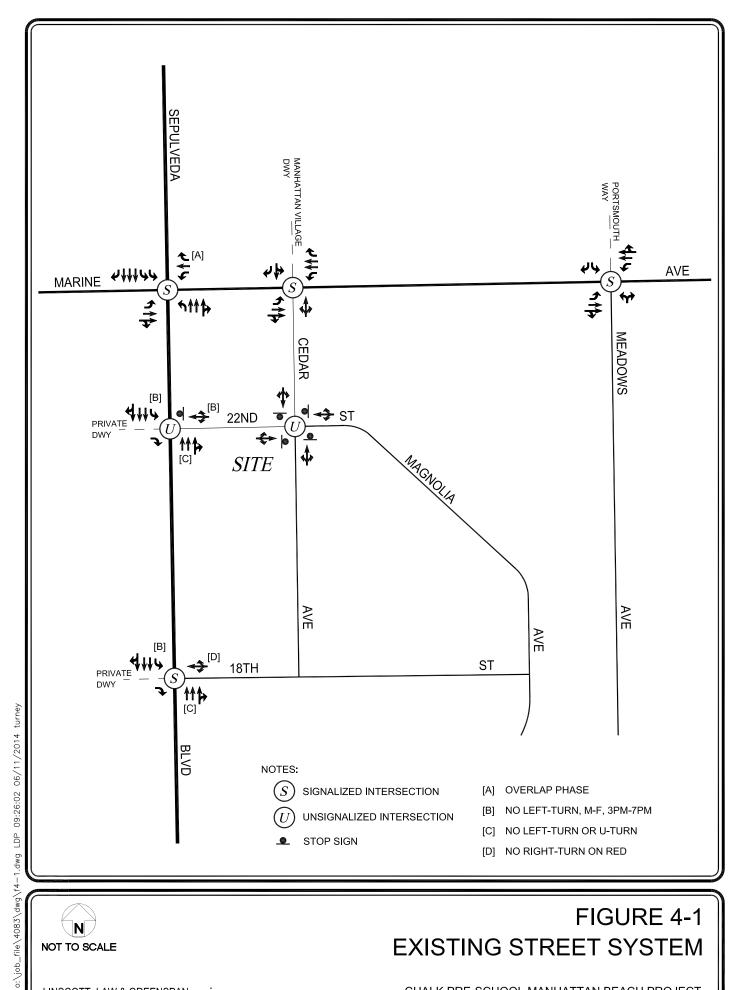




FIGURE 4-1 **EXISTING STREET SYSTEM**

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- *Freeways* are limited-access and high speed travel ways included in the state and federal highway systems. Their purpose is to carry regional through-traffic. Access is provided by interchanges with typical spacing of one mile or greater. No local access is provided to adjacent land uses.
- Arterial roadways are major streets that primarily serve through-traffic and provide access to
 abutting properties as a secondary function. Arterials are generally designed with two to six
 travel lanes and their major intersections are signalized. This roadway type is divided into two
 categories: principal and minor arterials. Principal arterials are typically four-or-more lane
 roadways and serve both local and regional through-traffic. Minor arterials are typically two-tofour lane streets that service local and commute traffic.
- Collector roadways are streets that provide access and traffic circulation within residential and non-residential (e.g., commercial and industrial) areas. Collector roadways connect local streets to arterials and are typically designed with two through travel lanes (i.e., one through travel lane in each direction) that may accommodate on-street parking. They may also provide access to abutting properties.
- Local roadways distribute traffic within a neighborhood, or similar adjacent neighborhoods, and
 are not intended for use as a through-street or a link between higher capacity facilities such as
 collector or arterial roadways. Local streets are fronted by residential uses and do not typically
 serve commercial uses.

4.4 Roadway Descriptions

A brief description of the important roadways in the project site vicinity is provided in the following paragraphs.

Sepulveda Boulevard (State Route 1) is a north-south oriented roadway that is located one block west of the project site and provides connection to the I-105 Freeway to the north and Artesia Boulevard (SR-91) to the south. Sepulveda Boulevard is classified as the only Regional Arterial in the City of Manhattan Beach General Plan Infrastructure Element. Three to four through travel lanes are provided in each direction on Sepulveda Boulevard and separate left-turn lanes are provided at major signalized intersections, including dual southbound left-turn lanes at the Marine Avenue intersection. Sepulveda Boulevard is posted for a 35 miles per hour speed limit near the project site.

Cedar Avenue is a short north-south oriented roadway that borders the project site to the east and extends from Marine Avenue to the north to 18th Street to the south. Cedar Avenue is classified as a Local street in the Infrastructure Element of the City of Manhattan Beach General Plan. One through travel lane is provided in each direction on Cedar Avenue in the project vicinity. Cedar Avenue is posted for a 25 miles per hour speed limit in the project vicinity.

Meadows Avenue is a north-south oriented roadway located east of the project site. Meadows Avenue is classified as a Major Local in the City of Manhattan Beach General Plan. Meadows

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LLG Ref. 1-14-4083-1

PC MTG 9-10-14

Avenue provides one through travel lane in each direction south of Marine Avenue. Meadows Avenue is posted for a 25 miles per hour speed limit in the project vicinity.

Marine Avenue is an east-west oriented roadway that is located north of the project site. Marine Avenue is classified as a Minor Arterial and Residential Collector, east and west of Sepulveda Boulevard, respectively, in the Infrastructure Element of the City of Manhattan Beach General Plan. Marine Avenue extends across the City of Manhattan Beach and provides two lanes in each direction in the project vicinity. Marine Avenue provides left-turn lanes at major intersections, including Sepulveda Boulevard, Cedar Avenue and Meadows Avenue. Marine Avenue is posted for a speed limit of 35 and 25 miles per hour, east and west of Sepulveda Boulevard, respectively.

22nd Street is an east-west oriented roadway that is borders the project site to the north. 22nd Street is classified as a Local street in the City's General Plan Infrastructure Element. One through travel lane is provided in each direction along the project frontage. At its intersection with Sepulveda Boulevard, 22nd Street is stop-sign controlled and westbound left-turns are prohibited from 22nd Street to Sepulveda Boulevard during the weekday afternoon peak period between 3:00 PM and 7:00 PM. The intersection of 22nd Street and Cedar Avenue functions as an all-way stop-sign controlled intersection adjacent to the project site. There is no posted speed limit on 22nd Street in the project vicinity, thus it is assumed to be a prima facie speed limit of 25 miles per hour.

18th Street is an east-west discontinuous roadway located south of the project site. 18th Street is a Local street that provides one through travel lane in each direction in the project vicinity. The segment of 18th Street in the project vicinity extends between Sepulveda Boulevard and Magnolia Avenue. No right-turns on red are permitted on 18th Street at its intersection with Sepulveda Boulevard. There is no posted speed limit on 18th Street in the project vicinity, thus it is assumed to be a prima facie speed limit of 25 miles per hour.

4.5 Existing Public Bus Transit Service

Public bus transit service within the Chalk Pre-School Manhattan Beach project study area is also currently provided by the Los Angeles County Metropolitan Transportation Authority. A summary of the existing transit service, including the transit route, destinations and peak hour headways is presented in *Table 4-1*. The existing public transit routes in the Chalk Pre-School Manhattan Beach project site vicinity are illustrated in *Figure 4-2*.

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Chalk Pre-School Manhattan Beach Project

Table 4-1
EXISTING TRANSIT ROUTES [1]

		ROADWAY(S)	NO. OF BUSES DURING PEAK HOUR		-
ROUTE	DESTINATIONS	NEAR SITE	DIR	AM	PM
Metro 232	Long Beach to LAX via Wilmington, Harbor City, Torrance, Redondo Beach, Hermosa Beach, Manhattan Beach, El Segundo	Sepulveda Boulevard	NB SB	4 4	3 4
			Total	8	7

[1] Source: Los Angeles County Metropolitan Transportation Authority (Metro) website, 2014.



PROJECT SITE

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EXISTING PUBLIC TRANSIT ROUTES

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

5.0 TRAFFIC COUNTS

5.1 Manual Intersection Traffic Counts

New manual counts of vehicular turning movements were conducted at each of the study intersections during the weekday morning (AM) and afternoon (PM) commuter periods to determine the peak hour traffic volumes. The manual counts were conducted by a traffic count subconsultant (City Traffic Counters) at study intersections from 7:00 to 9:00 AM to determine the AM peak commuter hour, and from 4:00 to 6:00 PM to determine the PM peak commuter hour. In conjunction with the manual turning movement vehicle counts, a count of bicycle and pedestrian volumes were collected during the peak periods. It is noted that all of the traffic counts were conducted when local schools were in session. Traffic volumes at the study intersections show the morning and afternoon peak periods typically associated with peak commuter hours in the metropolitan area.

The existing weekday AM and PM peak commuter period manual counts of turning vehicles at the study intersections are summarized in *Table 5-1*. The existing traffic volumes at the study intersections during the weekday AM and PM peak commuter hours are shown in *Figures 5-1* and *5-2*, respectively. For each study intersection, the highest one-hour total traffic volumes (i.e., four consecutive 15-minute time intervals) traversing through the intersection during the 7:00 to 9:00 AM and 4:00 to 6:00 PM time periods were selected so as to determine the respective AM and PM peak hour traffic volumes for each study intersection. For purposes of the traffic impact analysis, this common traffic engineering practice ensures that a more conservative (i.e., worst case) assessment of existing operating conditions be attained for each study intersection. Therefore, the traffic volumes shown in *Figures 5-1* and *5-2* for the study intersections do not necessarily reflect the same exact one hour time period during the morning and/or afternoon peak commuter conditions (i.e., one intersection's peak hour may have occurred between 7:30 and 8:30 AM, while another intersection's peak hour may have occurred between 7:45 and 8:45 AM). Summary data worksheets of the manual traffic counts of the study intersections are contained in *Appendix A*.

5.2 Automatic 24-Hour Machine Traffic Counts

Automatic 24-hour machine traffic counts of the two study street segments were conducted by a traffic subconsultant, City Traffic Counters. As noted above, the automatic 24-hour machine traffic counts were conducted when local schools were in session and in conjunction with the manual intersection traffic counts. Copies of the 24-hour machine traffic counts for the study street segment locations are also contained in *Appendix A*.

Table 5-1 **EXISTING TRAFFIC VOLUMES [1]**

				AM PE	AK HOUR	PM PEAK HOUR		
NO.	INTERSECTION	DATE	DIR	BEGAN	EGAN VOLUME		VOLUME	
1	Sepulveda Boulevard/	06/10/2014	NB	7:45	2,655	4:30	1,578	
	Marine Avenue		SB		1,152		2,406	
			EB		444		407	
			WB		717		614	
2	C 1 1 D 1 1/	06/10/2014	NID	7.45	2.646	4.00	1.706	
2	Sepulveda Boulevard/	06/10/2014	NB	7:45	2,646	4:00	1,726	
	22nd Street		SB		1,006		2,320	
			EB		0		0	
			WB		11		21	
3	Sepulveda Boulevard/	06/10/2014	NB	7:45	2,629	4:15	1,591	
3	18th Street	00/10/2014	SB	7.43	1,034	4.13	2,354	
	18th Street		EB		1,034		2,334	
			WB		55		72	
			11.15		33		72	
4	Cedar Avenue/	06/10/2014	NB	8:00	126	5:00	93	
	Marine Avenue		SB		174		576	
			EB		456		612	
			WB		951		644	
5	Cedar Avenue/	06/10/2014	NB	7:45	71	4:15	74	
	22nd Street		SB		104		125	
			EB		30		60	
			WB		49		28	
6	Meadows Avenue/	06/10/2014	NB	7:45	264	5:00	60	
	Marine Avenue		SB		25		15	
			EB		567		848	
			WB		740		705	

[1] Counts conducted by City Traffic Counters

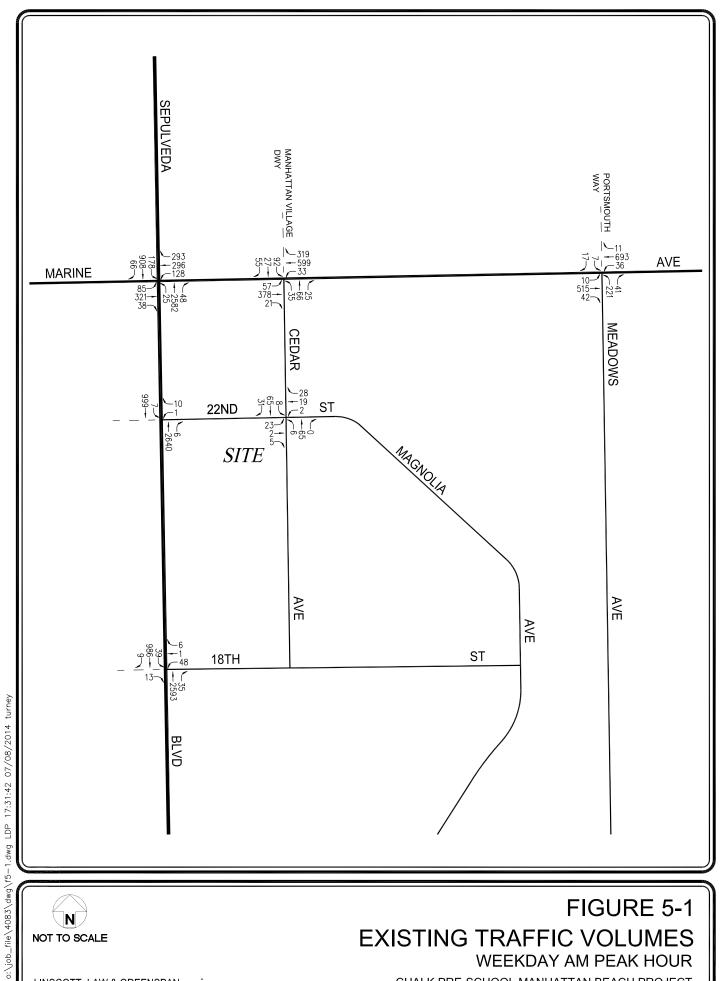




FIGURE 5-1 **EXISTING TRAFFIC VOLUMES** WEEKDAY AM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

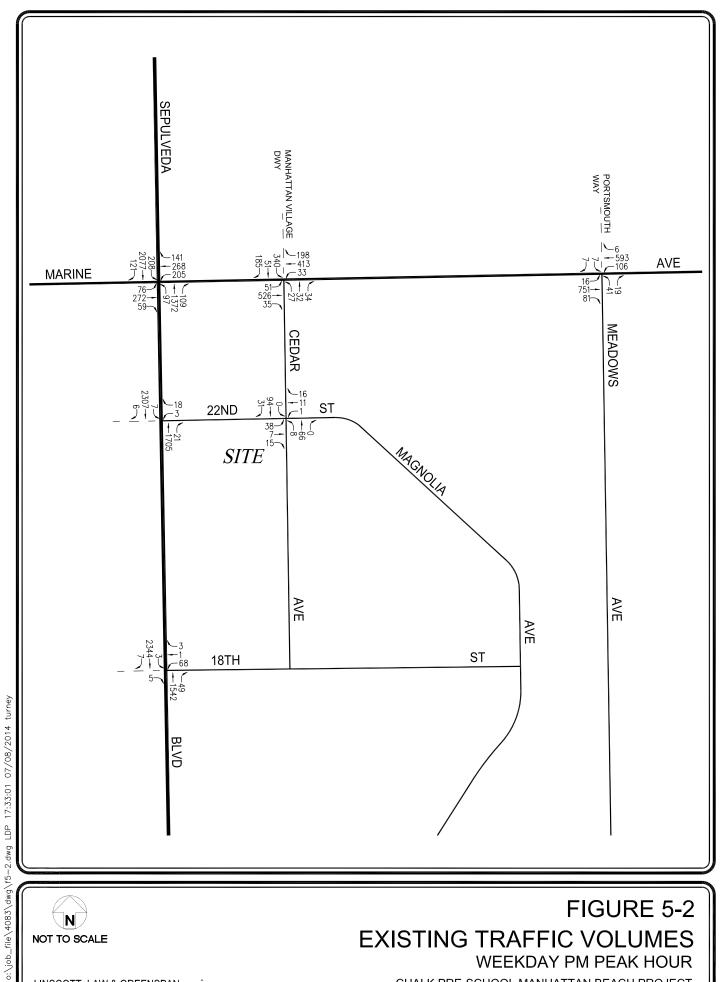




FIGURE 5-2 **EXISTING TRAFFIC VOLUMES** WEEKDAY PM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

6.0 CUMULATIVE DEVELOPMENT PROJECTS

The forecast of future pre-project conditions was prepared in accordance to procedures outlined in Section 15130 of the CEQA Guidelines. Specifically, the CEQA Guidelines provide two options for developing the future traffic volume forecast:

- "(A) A list of past, present, and probable future projects producing related or cumulative impacts, including, if necessary, those projects outside the control of the [lead] agency, or
- (B) A summary of projections contained in an adopted local, regional or statewide plan, or related planning document, that describes or evaluates conditions contributing to the cumulative effect. Such plans may include: a general plan, regional transportation plan, or plans for the reduction of greenhouse gas emissions. A summary of projections may also be contained in an adopted or certified prior environmental document for such a plan. Such projections may be supplemented with additional information such as a regional modeling program. Any such document shall be referenced and made available to the public at a location specified by the lead agency."

Accordingly, the traffic analysis provides a highly conservative estimate of future pre-project traffic volumes as it incorporates both the "A" and "B" options outlined in CEQA Guidelines for purposes of developing the forecast.

6.1 Cumulative Growth

A forecast of on-street traffic conditions prior to occupancy of the proposed project was prepared by incorporating the potential trips associated with other known development projects (related projects) in the area. With this information, the potential impact of the proposed project can be evaluated within the context of the cumulative impact of all ongoing development. The related projects research was based on information on file at the Cities of Manhattan Beach, El Segundo and Hermosa Beach. The list of related projects in the project site area and a brief description for each of the ten (10) related projects is presented in *Table 6–1*. The location of the related projects is shown in *Figure 6–1*.

Traffic volumes expected to be generated by the related projects were calculated using rates provided in the Institute of Transportation Engineers' (ITE) *Trip Generation Manual*⁴. The related projects' respective traffic generation for the weekday AM and PM peak hours, as well as on a daily basis for a typical weekday, is summarized in *Table 6-1*. As shown in *Table 6-1*, the related projects are expected to generate a combined total of 18,310 daily trips during a typical weekday, 1,209 trips (858 inbound trips and 351 outbound trips) during the weekday AM peak hour, and 1,567 trips (665 inbound trips and 902 outbound trips) during the weekday PM peak hour. The anticipated

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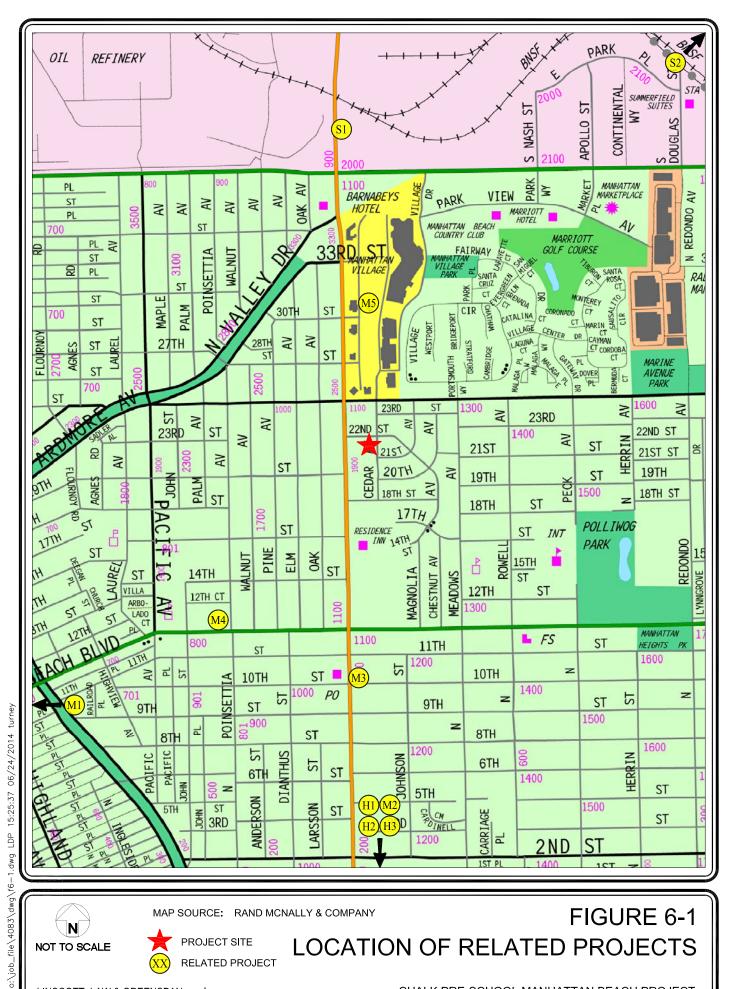
⁴ Institute of Transportation Engineers *Trip Generation Manual*, 9th Edition, 2012, Washington, D.C..

					PROTECT	DAILY	AMP	AM PEAK HOUR	MIR	PM	PM PEAK HOUR	ITR
MAP	PROJECT	PROJECT NAME/NUMBER	LAND USE DATA	ATA	_	TRIP ENDS [2]	VO	VOLUMES [2]	[2]	λ	VOLUMES [2]	[2]
NO.	STATUS	ADDRESS/LOCATION	LAND-USE	SIZE	SOURCE	VOLUMES	N	OUT	TOTAL	N	\mathbf{OUT}	TOTAL
			City of N	City of Manhattan Beach								
M1	Approved	213 Manhattan Beach Boulevard	Retail	3,371 GLSF	[3]	144	2 -		ε,	9 -	7	13
			Office	5,075 GSF	4	34	4	-	n	_	4	n
M2	Approved	1133 Artesia Boulevard	Grocery Store	12,000 GSF	[5]	1,227	25	16	41	28	99	114
M3	Proposed	1000 North Sepulveda Boulevard	Medical Office	23,050 GSF	[9]	833	43	12	55	23	59	82
			Pharmacy Coffee Shon	665 GSF 1715 GSF	E ≅	09 1	1 95	1 6	186	35	35	9
			(Less Existing Restaurant)	(5,400) GSF	<u> </u>	(687)	(32)	(26)	(58)	(32)	(21)	(53)
M4	Approved	865 Manhattan Beach Boulevard	Office	15,000 GSF	[4]	165	20	3	23	4	18	22
M5	Approved	Manhattan Village Shopping Center 3200-3600 North Sepulveda Boulevard	Shopping Center	697,000 GLSF	[10]	463	-	ю	4	26	(42)	(16)
			City	City of El Segundo								
S1	Under	820-850 South Sepulveda Boulevard	Shopping Center Restaurant Office	71,343 GLSF 25,627 GSF 27,338 GSF	[5]	3,046 3,258 302	42 152 38	26 125 5	68 277 43	127 151 7	138 101 34	265 252 41
S2	Under Construction	2355-2383 Utah Avenue	Office	203,591 GSF	[4]	2,246	280	38	318	52	251	303
			City of	City of Hermosa Beach								
HI	Proposed	2101 Pacific Coast Highway	Office	10,124 GSF	[4]	112	14	2	16	8	12	15
HZ	Proposed	Skechers Hermosa Beach 2851-2901, 3001 East Pacific Coast Highway	Corporate Headquarters	200 Employees	[11]	497	83	-1	84	8	31	34
Н3	Proposed	2420 Pacific Coast Highway	Retail Restaurant Office	100,000 GLSF 3,000 GSF 9,000 GSF	[6]	4,270 381 99	60 18 12	36 14 2	96 32 14	178 18	193 12 11	371 30 13
TOTAL	ı					18,310	858	351	1,209	999	902	1,567

[1] Source: City of Manhattan Beach Community Development Department, City of El Segundo Planning Division, and City of Hermosa Beach Community Development Department. Trip generation for the related projects are based on ITE "Trip Generation Manual", 9th Edition, 2012 (as referenced in the Project Data Source column).

^[2] Trips are one-way traffic movements, entering or leaving.
[3] ITE Land Use Code 820 (Shopping Center) trip generation average rates.
[4] ITE Land Use Code 710 (Office Building) trip generation average rates.
[5] ITE Land Use Code 850 (Supermarket) trip generation average rates.
[6] ITE Land Use Code 720 (Medical-Denal Office Building) trip generation average rates.
[7] ITE Land Use Code 880 (Pharmacy/Drugstore without Drive-Through Window) trip generation average rates.
[8] ITE Land Use Code 936 (Coffee/Donut Shop without Drive-Through Window) trip generation average rates.
[8] ITE Land Use Code 936 (Coffee/Donut Shop without Drive-Through Window) trip generation average rates. 10% of the daily trips.

^[9] ITE Land Use Code 932 (High-Tumover (Sit-Down) Restaurant) trip generation average rates.
[10] Source: "Traffic Study for Manhattan Village Shopping Center", Components I + II + III Total New Trips, prepared by Gibson Transportation Consulting, Inc., May 2012.
[11] Source: "Skechers Hermosa Beach Project Draft Traffic Impact Study", LLG Engineers, June 2014.





MAP SOURCE: RAND MCNALLY & COMPANY

FIGURE 6-1

PROJECT SITE RELATED PROJECT LOCATION OF RELATED PROJECTS

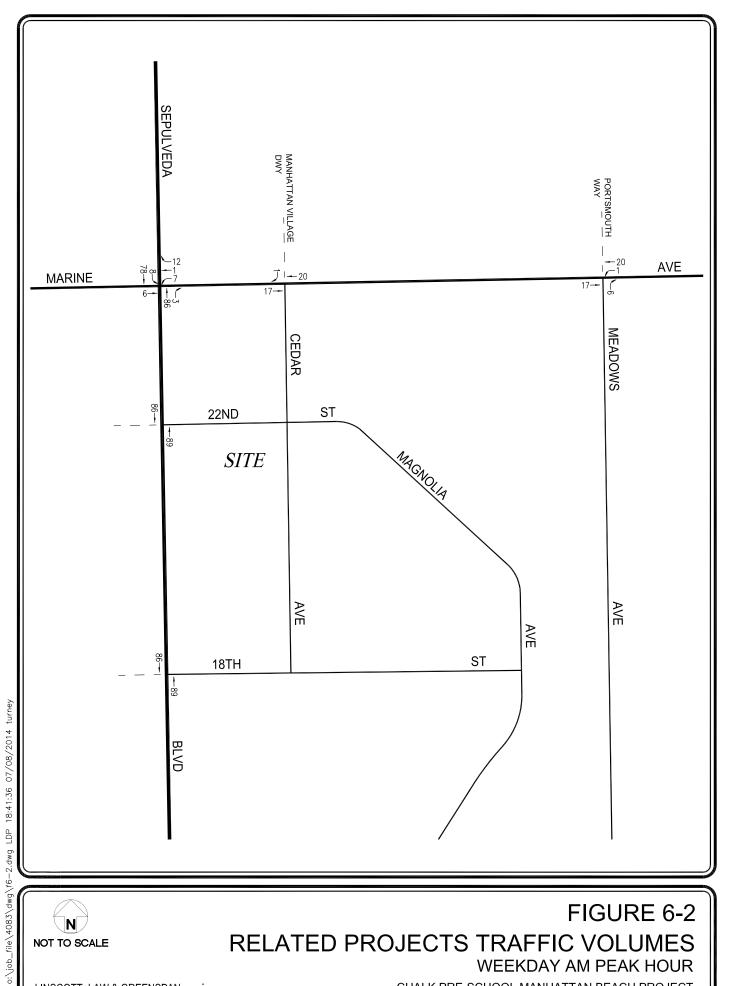
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distribution of the related projects traffic volumes to the study intersections during the weekday AM and PM peak hours is displayed in *Figures 6-2* and *6-3*, respectively.

6.2 Ambient Traffic Growth

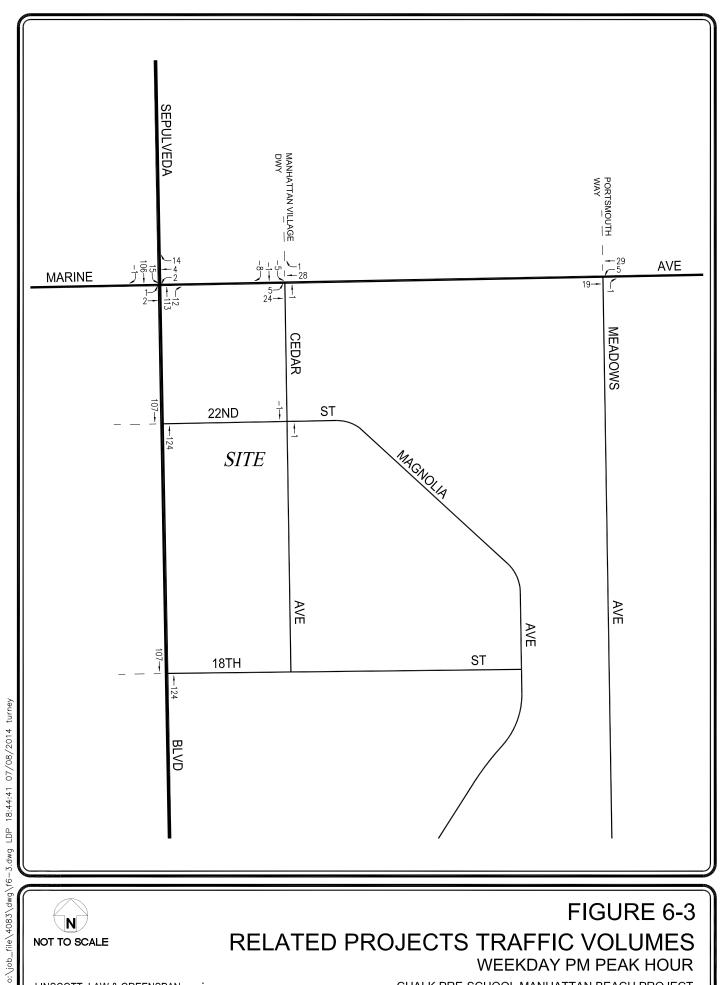
Horizon year, background traffic growth estimates also have been calculated by using an ambient traffic growth factor. The ambient traffic growth factor is intended to include unknown related projects in the study area, as well as account for typical growth in traffic volumes due to the development of projects outside the study area. The future growth in traffic volumes has been calculated at one percent (1.0%) per year. The ambient growth factor was based on review of the background traffic growth estimates for the South Bay/LAX area (RSA 18) published in the 2010 Congestion Management Program for Los Angeles County, which indicate that existing traffic volumes would be expected to increase at an annual rate of less than one percent (approximately 0.26% per year) between years 2010 and 2020. However, a one percent (1.0%) ambient traffic growth factor has been employed in this analysis in order to provide a conservative, worst case forecast of future traffic volumes in the area. Application of the ambient traffic growth factor to existing Year 2014 traffic volumes results in a one percent (1.0%) increase in existing traffic volumes to horizon Year 2015. Further, it is noted that the CMP manual's traffic growth rate is intended to anticipate future traffic generated by development projects in the project vicinity. Thus, the inclusion in this traffic analysis of both a forecast of traffic generated by known related projects plus the use of an ambient growth traffic factor based on CMP traffic model data results in a conservative estimate of future traffic volumes at the study intersections.



N NOT TO SCALE

FIGURE 6-2 RELATED PROJECTS TRAFFIC VOLUMES WEEKDAY AM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT



N NOT TO SCALE

FIGURE 6-3 RELATED PROJECTS TRAFFIC VOLUMES

WEEKDAY PM PEAK HOUR

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CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

7.0 Traffic Forecasting Methodology

In order to estimate the traffic impact characteristics of the Chalk Pre-School Manhattan Beach project, a multi-step process has been utilized. The first step is trip generation, which estimates the total arriving and departing traffic volumes on a peak hour and daily basis. The traffic generation potential is forecast by applying the appropriate vehicle trip generation equations or rates to the project development tabulation.

The second step of the forecasting process is trip distribution, which identifies the origins and destinations of inbound and outbound project traffic volumes. These origins and destinations are typically based on demographics and existing/anticipated travel patterns in the study area.

The third step is traffic assignment, which involves the allocation of project traffic to study area streets and intersections. Traffic assignment is typically based on minimization of travel time, which may or may not involve the shortest route, depending on prevailing operating conditions and travel speeds. Traffic distribution patterns are indicated by general percentage orientation, while traffic assignment allocates specific volume forecasts to individual roadway links and intersection turning movements throughout the study area.

With the forecasting process complete and project traffic assignments developed, the impact of the proposed project is isolated by comparing operational (i.e., Level of Service) conditions at the selected key intersections using existing and expected future traffic volumes with and without forecast project traffic. The need for site-specific and/or cumulative local area traffic improvements can then be evaluated and the significance of the project's impacts identified.

7.1 Project Trip Generation

Traffic generation is expressed in vehicle trip ends, defined as one-way vehicular movements, either entering or exiting the generating land use. Trip generation rates provided in the ITE *Trip Generation Manual*, 9th Edition publication were utilized to forecast project-related trips. The ITE document contains trip rates for a variety of land uses which have been derived based on traffic counts conducted at existing sites throughout California and the United States (i.e., trip rates for the day care center land use category are based on traffic counts conducted at existing day care centers). Trip generation forecasts for the proposed land use and existing uses to be removed are summarized in the following paragraphs.

7.1.1 Proposed Project Trip Generation

Traffic volumes expected to be generated by the proposed project were based upon rates per number of students. Specifically, the daily, AM and PM peak hour traffic volumes expected to be generated by the proposed project were forecast based on ITE Land Use Code 565 (Day Care Center) trip generation average rates. As mentioned previously, while it is recognized that there can be many instances where not all 119 pre-school students are present, all 119 students have been assumed to be on-site and included in the project trip generation forecasts.

The traffic generation forecast for the proposed project is summarized in *Table 7-1*. As presented in *Table 7-1*, the proposed project is expected to generate 95 vehicle trips (50 inbound trips and 45 outbound trips) during the weekday AM peak hour. During the weekday PM peak hour, the proposed project is expected to generate 96 vehicle trips (45 inbound trips and 51 outbound trips). Over a 24-hour weekday period, the proposed project is forecast to generate 522 daily trip ends during a typical weekday (261 inbound trips and 261 outbound trips).

7.1.2 Existing Trip Generation

Traffic volumes generated by the existing site uses also were forecast for the weekday AM and PM peak hours, and over a 24-hour weekday period, using trip generation rates in the ITE *Trip Generation Manual* publication. Specifically, the daily, AM and PM peak hour traffic volumes expected to be generated by the existing uses were forecast based on ITE Land Use Code 820 (Shopping Center) trip generation average rates.

Table 7-1 also provides a summary of the existing use trip generation (i.e., to be applied as a credit/reduction). The existing project site is determined to generate an average of 8 vehicle trips during the weekday AM peak hour (i.e., 5 inbound and 3 outbound trips) and 31 vehicle trips during the PM peak hour (i.e., 15 inbound and 16 outbound trips).

7.1.3 Project Trip Generation Summary

The traffic generation forecast for the proposed project is summarized in *Table 7-1*. The trip generation forecast for the proposed project was submitted for review and approval by City staff. As presented in *Table 7-1*, the proposed project is expected to generate a net increase of 87 vehicle trips (45 inbound trips and 42 outbound trips) during the weekday AM peak hour. During the weekday PM peak hour, the proposed project is expected to generate a net increase of 65 vehicle trips (30 inbound trips and 35 outbound trips). Over a 24-hour period, the proposed project is forecast to generate a net increase of 170 daily trip ends during a typical weekday (85 inbound trips and 85 outbound trips).

7.2 Project Trip Distribution and Assignment

Project traffic volumes both entering and exiting the site have been distributed and assigned to the adjacent street system based on the following considerations:

- The site's proximity to major traffic corridors (i.e., Sepulveda Boulevard, Marine Avenue etc.);
- Expected localized traffic flow patterns based on adjacent roadway channelization and presence of traffic signals;
- Existing site parcel access ingress/egress schemes;
- Ingress/egress scheme planned for the proposed project; and

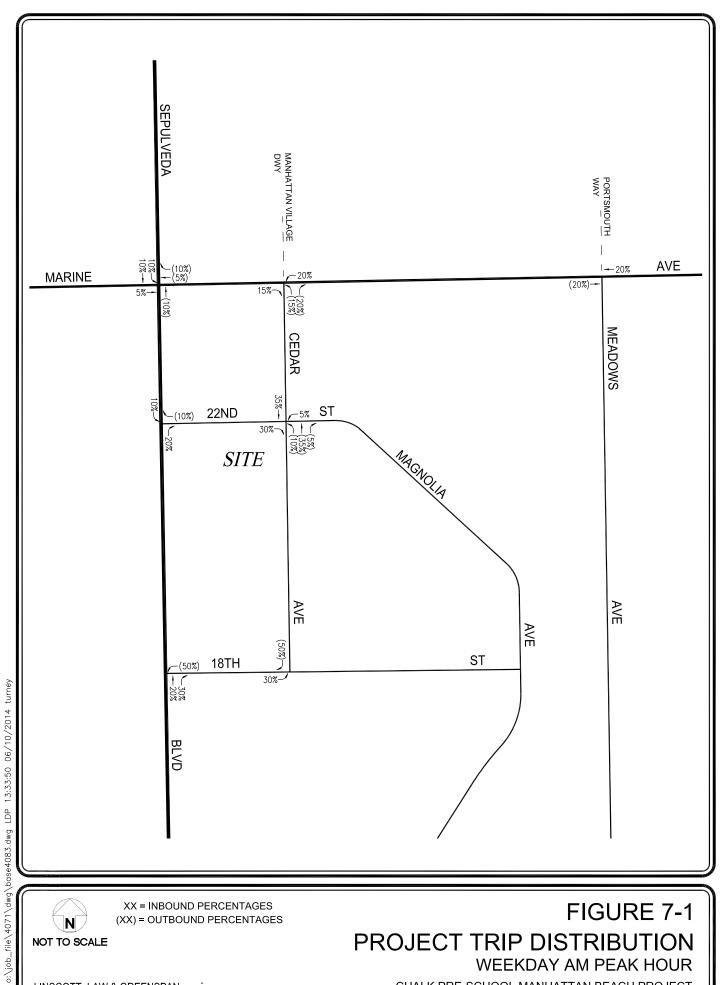
Table 7-1 PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]	AM PEAK HOUR VOLUMES [2]			PM PEAK HOUR VOLUMES [2]			
LAND USE SIZE		VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL	
Proposed Project Day Care Center [3]	119 Students	522	50	45	95	45	51	96	
Subtotal Proposed		522	50	45	95	45	51	96	
Less Existing Retail [4]	(8,232) GLSF	(352)	(5)	(3)	(8)	(15)	(16)	(31)	
Subtotal Existing	<u> </u>	(352)	(5)	(3)	(8)	(15)	(16)	(31)	
NET INCREASE		170	45	42	87	30	35	65	

- [1] Source: ITE "Trip Generation Manual", 9th Edition, 2012.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 565 (Day Care Center) trip generation average rates.
 - Daily Trip Rate: 4.38 trips/student; 50% inbound/50% outbound
 - AM Peak Hour Trip Rate: 0.80 trips/student; 53% inbound/47% outbound
 - PM Peak Hour Trip Rate: 0.81 trips/student; 47% inbound/53% outbound
- [4] ITE Land Use Code 820 (Shopping Center) trip generation average rates.
 - Daily Trip Rate: $42.7\ trips/1,\!000\ SF$ of floor area; 50% inbound/50% outbound
 - AM Peak Hour Trip Rate: 0.96 trips/1,000 SF of floor area; 62% inbound/38% outbound
 - PM Peak Hour Trip Rate: 3.71 trips/1,000 SF of floor area; 48% inbound/52% outbound

• Input from the City's consultant Traffic Engineer.

The project traffic distribution percentages at the study intersections for the weekday AM and PM peak hours are illustrated in *Figures 7-1* and *7-2*, respectively. The existing uses trip distribution percentages at the study intersections for the weekday AM and PM peak hours are contained in *Appendix B* (refer to *Appendix Figures B-1* and *B-2*). The forecast net new project traffic volumes at the study intersections for the weekday AM and PM peak hours are displayed in *Figures 7-3* and *7-4*, respectively. The net new project traffic volume assignments presented in *Figures 7-3* and *7-4* reflect the project traffic distribution characteristics shown in *Figures 7-1* and *7-2* as well as the existing traffic distribution shown in *Appendix B* and the project and existing uses traffic generation forecast presented in *Table 7-1*.



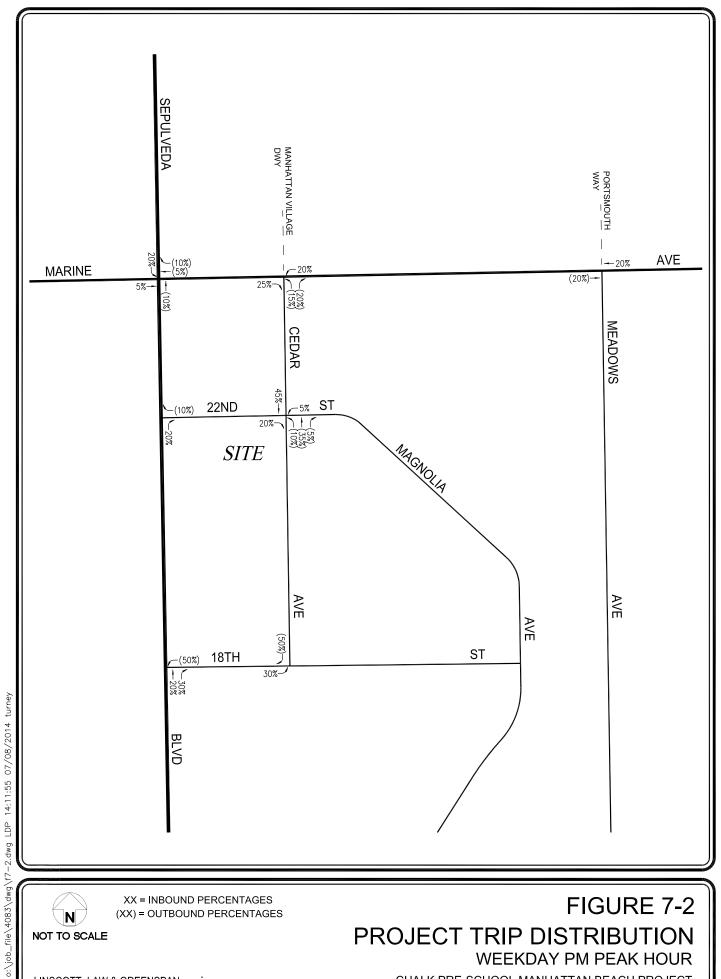


XX = INBOUND PERCENTAGES (XX) = OUTBOUND PERCENTAGES

NOT TO SCALE

FIGURE 7-1 PROJECT TRIP DISTRIBUTION WEEKDAY AM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT





XX = INBOUND PERCENTAGES (XX) = OUTBOUND PERCENTAGES

FIGURE 7-2 PROJECT TRIP DISTRIBUTION WEEKDAY PM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

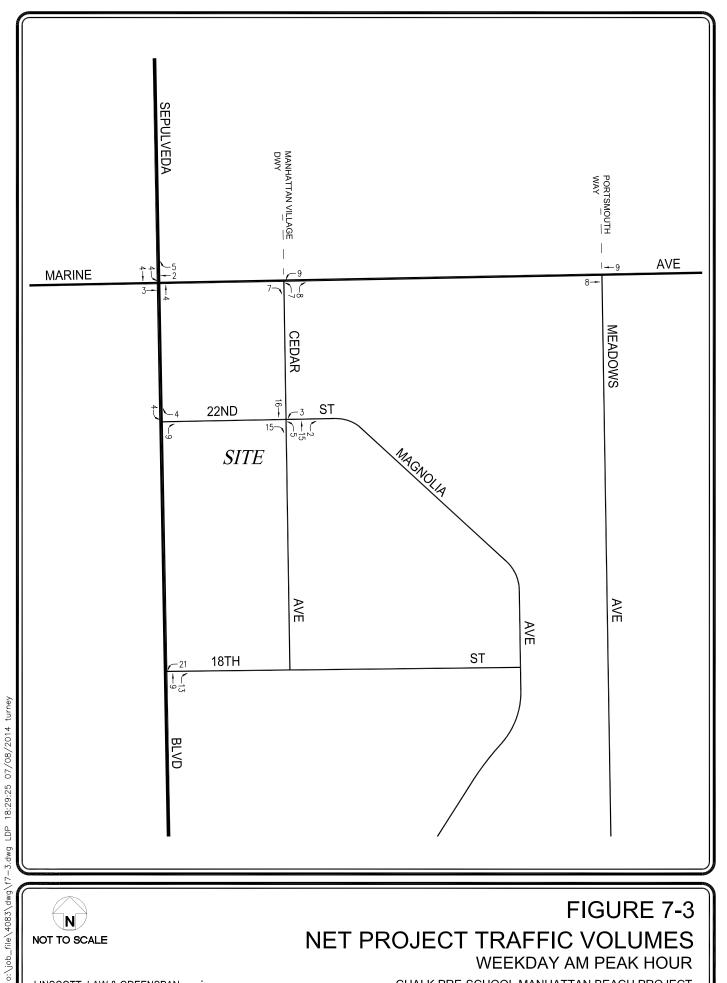




FIGURE 7-3 NET PROJECT TRAFFIC VOLUMES WEEKDAY AM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

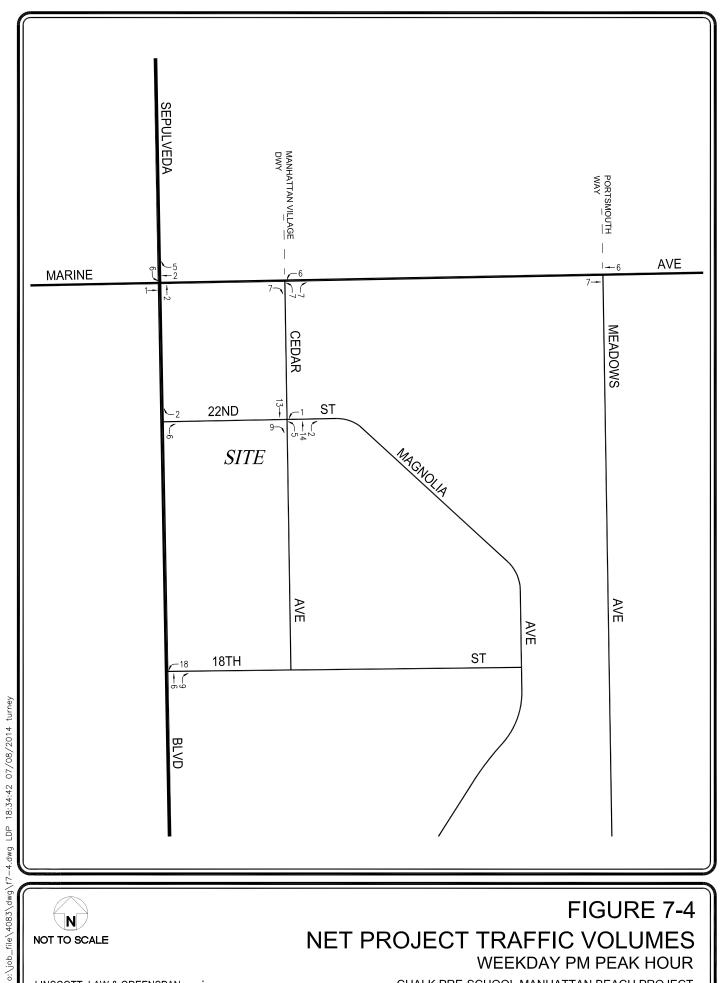




FIGURE 7-4 NET PROJECT TRAFFIC VOLUMES WEEKDAY PM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

8.0 TRAFFIC IMPACT ANALYSIS METHODOLOGY

8.1 Study Intersections

8.1.1 Intersection Analysis Methodology

The six study intersections were evaluated using the Intersection Capacity Utilization (ICU) method of analysis which determines Volume-to-Capacity (v/c) ratios on a critical lane basis. The overall intersection v/c ratio is subsequently assigned a Level of Service (LOS) value to describe intersection operations. Level of Service varies from LOS A (free flow) to LOS F (jammed condition). A description of the ICU method and corresponding Level of Service is provided in *Appendix C*.

The AM and PM peak hour operating conditions for the study intersections were evaluated using the ICU methodology for signalized intersections and the methodology outlined in Chapter 19 of the HCM2010 Highway Capacity Manual (HCM2010) for stop-controlled intersections. This methodology estimates the average control delay for each of the subject movements and determines the level of service for each constrained movement. Average control delay for any particular movement is a function of the capacity of the approach and the degree of saturation. The overall average control delay is measured in seconds per vehicle. A description of the HCM method and corresponding Level of Service is also provided in Appendix C.

8.1.2 City of Manhattan Beach Intersection Impact Criteria and Thresholds

The relative impact of the added project traffic volumes generated by the proposed Chalk Pre-School Manhattan Beach project during the AM and PM peak hours was evaluated based on analysis of future operating conditions at the six key study intersections, without, then with, the proposed project. The significance of the potential project impacts at each key intersection was then evaluated using the traffic impact criteria employed in previous analyses for projects in the City of Manhattan Beach. Per the City of Manhattan Beach policy, the significance of the potential impacts of project generated traffic at each study intersection was identified using criteria consistent with the 2010 Congestion Management Program for Los Angeles County, County of Los Angeles Metropolitan Transportation Authority. A significant transportation impact is determined based on a change in the calculated v/c ratio of two percent (0.02) or more due to project-related traffic for an intersection operating at LOS F or worse (v/c > 1.00).

8.2 Intersection Capacity Utilization Methods of Analysis

8.2.1 Signalized Intersections

In conformance with the City of Manhattan Beach and Los Angeles County Congestion Management Program requirements, existing weekday AM and PM peak hour operating conditions for the signalized study intersections were evaluated using the ICU method. The ICU methodology is intended for signalized intersection analyses and estimates the v/c relationship for an intersection based on the individual v/c ratios for key conflicting traffic movements.

The ICU numerical value represents the percent signal (green) time, and thus capacity, required by existing and/or future traffic. It should be noted that the ICU methodology assumes uniform traffic distribution per intersection approach lane and optimal signal timing. The ICU value translates to a LOS estimate, which is a relative measure of the intersection performance. The six qualitative categories of Level of Service have been defined along with the corresponding ICU value range and are shown in *Table 8-1*. A description of the ICU method and corresponding Level of Service is provided in *Appendix C*.

TABLE 8-1
LEVEL OF SERVICE CRITERIA FOR SIGNALIZED INTERSECTIONS

Level of Service (LOS)	Intersection Capacity Utilization Value (V/C)	Level of Service Description
A	≤ 0.600	EXCELLENT. No vehicle waits longer than one red light, and no approach phase is fully used.
В	0.601 - 0.700	VERY GOOD. An occasional approach phase is fully utilized; many drivers begin to feel somewhat restricted within groups of vehicles.
С	0.701 - 0.800	GOOD. Occasionally drivers may have to wait through more than one red light; backups may develop behind turning vehicles.
D	0.801 - 0.900	FAIR. Delays may be substantial during portions of the rush hours, but enough lower volume periods occur to permit clearing of developing lines, preventing excessive backups.
E	0.901 – 1.000	POOR. Represents the most vehicles intersection approaches can accommodate; may be long lines of waiting vehicles through several signal cycles.
F	> 1.000	FAILURE. Backups from nearby locations or on cross streets may restrict or prevent movement of vehicles out of the intersection approaches. Potentially very long delays with continuously increasing queue lengths.

Pursuant to Los Angeles County CMP requirements, the ICU calculations use a lane capacity of 1,600 vehicles per hour (vph) for left-turn, through, and right-turn lanes, and a dual left-turn capacity of 2,880 vph. Additionally, a clearance adjustment factor of 0.10 was added to each Level of Service (LOS) calculation. The ICU value is the sum of the critical volume to capacity ratios at an intersection; it is not intended to be indicative of the LOS of each of the individual turning movements.

8.2.2 Unsignalized Intersections

The *Highway Capacity Manual 2010* (HCM2010) methodology outlined in Chapter 19 for unsignalized/two-way stop-controlled study intersections was utilized for the analysis of the unsignalized intersections. The TWSC methodology estimates the average control delay for each

minor-street movement (or shared movement) as well as major-street left-turns and determines the LOS for each constrained movement. It should be noted that LOS is not defined for the overall TWSC intersection because major-street movements with no delays typically result in a weighted average delay that is extremely low. Average control delay for any particular movement is a function of the capacity of the approach and the degree of saturation. The average control delay is measured in seconds per vehicle, and includes delay due to deceleration to a stop at the back of the queue from free-flow speed, move-up time within the queue, stopped delay at the front of the queue, and delay due to acceleration back to free-flow speed. A description of the HCM method and corresponding Level of Service is also provided in *Appendix C*. The six qualitative categories of Level of Service have been defined along with the corresponding HCM control delay value range, as shown in *Table 8-2*.

Table 8-2
Level of Service Criteria For Unsignalized Intersections

Level of Service (LOS)	Highway Capacity Manual Delay Value (sec/veh)	Level of Service Description
A	≤ 10.0	Little or no delay
В	$> 10.0 \text{ and} \le 15.0$	Short traffic delays
С	$> 15.0 \text{ and} \le 25.0$	Average traffic delays
D	> 25.0 and ≤ 35.0	Long traffic delays
E	$> 35.0 \text{ and} \le 50.0$	Very long traffic delays
F	> 50.0	Severe congestion

8.3 Traffic Impact Analysis Scenarios

Pursuant to City of Manhattan Beach and Los Angeles County Congestion Management Program requirements, Level of Service calculations have been prepared for the following scenarios for the study intersections:

- (a) Existing (Year 2014) conditions.
- (b) Condition (a) with completion and occupancy of the project.
- (c) Condition (b) with implementation of project mitigation measures where necessary.
- (d) Condition (a) plus one percent (1.0%) annual ambient traffic growth through Year 2015 and with completion and occupancy of the related projects (i.e., future Year 2015 pre-project conditions)
- (e) Condition (d) with completion and occupancy of the project (i.e., future Year 2015 with project conditions).
- (f) Condition (e) with implementation of project mitigation measures where necessary.

The traffic volumes for each new condition were added to the volumes in the prior condition to determine the change in capacity utilization at the study intersections.

8.4 Study Street Segments

8.4.1 Street Segment Analysis Methodology

Based on coordination with City of Manhattan Beach staff, street segment level of service analyses were prepared for two roadway segments located in the immediate project vicinity. Automatic 24-hour machine traffic counts were conducted at the two street segment locations in June 2014. The weekday AM peak hour and PM peak hour volumes were then determined based on the automatic 24-hour machine traffic counts. Copies of the 24-hour machine traffic counts are contained in *Appendix A*.

8.4.2 Street Segment Impact Criteria and Thresholds

The City of Manhattan Beach does not have specific impact criteria and thresholds applicable to roadway segments. Pursuant to coordination with the City contract Traffic Engineer, the significance of the potential impacts of project-generated net new traffic at the study street segments was identified using the two-lane roadway criteria set forth in the Los Angeles County Department of Public Works' *Traffic Impact Analysis Report Guidelines* document. Total capacity (PCPH) is based on existing roadway directional split pursuant to the County's traffic study guidelines. However, please note that the PCPH capacity used in this analysis is one-half (i.e., 50%) of the County's identified capacities in order to better reflect the type of roadways, adjoining land uses, and other local roadway network characteristics (e.g., residential driveways, on-street parking, etc.) in order to provide a conservative analysis. Accordingly, a transportation impact on a roadway shall be deemed significant based on a percentage increase in passenger cars per hour (PCPH) by the project as shown in *Table 8-3*.

	TABLE 8-3 RECOMMENDED CITY OF MANHATTAN BEACH ROADWAY SEGMENT IMPACT THRESHOLD CRITERIA											
	Percentage Increase in Passenger Cars Per Hour (PCPH) by Project											
	Total Capacity	Pre-Project LOS										
Directional Split	(PCPH) [a]	С	D	E/F								
50/50	1,400	4	2	1								
60/40	1,325	4	2	1								
70/30	1,250	4	2	1								
80/20	1,150	4	2	1								
90/10	1,050	4	2	1								
100/0	1,000	4	2	1								

[[]a] Total capacity (pcph) based on 50 percent of the values established by Los Angeles County.

9.0 TRAFFIC ANALYSIS

The traffic impact analysis prepared for the study intersections using the ICU and HCM methodology and application of the City of Manhattan Beach significant traffic impact criteria is summarized in *Table 9-1*. The ICU data worksheets for the analyzed intersections are contained in *Appendix C*.

9.1 Study Intersections

9.1.1 Existing Conditions

As indicated in column [1] of *Table 9-1*, four of the six study intersections are presently operating at LOS D or better during the weekday AM and PM peak hours under existing conditions. The following study intersections are currently operating at LOS E or F during the peak hour(s) shown below under existing conditions:

• Int. No. 1: Sepulveda Blvd./Marine Ave. AM Peak Hour: v/c=0.948, LOS E

• Int. No. 2: Sepulveda Blvd./22nd St. AM Peak Hour: 98.0 sec. of delay, LOS F

PM Peak Hour: 57.6 sec. of delay, LOS F

As previously mentioned, the existing traffic volumes at the study intersections during the weekday AM and PM peak hours are displayed in *Figures 5-1* and *5-2*, respectively.

9.1.2 Existing With Project Conditions

As shown in column [2] of *Table 9–1*, application of the City's threshold criteria to the "Existing Plus Project" scenario indicates that the proposed project is not expected to create significant impacts at any of the six study intersections. Incremental, but not significant, impacts are noted at the study intersections. Because there are no significant impacts, no traffic mitigation measures are required or recommended for the study intersections under the "Existing With Project" conditions. The existing with project traffic volumes at the study intersections during the weekday AM and PM peak hours are illustrated in *Figures 9-1* and *9-2*, respectively.

9.1.3 Future Pre-Project Conditions

The future year 2015 pre-project conditions were forecast based on the addition of traffic generated by the related projects, as well as the growth in traffic due to the combined effects of continuing development, intensification of existing developments and other factors (i.e., ambient growth). The v/c ratios and delay values at the study intersections appropriately reflect the addition of traffic generated by the related projects listed in *Table 6-1* and growth in ambient traffic.

As indicated in column [3] of *Table 9-1*, four of the six study intersections are anticipated to operate at LOS D or better during the weekday AM and PM peak hours under future pre-project conditions. The following study intersections are expected to operate at LOS E or F during the peak hour(s) shown below under future pre-project conditions:

Table 9-1 SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE WEEKDAY AM AND PM PEAK HOURS

			[1]			[2]		[3]		[4]				
									YEAR		YEAR			
					VEAD (2014			FUTU		FUTURE WITH			
			YEAR	2014		YEAR 2014 EXISTING PLUS CHANGE			PRE-PRO		PROPOSED		CHANGE	
			EXIST	-	PROJE		DELAY or SIGNIF.		W/ AG & REL. PROJECTS		PROJECT		V/C or SIGNIF.	
		PEAK	V/C or	LOS	V/C or	LOS	V/C	IMPACT	V/C or	LOS	V/C or	LOS	Delay	IMPACT
NO.	INTERSECTION	HOUR	Delay	[b]	Delay	[b]	[(2)-(1)]	[c]	Delay	[b]	Delay	[b]	[(4)-(3)]	[c]
	0 1 1 2 1 1/		0.040	1	0.051		0.002	NO	0.070		0.002		0.004	NO
1	Sepulveda Boulevard/	AM PM	0.948 0.825	E D	0.951	E D	0.003	NO NO	0.978 0.856	E D	0.982	E D	0.004	NO NO
	Marine Avenue	PM	0.825	D	0.825	D	0.000	NO	0.856	D	0.856	D	0.000	NO
	6 1 1 2 1 1/	437	00.0	1	111.4	Б	0.006	NO	125.1	-	122.2	Б	0.007	NO
2	Sepulveda Boulevard/ 22nd Street [a]	AM PM	98.0 57.6	F F	111.4 55.4	F F	0.006 0.001	NO NO	125.1 78.6	F F	133.3 74.4	F F	0.007 0.001	NO NO
	22lid Street [a]	rivi	37.0	Г	33.4	Г	0.001	NO	76.0	Г	74.4	Г	0.001	NO
			0.663		0.669				0.687		0.694			
			0.595		0.596				0.622		0.623			
3	Sepulveda Boulevard/	AM	0.710	С	0.728	С	0.018	NO	0.735	С	0.753	С	0.018	NO
3	18th Street	PM	0.710	В	0.728	В	0.013	NO	0.753	В	0.733	В	0.018	NO
4	Cedar Avenue/	AM	0.471	A	0.481	A	0.010	NO	0.475	A	0.484	A	0.009	NO
	Marine Avenue	PM	0.567	A	0.581	A	0.014	NO	0.576	A	0.591	A	0.015	NO
5	Cedar Avenue/	AM	7.7	A	8.0	A	0.015	NO	7.8	A	8.0	A	0.015	NO
	22nd Street [a]	PM	7.7	A	7.9	A	0.015	NO	7.8	A	7.9	A	0.014	NO
			0.214		0.229				0.215		0.230			
			0.224		0.239				0.225		0.239			
6	Meadows Avenue/	AM	0.496	A	0.498	A	0.002	NO	0.510	A	0.512	A	0.002	NO
	Marine Avenue	PM	0.468	A	0.470	A	0.002	NO	0.481	A	0.484	A	0.003	NO

[a] Unsignalized intersection

[b] Level of Service (LOS) is based on the reported ICU value for signalized intersections and on the delay for unsignalized intersections

[c] According to the City of Manhattan Beach threshold of significance, a transportation impact at an intersection shall be deemed significant in accordance with the following:

Final v/c LOS Project Related Increase in v/c

> 1.000 F equal to or greater than 0.02

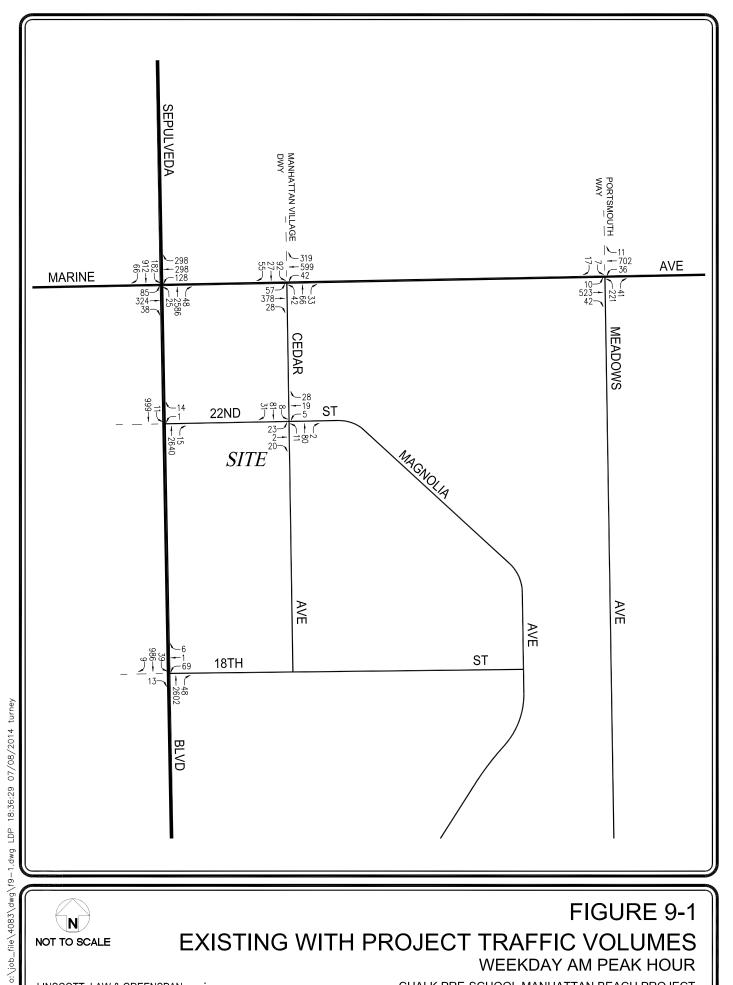


FIGURE 9-1 EXISTING WITH PROJECT TRAFFIC VOLUMES

WEEKDAY AM PEAK HOUR

LINSCOTT, LAW & GREENSPAN, engineers

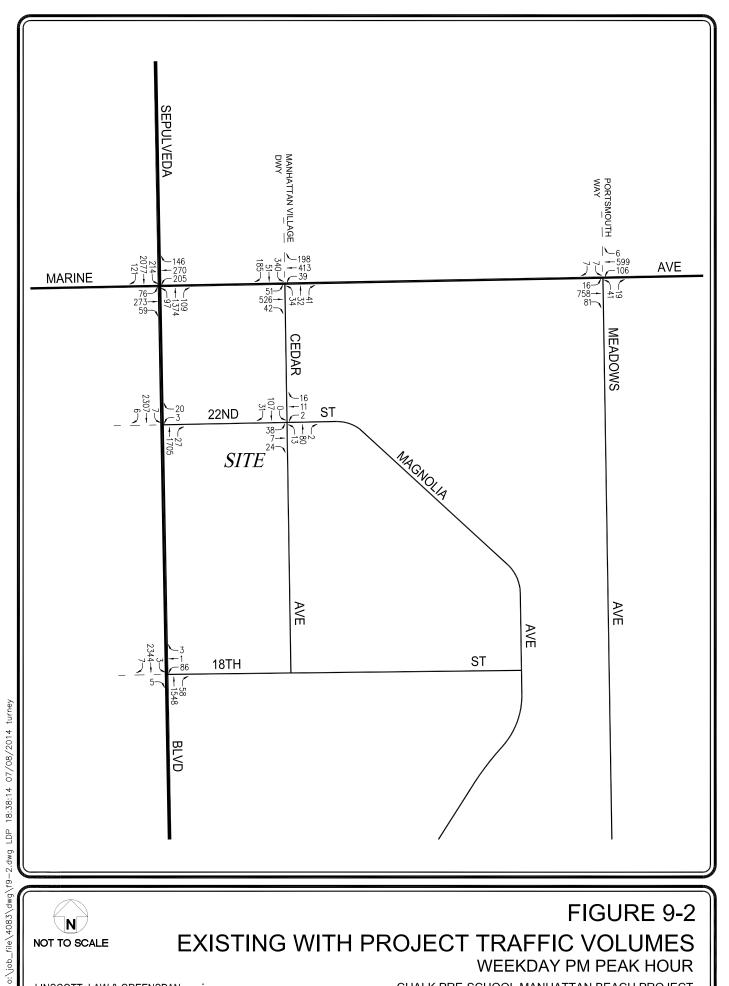


FIGURE 9-2 EXISTING WITH PROJECT TRAFFIC VOLUMES

WEEKDAY PM PEAK HOUR CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

• Int. No. 1: Sepulveda Blvd./Marine Ave. AM Peak Hour: v/c=0.978, LOS E

• Int. No. 2: Sepulveda Blvd./22nd St. AM Peak Hour: 125.1 sec. of delay, LOS F

PM Peak Hour: 78.6 sec. of delay, LOS F

The future pre-project (existing, ambient growth and related projects) traffic volumes at the study intersections during the weekday AM and PM peak hours are presented in *Figures 9-3* and *9-4*, respectively.

9.1.4 Future With Project Conditions

As shown in column [4] of *Table 9-1*, application of the City's threshold criteria to the "With Proposed Project" scenario indicates that the proposed project is not expected to create significant impacts at any of the six study intersections. Incremental, but not significant, impacts are noted at the study intersections. Because there are no significant impacts, no traffic mitigation measures are required or recommended for the study intersections. The future with project (existing, ambient growth, related projects and project) traffic volumes at the study intersections during the weekday AM and PM peak hours are illustrated in *Figures 9-5* and *9-6*, respectively.

9.2 Study Street Segments Analysis

9.2.1 Existing and Existing With Project Conditions

The existing and forecast existing with project traffic volumes at the street segment study locations are summarized in *Table 9-2*. The directional traffic splits for each study street segment based on existing traffic count data as well as the corresponding total peak hour roadway capacities are also displayed. As presented in column [1] of *Table 9-2*, both study street segments are presently operating at LOS A during the weekday AM peak hour and PM peak hour under existing conditions. As shown in column [2] of *Table 9-2*, both study street segments are expected to continue operating at LOS A during the weekday AM peak hour and PM peak hour with the addition of project traffic. Application of the County's two-lane roadway threshold criteria for street segment analysis with capacity reductions, indicates that the proposed project is not anticipated to significantly impact the analyzed street segments. Incremental, but less than significant impacts are noted at the study street segments under existing conditions, as presented in *Table 9-2*. Thus, no mitigation measures are required or recommended.

9.2.2 Future Pre-Project and Future With Project Conditions

The forecast traffic conditions at the analyzed street segments for future pre-project (i.e., existing traffic volumes, ambient traffic growth and related projects traffic volumes) and future with project analysis scenarios are summarized in *Table 9-3*. The directional traffic splits for each study street segment based on existing traffic count data as well as the corresponding total peak hour roadway capacities are also displayed. As presented in column [1] of *Table 9-3*, both study street segments are expected to continue operating at LOS A during the weekday AM peak hour and PM peak hour under the future pre-project conditions. As shown in column [2] of *Table 9-3*, both study street

LINSCOTT, LAW & GREENSPAN, engineers

LLG Ref. 1-14-4083-1 Chalk Pre-School Manhattan Beach Project

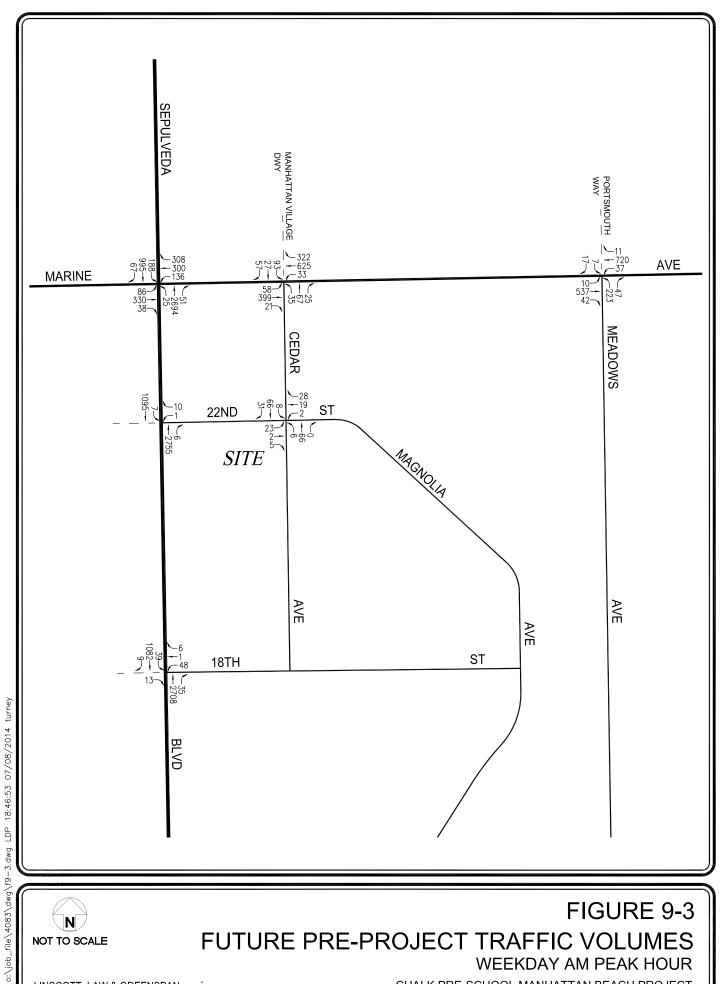


FIGURE 9-3 FUTURE PRE-PROJECT TRAFFIC VOLUMES

WEEKDAY AM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

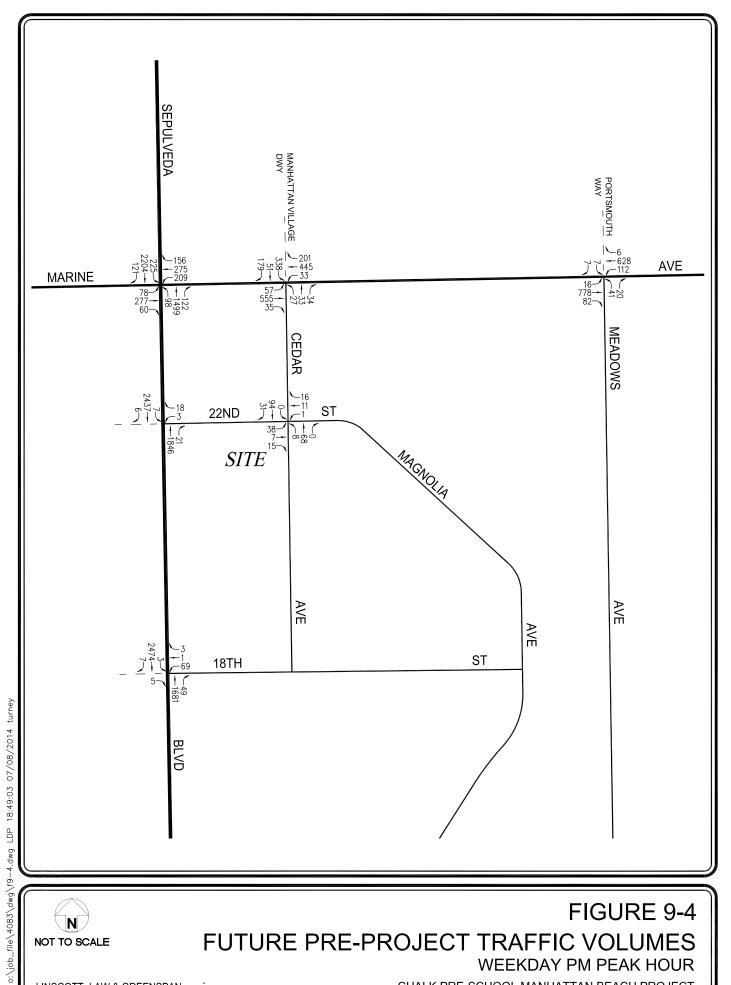


FIGURE 9-4 FUTURE PRE-PROJECT TRAFFIC VOLUMES

WEEKDAY PM PEAK HOUR

LINSCOTT, LAW & GREENSPAN, engineers

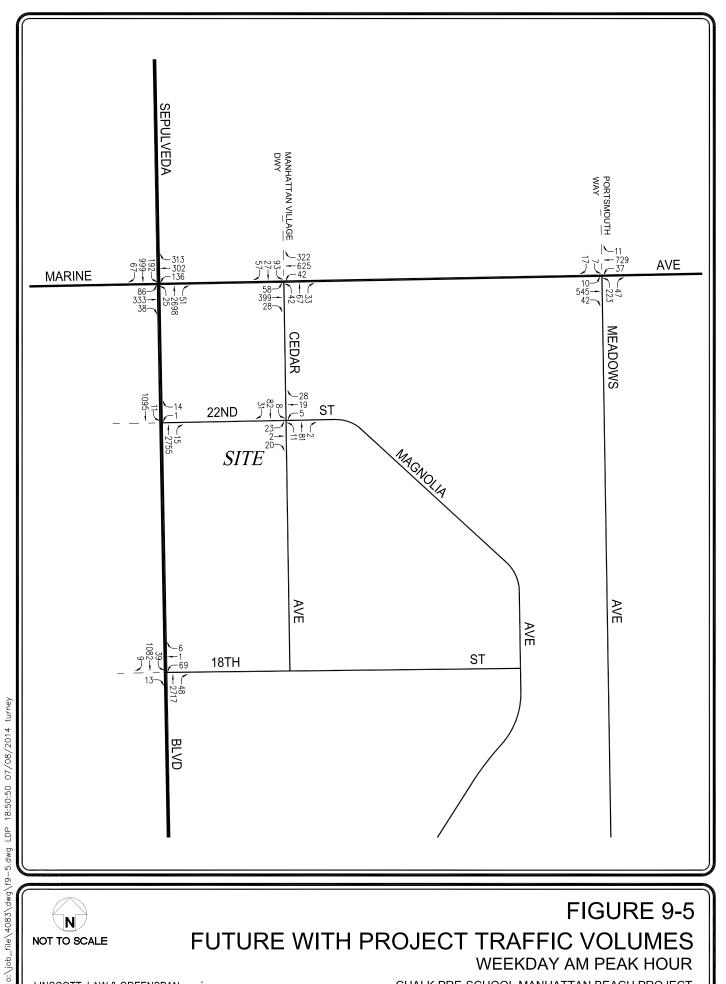


FIGURE 9-5 FUTURE WITH PROJECT TRAFFIC VOLUMES WEEKDAY AM PEAK HOUR

LINSCOTT, LAW & GREENSPAN, engineers

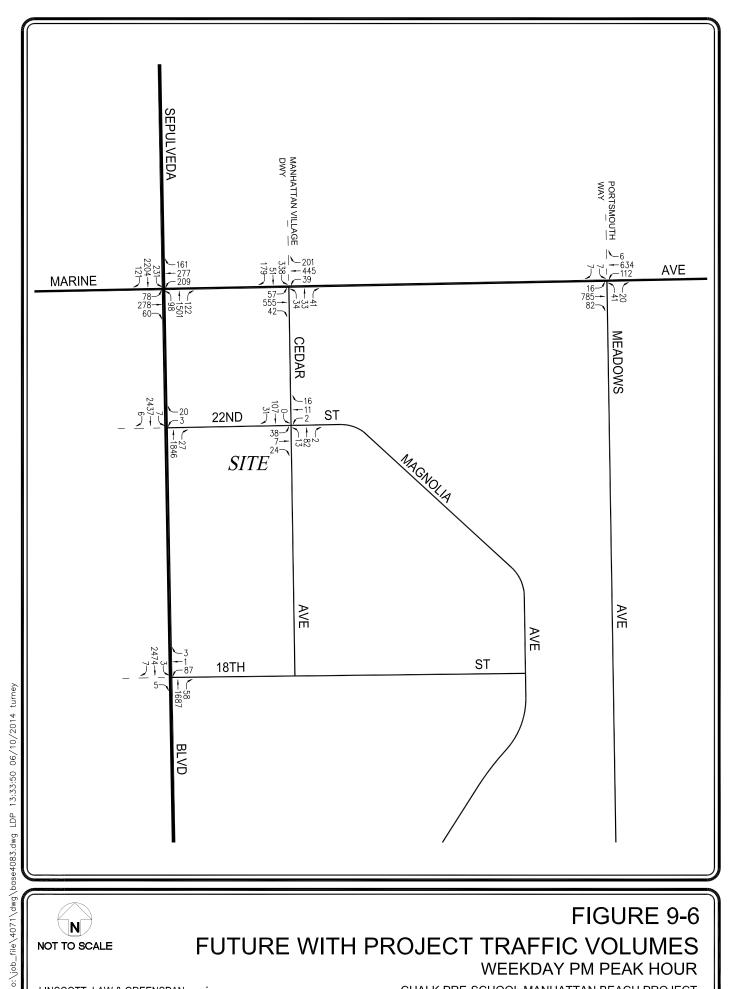


FIGURE 9-6 FUTURE WITH PROJECT TRAFFIC VOLUMES WEEKDAY PM PEAK HOUR

LINSCOTT, LAW & GREENSPAN, engineers

Table 9-2
EXISTING AND EXISTING WITH PROJECT ROADWAY SEGMENT LEVEL OF SERVICE SUMMARY

				TOTAL	(1) EXISTING CONDITIONS			(2) EXISTING WITH PROJECT TRAFFIC CONDITIONS						
NO.	ROADWAY SEGMENT	PEAK HOUR	DIRECT- IONAL SPLIT [a]	CAPACITY (PCPH) [b]	PEAK HOUR VOL [c]	V/C	LOS	PROJ. VOL. [d]	PEAK HOUR VOL [e]	V/C	LOS	PCPH PERCENT INCREASE	SIG. IMPACT YES/NO [f]	
1	22nd Street Between Sepulveda Boulevard and Cedar Avenue	AM PM	60 / 40 70 / 30	1,325 1,250	69 86	0.052 0.069	A A	17 8	86 94	0.065 0.075	A A	24.6% 9.3%	NO NO	
2	Cedar Avenue Between 22nd Street and 21st Avenue	AM PM	70 / 30 60 / 40	1,250 1,325	189 242	0.151 0.183	A A	34 27	223 269	0.178 0.203	A A	18.0% 11.2%	NO NO	

Note: PCPH = Passenger Cars Per Hour

- [a] Directional split of the roadway based on existing traffic count data.
- [b] Total capacity, in passenger cars per hour (PCPH), based on existing roadway directional split per County of Los Angeles Department of Public Works' Traffic Impact Analysis Report Guidelines, January 1, 1997.
- [c] 24-hour machine counts conducted by City Traffic Counters in June 2014.
- [d] Represents net new project trips based on the project trip generation and trip distribution for the proposed project.
- [e] Derived by combining the existing traffic volumes and the proposed project volumes.
- [f] According to the County of Los Angeles Department of Public Works' "Traffic Impact Analysis Report Guidelines", January 1, 1997, Page 6: an impact is considered significant if the project related increase in Passenger Cars Per Hour (PCPH) equals or exceeds the thresholds shown below. It should be noted that the typical LA County two-lane roadway capacities have been reduced by half (50%) for purposes of this roadway segment analysis. Refer to report text for further discussion.

Percentages Increase in PCPH by Project

		Pre-	project L	<u>.OS</u>
Directional Split	Total Capacity (PCPH)	<u>C</u>	D	E/F
50/50	1,400	4	2	1
60/40	1,325	4	2	1
70/30	1,250	4	2	1
80/20	1,150	4	2	1
90/10	1,050	4	2	1
100/0	1.000	4	2	1

Table 9-3
YEAR 2015 CONDITIONS ROADWAY SEGMENT LEVEL OF SERVICE SUMMARY

				TOTAL		(1) TURE (20 E-PROJE							
NO.	ROADWAY SEGMENT	PEAK HOUR	DIRECT- IONAL SPLIT [a]	CAPACITY (PCPH) [b]	PEAK HOUR VOL [c]	V/C	LOS	PROJ. VOL. [d]	PEAK HOUR VOL [e]	V/C	LOS	PCPH PERCENT INCREASE	SIG. IMPACT YES/NO [f]
1	22nd Street Between Sepulveda Boulevard and Cedar Avenue	AM PM	60 / 40 70 / 30	1,325 1,250	70 87	0.053 0.069	A A	17 8	87 95	0.065 0.076	A A	24.4% 9.2%	NO NO
2	Cedar Avenue Between 22nd Street and 21st Avenue	AM PM	70 / 30 60 / 40	1,250 1,325	191 244	0.153 0.184	A A	34 27	225 271	0.180 0.205	A A	17.8% 11.0%	NO NO

Note: PCPH = Passenger Cars Per Hour

- [a] Directional split of the roadway based on existing traffic count data.
- [b] Total capacity, in passenger cars per hour (PCPH), based on existing roadway directional split per County of Los Angeles Department of Public Works' Traffic Impact Analysis Report Guidelines, January 1, 1997.
- [c] 24-hour machine counts conducted by City Traffic Counters in June 2014. An ambient growth rate of 1.0% per year was applied to derive the year 2015 traffic volumes.
- [d] Represents net new project trips based on the project trip generation and trip distribution for the proposed project.
- [e] Derived by combining the year 2015 traffic volumes and the proposed project volumes.
- [f] According to the County of Los Angeles Department of Public Works' "Traffic Impact Analysis Report Guidelines", January 1, 1997, Page 6: an impact is considered significant if the project related increase in Passenger Cars Per Hour (PCPH) equals or exceeds the thresholds shown below. It should be noted that the typical LA County two-lane roadway capacities have been reduced by half (50%) for purposes of this roadway segment analysis. Refer to report text for further discussion.

Percentages Increase in PCPH by Project

		Pre	-project L	OS
Directional Split	Total Capacity (PCPH)	<u>C</u>	D	E/F
50/50	1,400	4	2	1
60/40	1,325	4	2	1
70/30	1,250	4	2	1
80/20	1,150	4	2	1
90/10	1,050	4	2	1
100/0	1,000	4	2	1

segments are expected to continue operating at LOS A during the weekday AM peak hour and PM peak hour in the future conditions with the addition of project traffic. Application of the County's two-lane roadway threshold criteria for street segment analysis indicates that the proposed project is not anticipated to significantly impact the analyzed street segments. Incremental, but less than significant impacts are noted at the study street segments under future with project conditions, as presented in *Table 9-3*. Thus, no mitigation measures are required or recommended.

10.0 TRANSPORTATION IMPROVEMENT MEASURES

The following sections provide an overview of transportation improvement measures that are anticipated to address impacts to the local roadway network associated with the proposed project. It is important to note that the traffic analysis has been based on a conservative approach with respect to the analysis of potential project-related impacts.

10.1 Study Intersections

As summarized in Subsections 9.1.2 (Existing With Project Conditions) and 9.1.4 (Future With Project Conditions) herein, application of the City's threshold criteria to the with proposed project scenarios indicates that the proposed project is not expected to create significant impacts at any of the six study intersections. Incremental, but not significant, impacts are noted at the study intersections. Because there are no significant impacts, no traffic mitigation measures are required or recommended for the study intersections.

10.2 Street Segment Improvement Measures

As summarized in Subsection 9.2 (Study Street Segment Analysis) herein, application of the City's threshold criteria to the with proposed project scenarios indicates that the proposed project is not expected to result in any significant traffic impacts at the two study street segments under existing with project or future with project conditions. Because there are no significant impacts, no traffic mitigation measures are required or recommended for the study street segment locations.

11.0 CONGESTION MANAGEMENT PROGRAM TRAFFIC IMPACT ASSESSMENT

The Congestion Management Program (CMP) is a state-mandated program that was enacted by the State Legislature with the passage of Proposition 111 in 1990. The program is intended to address the impact of local growth on the regional transportation system.

As required by the 2010 Congestion Management Program for Los Angeles County, a Traffic Impact Assessment (TIA) has been prepared to determine the potential impacts on designated monitoring locations on the CMP highway system. The analysis has been prepared in accordance with procedures outlined in the 2010 Congestion Management Program for Los Angeles County, County of Los Angeles Metropolitan Transportation Authority, July 2010.

11.1 Intersections

The following CMP intersection monitoring locations in the project vicinity have been identified:

•	CMP Station	Intersection
	No. 110	Sepulveda Boulevard/Rosecrans Avenue
	No. 22	Pacific Coast Highway/Artesia Boulevard-Gould Avenue

The CMP TIA guidelines require that intersection monitoring locations must be examined if the proposed project will add 50 or more trips during either the AM or PM weekday peak hours. The proposed project will not add 50 or more trips during either the AM or PM weekday peak hours (i.e., of adjacent street traffic) at the above CMP monitoring intersections in the project vicinity, which is stated in the CMP manual as the threshold criteria for a traffic impact assessment. Therefore, no further review of potential impacts to intersection monitoring locations that are part of the CMP highway system is required.

11.2 Freeways

No CMP freeway monitoring locations are located in the project vicinity. Further, the CMP TIA guidelines require that freeway monitoring locations must be examined if the proposed project will add 150 or more trips (in either direction) during either the AM or PM weekday peak periods. The proposed project will not add 150 or more trips (in either direction), during either the AM or PM weekday peak hours to the CMP freeway monitoring location, which is the threshold for preparing a traffic impact assessment, as stated in the CMP manual. Therefore, no further review of potential impacts to freeway monitoring locations that are part of the CMP highway system is required.

11.3 Transit Impact Review

As required by the 2010 Congestion Management Program for Los Angeles County, a review has been made of the CMP transit service. Existing transit service is provided in the vicinity of the proposed Chalk Pre-School Manhattan Beach project.

The project trip generation, as shown in *Table 7-1*, was adjusted by values set forth in the CMP (i.e., person trips equal 1.4 times vehicle trips, and transit trips equal 3.5 percent of the total person trips) to estimate transit trip generation. Pursuant to the CMP guidelines, the proposed project is forecast to generate demand for five transit trips during both the weekday AM and PM peak hours. Over a 24-hour period, the proposed project is forecast to generate demand for 26 daily transit trips. The calculations are as follows:

- AM Peak Hour = $95 \times 1.4 \times 0.035 = 5$ Transit Trips
- PM Peak Hour = $96 \times 1.4 \times 0.035 = 5$ Transit Trip
- Daily Trips = $522 \times 1.4 \times 0.035 = 26$ Transit Trips

As shown in *Table 4-1*, one bus transit line is provided adjacent to or in close proximity to the project site. As outlined in *Table 4-1* under the "No. of Buses During Peak Hour" column, the transit line provides service for an average (i.e., an average of the directional number of buses during the peak hours) of approximately eight buses during the AM peak hour and seven buses during the PM peak hour. Therefore, based on the above calculated peak hour transit trips, this would correspond to less than one transit rider per bus. Thus, given the low number of generated transit trips per bus, no impacts on existing or future transit services in the project area are expected to occur as a result of the proposed project.

12.0 SUMMARY OF FINDINGS AND CONCLUSIONS

This traffic impact study has been prepared to identify and evaluate the potential impacts of traffic generated by the proposed Chalk Pre-School Manhattan Beach project. The proposed project consists of the renovation and re-use of existing buildings on-site for a private pre-school operation of up to 119 children. Completion and occupancy of the proposed project is planned to be by the year 2015.

In order to evaluate the potential impacts due to the proposed project, six intersections and two street segment locations were identified for evaluation in consultation with the City staff to determine changes in traffic operations following occupancy and operation of the project. The proposed project is expected to generate 87 net new vehicle trips (45 inbound trips and 42 outbound trips) during the AM peak hour. During the PM peak hour, the proposed project is expected to generate 65 net new vehicle trips (30 inbound trips and 35 outbound trips). Over a 24-hour period, the proposed project is forecast to generate approximately 170 net new daily trip ends during a typical weekday (approximately 85 inbound trips and 85 outbound trips).

It is concluded that the proposed project is not expected to result in significant traffic impacts at any of the study intersections for existing with project and future with project conditions. Incremental, but less than significant impacts are noted at the study intersections. Therefore, no traffic mitigation measures are required or recommended for the study intersections.

The results of the Los Angeles CMP indicated that the proposed Chalk Pre-School Manhattan Beach project will not adversely affect any CMP arterial monitoring intersections or freeway monitoring locations, as well as nearby transit operations. Therefore, no improvements/mitigation measures are required of this project.

The proposed parking supply is expected to meet City of Manhattan Beach Code parking requirements. General pre-school traffic procedures are recommended in addition to a Traffic Management Plan, which should be maintained and included as part of the formal school policies. These school policies should be communicated to faculty, staff, students and parents/guardians at the beginning of the school year and be reinforced throughout the school year.

Α	P	P	Ε	N	D	IX	Α
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MANUAL INTERSECTION TRAFFIC COUNT DATA 24-HOUR AUTOMATIC STREET SEGMENT TRAFFIC COUNT DATA

LINSCOTT, LAW & GREENSPAN, engineers

LLG Ref. 1-14-4083-1

626.991.7522

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File Name : Sep_Marine Site Code : 00000000

Start Date : 6/10/2014

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					Groups r	milicu- Oi	isiiiiteu						
	Sepu	lveda Blvd		Ma	arine Ave		Sepu	lveda Blvd		Ma	arine Ave		
	Sou	thbound		We	estbound		No	rthbound		Ea	stbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	17	164	5	14	34	65	4	402	9	28	34	5	781
07:15 AM	25	167	13	26	44	65	6	462	8	42	37	7	902
07:30 AM	40	229	4	28	49	48	2	510	12	19	51	8	1000
07:45 AM	42	286	2	38	66	66	5	679	5	13	98	4	1304_
Total	124	846	24	106	193	244	17	2053	34	102	220	24	3987
08:00 AM	65	225	28	30	89	72	1	605	15	21	88	11	1250
08:15 AM	33	162	22	32	69	81	10	670	17	25	60	14	1195
08:30 AM	38	235	14	28	72	74	9	628	11	26	75	9	1219
08:45 AM	32	289	17	44	76	87	10	579	16	28	57	12	1247
Total	168	911	81	134	306	314	30	2482	59	100	280	46	4911
04:00 PM	33	452	25	37	52	42	20	340	28	22	36	15	1102
04:15 PM	50	570	34	43	58	31	19	341	31	14	70	15	1276
04:30 PM	49	566	33	52	76	43	22	329	28	22	82	12	1314
04:45 PM	56	522	34	50	61	31	24	370	28	19	46	14	1255
Total	188	2110	126	182	247	147	85	1380	115	77	234	56	4947
05:00 PM	47	432	25	55	71	38	24	310	26	22	75	23	1148
05:15 PM	56	557	29	48	60	29	27	363	27	13	69	10	1288
05:30 PM	57	460	26	65	74	44	20	298	23	28	68	11	1174
05:45 PM	34	545	27	44	86	43	20	349	31	14	65	12	1270
Total	194	1994	107	212	291	154	91	1320	107	77	277	56	4880
Grand Total	674	5861	338	634	1037	859	223	7235	315	356	1011	182	18725
Apprch %	9.8	85.3	4.9	25.1	41	34	2.9	93.1	4.1	23	65.3	11.7	
Total %	3.6	31.3	1.8	3.4	5.5	4.6	1.2	38.6	1.7	1.9	5.4	1	

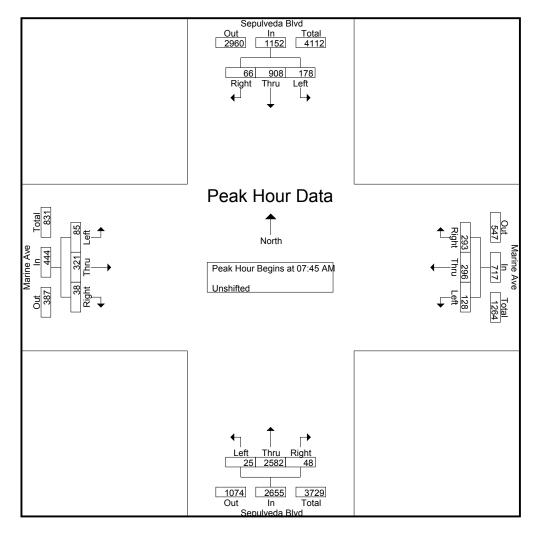
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File Name : Sep_Marine Site Code : 00000000

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		Sepulve	da Blvd			Mari	ne Ave			Sepulve	da Blvd		Marine Ave				
		South	oound			Westl	oound		Northbound				Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 11:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	42	286	2	330	38	66	66	170	5	679	5	689	13	98	4	115	1304
08:00 AM	65	225	28	318	30	89	72	191	1	605	15	621	21	88	11	120	1250
08:15 AM	33	162	22	217	32	69	81	182	10	670	17	697	25	60	14	99	1195
08:30 AM	38	235	14	287	28	72	74	174	9	628	11	648	26	75	9	110	1219
Total Volume	178	908	66	1152	128	296	293	717	25	2582	48	2655	85	321	38	444	4968
% App. Total	15.5	78.8	5.7		17.9	41.3	40.9		0.9	97.3	1.8		19.1	72.3	8.6		
PHF	.685	.794	.589	.873	.842	.831	.904	.938	.625	.951	.706	.952	.817	.819	.679	.925	.952



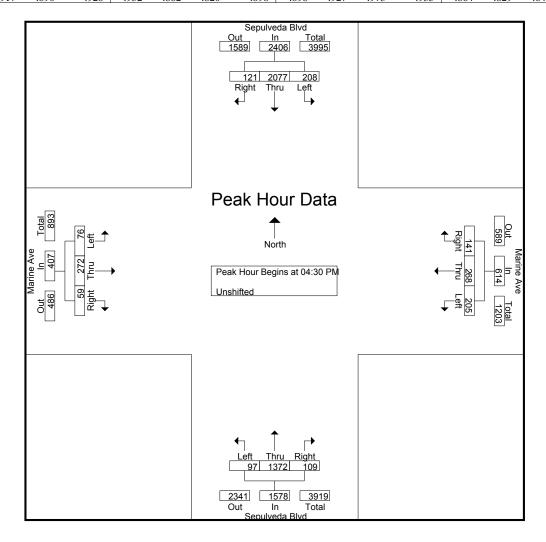
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Start Date : 6/10/2014

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		Southl	oound			Westl	bound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 1	2:00 PM	to 05:45	PM - Peak	1 of 1												
Peak Hour for Ent	ire Interse	ection Be	gins at 04	:30 PM													
04:30 PM	49	566	33	648	52	76	43	171	22	329	28	379	22	82	12	116	1314
04:45 PM	56	522	34	612	50	61	31	142	24	370	28	422	19	46	14	79	1255
05:00 PM	47	432	25	504	55	71	38	164	24	310	26	360	22	75	23	120	1148
05:15 PM	56	557	29	642	48	60	29	137	27	363	27	417	13	69	10	92	1288
Total Volume	208	2077	121	2406	205	268	141	614	97	1372	109	1578	76	272	59	407	5005
% App. Total	8.6	86.3	5		33.4	43.6	23		6.1	86.9	6.9		18.7	66.8	14.5		
PHF	.929	.917	.890	.928	.932	.882	.820	.898	.898	.927	.973	.935	.864	.829	.641	.848	.952



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				s Printea- Ban					
	Sepulveda B	lvd	Marine A	ve	Sepulveda B	lvd	Marine A	ve	
	Southbour		Westboun	d	Northbour		Eastbound	i	
Start Time	Thru	Peds	Thru	Peds	Thru	Peds	Thru	Peds	Int. Total
07:15 AM	0	0	0	4	0	2	0	0	6
07:30 AM	1	1	0	0	0	0	0	1	3
07:45 AM	1	0	0	2	0	5	0	1	9_
Total	2	1	0	6	0	7	0	2	18
08:00 AM	1	1	1	0	0	0	0	1	4
08:15 AM	0	0	0	1	0	3	1	0	5
08:30 AM	0	2	0	2	0	2	0	1	7
08:45 AM	0	5	0	0	0	0	0	3	8
Total	1	8	1	3	0	5	1	5	24
		1		1		1			
04:00 PM	0	0	1	0	0	2	0	1	4
04:15 PM	0	0	0	0	1	0	0	1	2
04:30 PM	1	0	0	0	0	0	0	1	2
04:45 PM	0	4	0	1	11	1	1	1	9
Total	1	4	1	1	2	3	1	4	17
1		1		1		1			
05:00 PM	0	2	0	0	3	2	0	2	9
05:15 PM	0	1	0	1	0	7	0	1	10
05:30 PM	2	1	0	0	1	3	0	0	7
05:45 PM	2	1	0	1	0	0	0	0	4
Total	4	5	0	2	4	12	0	3	30
1		1		1		1			
Grand Total	8	18	2	12	6	27	2	14	89
Apprch %	30.8	69.2	14.3	85.7	18.2	81.8	12.5	87.5	
Total %	9	20.2	2.2	13.5	6.7	30.3	2.2	15.7	

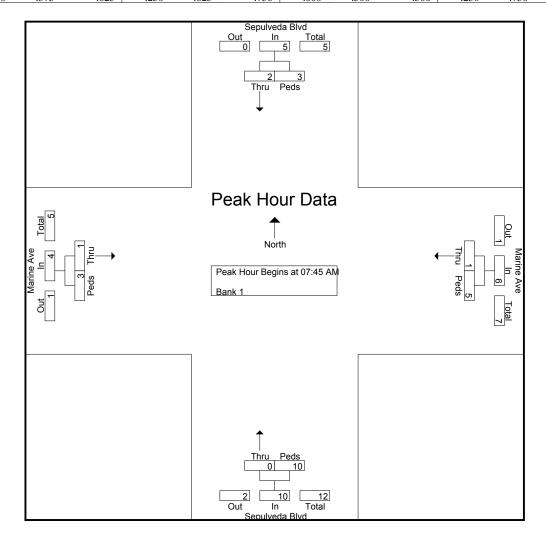
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	Sep	ulveda Blvd	i	N	Iarine A	ve	Sep	ulveda B	lvd	I	Marine Av	re	
	Se	outhbound		W	estboun	d	N	orthbour	ıd]	Eastbound	i	
Start Time	Thru	Peds A	pp. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 07:00 A	M to 11:45	AM - Peak	1 of 1									
Peak Hour for Entire I	ntersection	Begins at 07	:45 AM										
07:45 AM	1	0	1	0	2	2	0	5	5	0	1	1	9
08:00 AM	1	1	2	1	0	1	0	0	0	0	1	1	4
08:15 AM	0	0	0	0	1	1	0	3	3	1	0	1	5
08:30 AM	0	2	2	0	2	2	0	2	2	0	1	1	7
Total Volume	2	3	5	1	5	6	0	10	10	1	3	4	25
% App. Total	40	60		16.7	83.3		0	100		25	75		
PHF	.500	.375	.625	.250	.625	.750	.000	.500	.500	.250	.750	1.00	.694



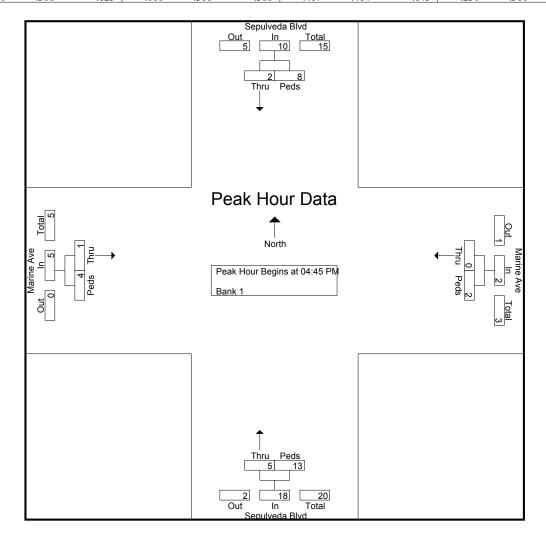
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	Sep	ulveda Blvd	l	N	Iarine Av	re	Sep	ulveda B	lvd	N	Marine Av	re	
	So	outhbound		W	estbound	d	N	orthbour	ıd	I	Eastbound	l	
Start Time	Thru	Peds A	pp. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 12:00 P	M to 05:45 I	PM - Peak 1	of 1									
Peak Hour for Entire I	intersection l	Begins at 04	:45 PM										
04:45 PM	0	4	4	0	1	1	1	1	2	1	1	2	9
05:00 PM	0	2	2	0	0	0	3	2	5	0	2	2	9
05:15 PM	0	1	1	0	1	1	0	7	7	0	1	1	10
05:30 PM	2	1	3	0	0	0	1	3	4	0	0	0	7_
Total Volume	2	8	10	0	2	2	5	13	18	1	4	5	35
% App. Total	20	80		0	100		27.8	72.2		20	80		
PHF	.250	.500	.625	.000	.500	.500	.417	.464	.643	.250	.500	.625	.875



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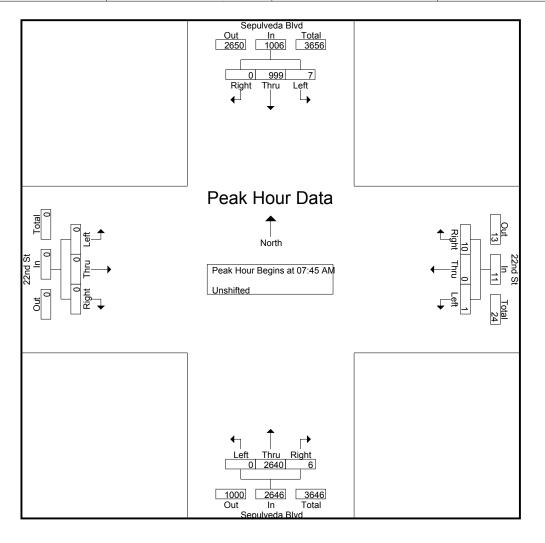
					Groups I	<u> Printed-U</u>	nshifted						
	Sepu	lveda Blvd		2	2nd St		Sepu	lveda Blvd		2	22nd St		
	Sou	ıthbound		We	estbound		No	rthbound		Ea	stbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	1	173	1	0	0	1	0	392	0	0	0	0	568
07:15 AM	0	180	0	0	0	5	0	470	1	0	0	0	656
07:30 AM	0	230	1	0	0	0	0	526	1	0	0	0	758
07:45 AM	2	275	0	0	0	3	0	667	3	0	0	0	950_
Total	3	858	2	0	0	9	0	2055	5	0	0	0	2932
08:00 AM	2	233	0	0	0	0	0	612	1	0	0	0	848
08:15 AM	0	188	0	0	0	2	0	691	1	0	0	0	882
08:30 AM	3	303	0	1	0	5	0	670	1	0	0	0	983
08:45 AM	6	299	0	0	0	3	0	588	3	0	0	0	899
Total	11	1023	0	1	0	10	0	2561	6	0	0	0	3612
												1	
04:00 PM	2	511	1	3	0	5	0	443	6	0	0	0	971
04:15 PM	1	619	0	0	0	7	0	388	4	0	0	0	1019
04:30 PM	1	589	5	0	0	2	0	411	3	0	0	0	1011
04:45 PM	3	588	0	0	0	4	0	463	8	0	0	0	1066
Total	7	2307	6	3	0	18	0	1705	21	0	0	0	4067
05:00 PM	1	540	0	0	0	0	0	405	6	0	0	0	952
05:15 PM	0	599	0	2	0	2	0	422	9	0	0	0	1034
05:30 PM	0	521	0	0	1	1	0	332	3	0	0	0	858
05:45 PM	1	601	0	2	1	3	0	415	5	0	0	0	1028
Total	2	2261	0	4	2	6	0	1574	23	0	0	0	3872
												i	
Grand Total	23	6449	8	8	2	43	0	7895	55	0	0	0	14483
Apprch %	0.4	99.5	0.1	15.1	3.8	81.1	0	99.3	0.7	0	0	0	
Total %	0.2	44.5	0.1	0.1	0	0.3	0	54.5	0.4	0	0	0	

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																	_
		Sepulve	da Blvd			22n	d St			Sepulve	da Blvd			22n	d St		
		South	oound			Westl	oound			North	bound			Easth	ound		<u> </u>
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 0	7:00 AM	to 11:45	AM - Peak	1 of 1		_				_				_		
Peak Hour for Ent	ire Interse	ection Be	gins at 07	7:45 AM													
07:45 AM	2	275	0	277	0	0	3	3	0	667	3	670	0	0	0	0	950
08:00 AM	2	233	0	235	0	0	0	0	0	612	1	613	0	0	0	0	848
08:15 AM	0	188	0	188	0	0	2	2	0	691	1	692	0	0	0	0	882
08:30 AM	3	303	0	306	1	0	5	6	0	670	1	671	0	0	0	0	983
Total Volume	7	999	0	1006	1	0	10	11	0	2640	6	2646	0	0	0	0	3663
% App. Total	0.7	99.3	0		9.1	0	90.9		0	99.8	0.2		0	0	0		
PHF	.583	.824	.000	.822	.250	.000	.500	.458	.000	.955	.500	.956	.000	.000	.000	.000	.932

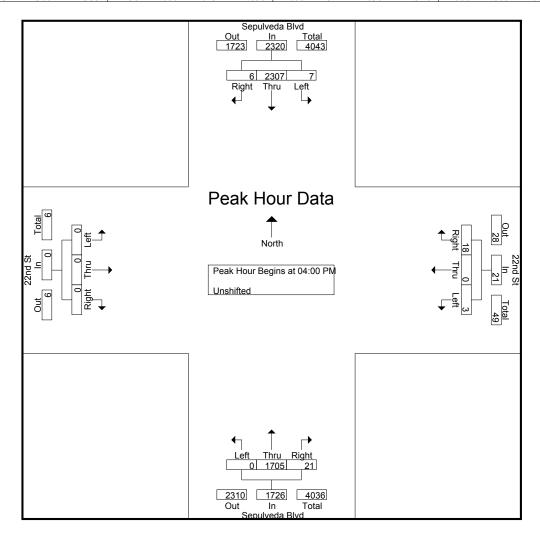


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		South	bound			Westl	ound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 1	2:00 PM	to 05:45	PM - Peak	1 of 1												
Peak Hour for Ent	ire Interse	ection Be	gins at 04	1:00 PM													
04:00 PM	2	511	1	514	3	0	5	8	0	443	6	449	0	0	0	0	971
04:15 PM	1	619	0	620	0	0	7	7	0	388	4	392	0	0	0	0	1019
04:30 PM	1	589	5	595	0	0	2	2	0	411	3	414	0	0	0	0	1011
04:45 PM	3	588	0	591	0	0	4	4	0	463	8	471	0	0	0	0	1066
Total Volume	7	2307	6	2320	3	0	18	21	0	1705	21	1726	0	0	0	0	4067
% App. Total	0.3	99.4	0.3		14.3	0	85.7		0	98.8	1.2		0	0	0		
PHF	.583	.932	.300	.935	.250	.000	.643	.656	.000	.921	.656	.916	.000	.000	.000	.000	.954



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File Name: sep_22nd_bp Site Code: 00000000

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				1 I IIIICu- Dan					
	Sepulveda B		22nd St		Sepulveda B		22nd St		
	Southboun	ıd	Westboun	d	Northbour	nd	Eastbound	1	
Start Time	Thru	Peds	Thru	Peds	Thru	Peds	Thru	Peds	Int. Total
07:00 AM	0	0	0	0	0	0	0	1	1
07:15 AM	0	0	0	0	0	0	0	1	1
Total	0	0	0	0	0	0	0	2	2
08:00 AM	0	0	0	0	0	0	0	1	1
08:15 AM	0	0	0	0	0	0	1	0	1
08:30 AM	0	0	0	0	0	0	0	1	1
Total	0	0	0	0	0	0	1	2	3
		- 1		- 1		- 1	_	. 1	
04:00 PM	0	0	0	0	0	0	0	1	1
04:15 PM	0	1	0	0	0	0	0	0	1
04:30 PM	0	0	0	0	0	0	5	1	6
Total	0	1	0	0	0	0	5	2	8
05:00 PM	0	0	0	0	0	0	0	2	2
05:15 PM	0	0	0	0	0	0	0	1	1
05:45 PM	0	1	0	0	0	0	0	2	3_
Total	0	1	0	0	0	0	0	5	6
Grand Total	0	2	0	0	0	0	6	11	19
Apprch %	0	100	0	0	0	0	35.3	64.7	
Total %	0	10.5	0	0	0	0	31.6	57.9	

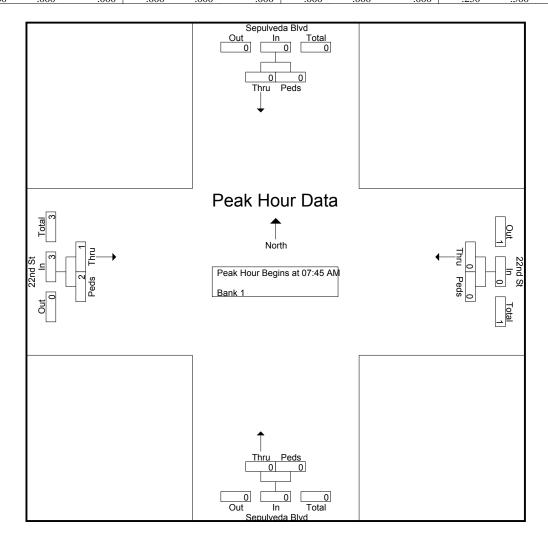
626.991.7522

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File Name: sep_22nd_bp

Site Code : 00000000 Start Date : 6/10/2014

	Sep	ulveda Bl	lvd		22nd St		Sep	ulveda B	lvd		22nd St		
	So	uthboun	d	v	Vestboun	d	N	orthbour	nd		Eastbound	d	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 07:00 A	M to 11:4	5 AM - Peak	1 of 1									
Peak Hour for Entire I	ntersection I	Begins at (07:45 AM	_									
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	1	1	1
08:15 AM	0	0	0	0	0	0	0	0	0	1	0	1	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	1	1	1_
Total Volume	0	0	0	0	0	0	0	0	0	1	2	3	3
% App. Total	0	0		0	0		0	0		33.3	66.7		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.500	.750	.750



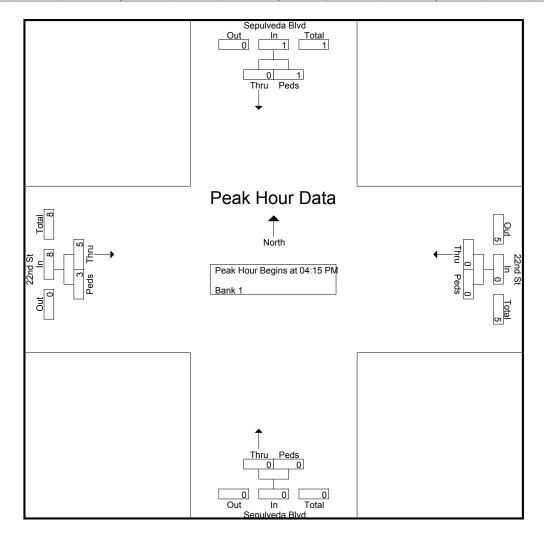
626.991.7522

www.ctcounters.com

File Name: sep_22nd_bp

Site Code : 00000000 Start Date : 6/10/2014

	Sep	ulveda Blv	'd		22nd St		Sep	ulveda B	lvd		22nd St		
	So	uthbound		W	estboun	d	Ñ	orthbour	ıd]	Eastboun	d	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis Fr	rom 12:00 Pl	M to 05:45	PM - Peak	l of 1									
Peak Hour for Entire I	ntersection I	Begins at 04	4:15 PM										
04:15 PM	0	1	1	0	0	0	0	0	0	0	0	0	1
04:30 PM	0	0	0	0	0	0	0	0	0	5	1	6	6
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	0	0	0	0	0	0	0	0	0	0	2	2	2
Total Volume	0	1	1	0	0	0	0	0	0	5	3	8	9
% App. Total	0	100		0	0		0	0		62.5	37.5		
PHF	.000	.250	.250	.000	.000	.000	.000	.000	.000	.250	.375	.333	.375



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File Name: Sep_18th Site Code: 00000000 Start Date: 6/10/2014

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Groups Printed- Unshifted

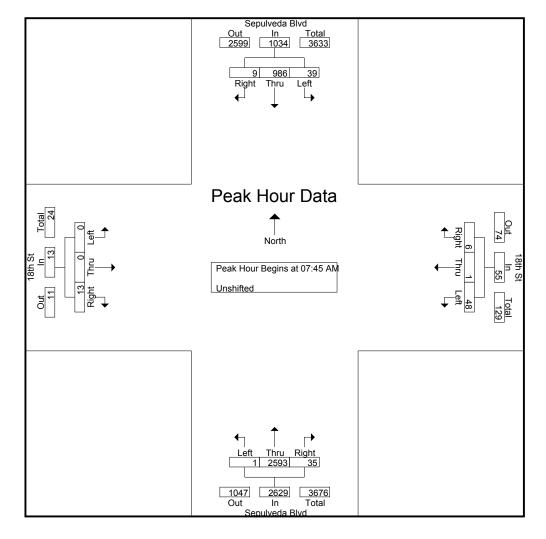
					Groups F	<u> Printed- U</u>	nshifted						
	Sepu	ılveda Blvd	ı I		18th St		Sepi	ulveda Blvd	ı		18th St		
	So	uthbound		We	estbound		No	orthbound		Ea	astbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	2	191	2	3	0	2	0	384	3	0	0	7	594
07:15 AM	2	214	2	6	0	2	1	460	2	0	0	4	693
07:30 AM	4	248	2	3	0	0	0	533	6	0	0	1	797
07:45 AM	17	269	2	3	0	2	0	660	3	0	0	2	958
Total	25	922	8	15	0	6	1	2037	14	0	0	14	3042
08:00 AM	11	234	2	26	1	1	1	617	15	0	0	3	911
08:15 AM	7	200	3	3	0	3	0	683	11	0	0	4	914
08:30 AM	4	283	2	16	0	0	0	633	6	0	0	4	948
08:45 AM	4	277	0	15	1	1	1	581	5	0	0	2	887
Total	26	994	7	60	2	5	2	2514	37	0	0	13	3660
	1											1	
04:00 PM	1	524	0	18	0	1	0	412	13	0	0	4	973
04:15 PM	0	613	1	11	1	0	0	376	12	0	0	2	1016
04:30 PM	1	615	3	16	0	0	0	366	6	0	0	1	1008
04:45 PM	1	553	1	18	0	2	0	423	16	0	0	1	1015
Total	3	2305	5	63	1	3	0	1577	47	0	0	8	4012
	ı		1			1						ı	
05:00 PM	1	563	2	23	0	1	0	377	15	0	0	1	983
05:15 PM	0	562	5	16	0	2	0	401	18	0	0	5	1009
05:30 PM	2	546	5	16	0	0	0	394	8	0	0	8	979
05:45 PM	0	579	4	28	0	4	0	392	10	0	0	6	1023
Total	3	2250	16	83	0	7	0	1564	51	0	0	20	3994
	ı		1			1						i	
Grand Total	57	6471	36	221	3	21	3	7692	149	0	0	55	14708
Apprch %	0.9	98.6	0.5	90.2	1.2	8.6	0	98.1	1.9	0	0	100	
Total %	0.4	44	0.2	1.5	0	0.1	0	52.3	1	0	0	0.4	

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File Name: Sep_18th Site Code: 00000000 Start Date: 6/10/2014

	1	Sepulve	da Blvd			18t	h St			Sepulve	da Blvd			18t	h St		
		South	bound			Westl	ound			North	bound			Eastb	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 0'	7:00 AM	to 11:45	AM - Peak	1 of 1		-				-				_		
Peak Hour for Ent	ire Interse	ction Be	gins at 07	:45 AM													
07:45 AM	17	269	2	288	3	0	2	5	0	660	3	663	0	0	2	2	958
08:00 AM	11	234	2	247	26	1	1	28	1	617	15	633	0	0	3	3	911
08:15 AM	7	200	3	210	3	0	3	6	0	683	11	694	0	0	4	4	914
08:30 AM	4	283	2	289	16	0	0	16	0	633	6	639	0	0	4	4	948_
Total Volume	39	986	9	1034	48	1	6	55	1	2593	35	2629	0	0	13	13	3731
% App. Total	3.8	95.4	0.9		87.3	1.8	10.9		0	98.6	1.3		0	0	100		
PHF	.574	.871	.750	.894	.462	.250	.500	.491	.250	.949	.583	.947	.000	.000	.813	.813	.974

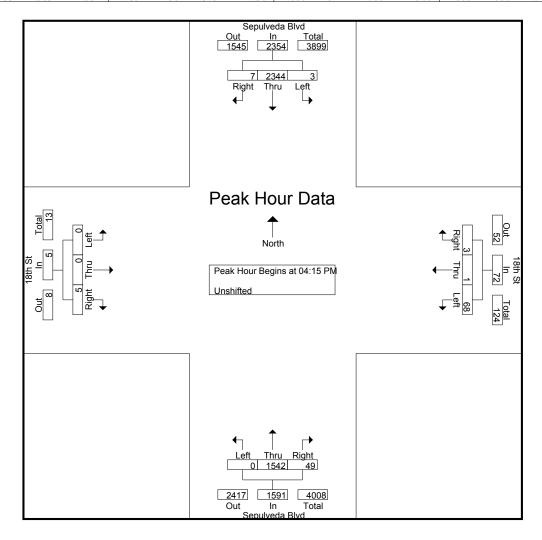


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File Name: Sep_18th Site Code: 00000000 Start Date: 6/10/2014

	Sepulveda Blvd				18th St				Sepulveda Blvd				18th St				
		South	bound			Westl	ound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 1	2:00 PM	to 05:45	PM - Peak	1 of 1												
Peak Hour for Ent	ire Interse	ection Be	gins at 04	1:15 PM													
04:15 PM	0	613	1	614	11	1	0	12	0	376	12	388	0	0	2	2	1016
04:30 PM	1	615	3	619	16	0	0	16	0	366	6	372	0	0	1	1	1008
04:45 PM	1	553	1	555	18	0	2	20	0	423	16	439	0	0	1	1	1015
05:00 PM	1	563	2	566	23	0	1	24	0	377	15	392	0	0	1	1	983
Total Volume	3	2344	7	2354	68	1	3	72	0	1542	49	1591	0	0	5	5	4022
% App. Total	0.1	99.6	0.3		94.4	1.4	4.2		0	96.9	3.1		0	0	100		
PHF	.750	.953	.583	.951	.739	.250	.375	.750	.000	.911	.766	.906	.000	.000	.625	.625	.990



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File Name: Sep_18th_BP

Site Code : 00000000 Start Date : 6/10/2014

Page No : 1

Groups Printed- Bank 1

Groups Frinteu- Bank 1													
	Sepulveda l		18th St	I	Sepulveda B		18th St						
	Southbou		Westboun		Northbour		Eastbound						
Start Time	Thru	Peds	Thru	Peds	Thru	Peds	Thru	Peds	Int. Total				
07:00 AM	0	4	0	2	0	1	0	1	8				
07:15 AM	0	0	0	3	0	5	0	4	12				
07:30 AM	0	2	0	9	0	0	0	4	15				
07:45 AM	0	4	1	2	0	0	0	2	9_				
Total	0	10	1	16	0	6	0	11	44				
08:00 AM	0	2	0	0	0	4	0	1	7				
08:15 AM	0	0	0	4	0	5	0	3	12				
08:30 AM	0	2	0	5	0	3	0	1	11				
08:45 AM	0	0	0	2	0	2	0	2	6_				
Total	0	4	0	11	0	14	0	7	36				
04:00 PM	0	4	0	0	0	2	0	2	8				
04:15 PM	0	1	0	0	0	1	0	1	3				
04:30 PM	0	0	0	0	0	1	0	2	3				
04:45 PM	0	2	0	0	0	1	0	2	5				
Total	0	7	0	0	0	5	0	7	19				
05:00 PM	0	3	0	1	0	1	0	2	7				
05:15 PM	0	0	0	1	0	5	0	3	9				
05:30 PM	0	1	0	0	0	2	0	0	3				
05:45 PM	0	0	0	0	0	2	0	0	2_				
Total	0	4	0	2	0	10	0	5	21				
Grand Total	0	25	1	29	0	35	0	30	120				
Apprch %	0	100	3.3	96.7	0	100	0	100					
Total %	0	20.8	0.8	24.2	0	29.2	0	25					
		·											

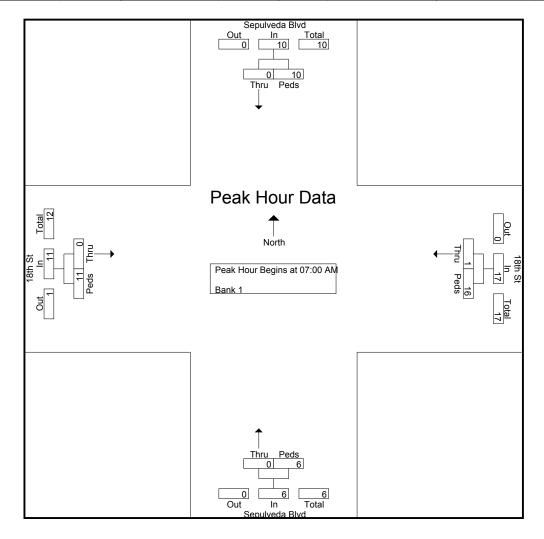
626.991.7522

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File Name: Sep_18th_BP

Site Code : 00000000 Start Date : 6/10/2014

	Sej	pulveda Blv	vd		18th St		Sej	oulveda B	lvd		18th St		
	S	outhbound	l	V	Vestboun	d	N	Northbour	ıd		Eastbound	d	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 07:00 A	AM to 11:45	5 AM - Peak	1 of 1									
Peak Hour for Entire I	ntersection	Begins at 0	7:00 AM										
07:00 AM	0	4	4	0	2	2	0	1	1	0	1	1	8
07:15 AM	0	0	0	0	3	3	0	5	5	0	4	4	12
07:30 AM	0	2	2	0	9	9	0	0	0	0	4	4	15
07:45 AM	0	4	4	1	2	. 3	0	0	0	0	2	2	9
Total Volume	0	10	10	1	16	17	0	6	6	0	11	11	44
% App. Total	0	100		5.9	94.1		0	100		0	100		
PHF	.000	.625	.625	.250	.444	.472	.000	.300	.300	.000	.688	.688	.733



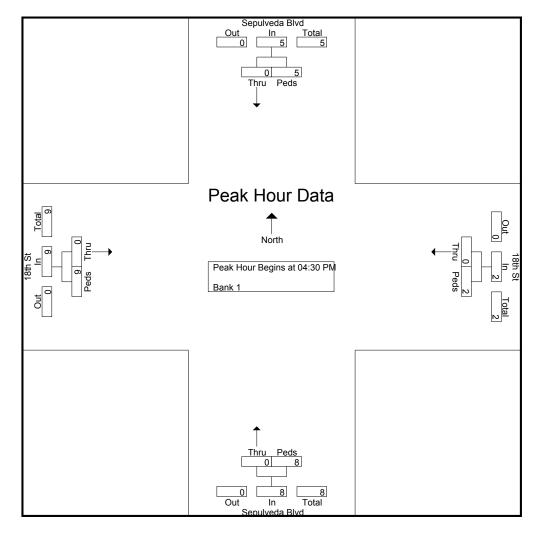
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File Name: Sep_18th_BP

Site Code : 00000000 Start Date : 6/10/2014

	Sep	ulveda Blv	d		18th St		Sep	oulveda B	lvd		18th St		
	So	outhbound		W	estboun	d	N	orthbour	ıd	1	Eastbound	d	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis Fi	rom 12:00 Pl	M to 05:45	PM - Peak	l of 1									
Peak Hour for Entire I	ntersection I	Begins at 04	4:30 PM										
04:30 PM	0	0	0	0	0	0	0	1	1	0	2	2	3
04:45 PM	0	2	2	0	0	0	0	1	1	0	2	2	5
05:00 PM	0	3	3	0	1	1	0	1	1	0	2	2	7
05:15 PM	0	0	0	0	1	1	0	5	5	0	3	3	9
Total Volume	0	5	5	0	2	2	0	8	8	0	9	9	24
% App. Total	0	100		0	100		0	100		0	100		
PHF	.000	.417	.417	.000	.500	.500	.000	.400	.400	.000	.750	.750	.667



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File Name : Cedar_Marine Site Code : 00000000 Start Date : 6/10/2014

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Groups Printed- Unshifted

						rintea- Oi							
	Ce	edar Ave		Ma	rine Ave		Ce	dar Ave		Ma	arine Ave		
	Sou	thbound		We	stbound		Nor	thbound		Ea	stbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	11	4	7	8	99	41	4	11	1	2	56	1	245
07:15 AM	10	5	9	6	113	51	2	4	4	6	65	6	281
07:30 AM	13	5	13	8	117	51	4	7	7	11	75	4	315
07:45 AM	15	5	5	6	152	59	3	11	9	6	155	3	429
Total	49	19	34	28	481	202	13	33	21	25	351	14	1270
08:00 AM	19	8	13	5	174	72	9	17	13	11	122	6	469
08:15 AM	22	5	9	11	137	74	6	19	6	18	90	4	401
08:30 AM	25	8	14	11	139	85	11	10	1	10	81	7	402
08:45 AM	26	6	19	6	149	88	9	20	5	18	85	4	435
Total	92	27	55	33	599	319	35	66	25	57	378	21	1707
						1							
04:00 PM	71	9	31	6	79	62	8	8	5	14	74	3	370
04:15 PM	72	13	51	10	87	60	2	7	8	15	132	14	471
04:30 PM	69	11	39	6	99	58	9	11	12	14	120	8	456
04:45 PM	77	21	41	4	109	80	2	9	10	15	107	11	486_
Total	289	54	162	26	374	260	21	35	35	58	433	36	1783
i						1							
05:00 PM	83	9	41	7	85	51	9	6	6	13	139	7	456
05:15 PM	84	15	54	3	84	53	7	11	9	17	131	10	478
05:30 PM	75	20	40	9	124	53	5	9	11	14	126	4	490
05:45 PM	98	7	50	14	120	41	6	6	8	7	130	14	501
Total	340	51	185	33	413	198	27	32	34	51	526	35	1925
Grand Total	770	151	436	120	1867	979	96	166	115	191	1688	106	6685
Apprch %	56.7	11.1	32.1	4	62.9	33	25.5	44	30.5	9.6	85	5.3	
Total %	11.5	2.3	6.5	1.8	27.9	14.6	1.4	2.5	1.7	2.9	25.3	1.6	

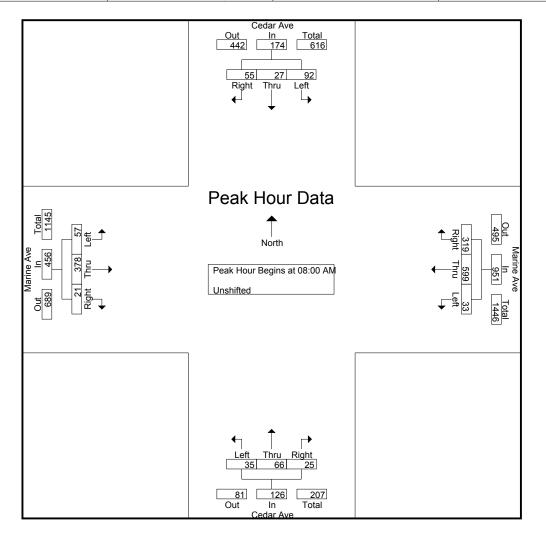
626.991.7522

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File Name: Cedar_Marine

Site Code : 00000000 Start Date : 6/10/2014

		Ceda	r Ave			Mari	ne Ave			Ceda	r Ave			Mari	ne Ave		
		South	oound			Westl	oound			North	bound			Eastb	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 0	7:00 AM	to 11:45	AM - Peak	1 of 1		_				_				_		
Peak Hour for Ent	ire Interse	ction Beg	gins at 08	:00 AM													
08:00 AM	19	8	13	40	5	174	72	251	9	17	13	39	11	122	6	139	469
08:15 AM	22	5	9	36	11	137	74	222	6	19	6	31	18	90	4	112	401
08:30 AM	25	8	14	47	11	139	85	235	11	10	1	22	10	81	7	98	402
08:45 AM	26	6	19	51	6	149	88	243	9	20	5	34	18	85	4	107	435
Total Volume	92	27	55	174	33	599	319	951	35	66	25	126	57	378	21	456	1707
% App. Total	52.9	15.5	31.6		3.5	63	33.5		27.8	52.4	19.8		12.5	82.9	4.6		
PHF	.885	.844	.724	.853	.750	.861	.906	.947	.795	.825	.481	.808	.792	.775	.750	.820	.910



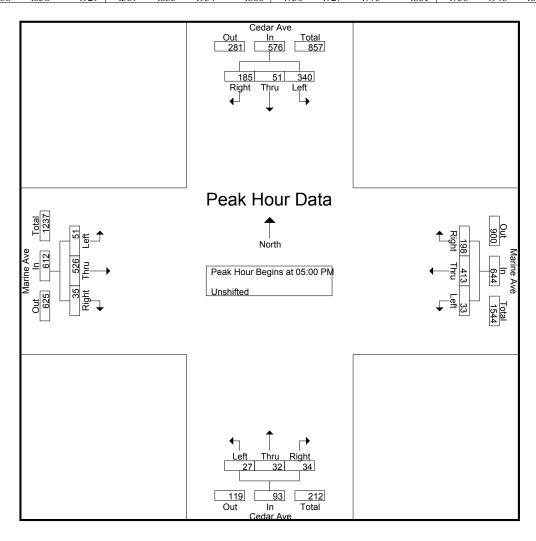
626.991.7522

www.ctcounters.com

File Name: Cedar_Marine

Site Code : 00000000 Start Date : 6/10/2014

																	_
		Ceda	r Ave			Mari	ne Ave			Ceda	r Ave			Mari	ne Ave		
		South	oound			Westl	oound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 12	2:00 PM	to 05:45	PM - Peak	1 of 1												
Peak Hour for Ent	ire Interse	ction Beg	gins at 05	:00 PM													
05:00 PM	83	9	41	133	7	85	51	143	9	6	6	21	13	139	7	159	456
05:15 PM	84	15	54	153	3	84	53	140	7	11	9	27	17	131	10	158	478
05:30 PM	75	20	40	135	9	124	53	186	5	9	11	25	14	126	4	144	490
05:45 PM	98	7	50	155	14	120	41	175	6	6	8	20	7	130	14	151	501
Total Volume	340	51	185	576	33	413	198	644	27	32	34	93	51	526	35	612	1925
% App. Total	59	8.9	32.1		5.1	64.1	30.7		29	34.4	36.6		8.3	85.9	5.7		
PHF	.867	.638	.856	.929	.589	.833	.934	.866	.750	.727	.773	.861	.750	.946	.625	.962	.961



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File Name : Cedar_Marine_BP Site Code : 00000000

Start Date : 6/10/2014

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Groups Printed- Bank 1

			Groups	s Printea-Bani	(1				
	Cedar Av	e	Marine A	ve	Cedar Av	re e	Marine A	ve	
	Southboun	ıd	Westboun	d	Northbour	nd	Eastbound	i	
Start Time	Thru	Peds	Thru	Peds	Thru	Peds	Thru	Peds	Int. Total
07:00 AM	0	0	1	1	0	0	0	2	4
07:30 AM	0	0	0	0	0	0	0	2	2
07:45 AM	1	1	0	0	0	1	0	2	5_
Total	1	1	1	1	0	1	0	6	11
		1				1			
08:00 AM	1	0	0	0	0	0	0	2	3
08:15 AM	0	1	0	1	0	1	0	1	4
08:30 AM	0	0	0	1	0	0	0	2	3
08:45 AM	0	0	0	0	0	0	0	3	3
Total	1	1	0	2	0	1	0	8	13
04.00 77.5				. 1		ا م			
04:00 PM	0	0	0	1	0	0	0	1	2
04:15 PM	2	1	1	1	1	0	0	1	7
04:30 PM	1	1	0	0	0	0	0	0	2
04:45 PM	0	0	0	2	0	2	0	2	6
Total	3	2	1	4	1	2	0	4	17
05:00 PM	1	0	1	0	1	3	0	0	6
05:15 PM	0	1	0	1	0	0	0	3	5
05:30 PM	1	0	1	0	0	0	0	2	<i>J</i>
05:45 PM	0	0	0	1	0	0	0	1	2
Total	2	1	2	2	1	3	0	6	17
Total	2	1	2	2	1	3	O	0	17
Grand Total	7	5	4	9	2	7	0	24	58
Appreh %	58.3	41.7	30.8	69.2	22.2	77.8	ő	100	23
Total %	12.1	8.6	6.9	15.5	3.4	12.1	0	41.4	
							~		

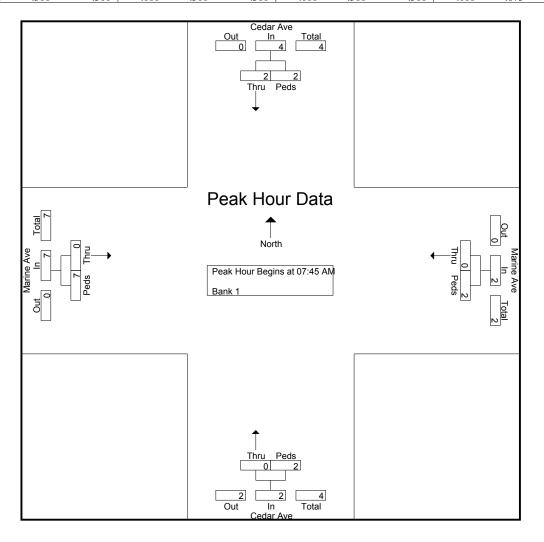
626.991.7522

www.ctcounters.com

File Name: Cedar_Marine_BP

Site Code : 00000000 Start Date : 6/10/2014

	(Cedar Ave		N	Aarine A	ve		Cedar Av	e		Marine Av	ve	
	So	outhbound		V	Vestboun	d	ľ	Northbour	ıd		Eastbound	d	
Start Time	Thru	Peds A	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 07:00 A	M to 11:45	AM - Peak	1 of 1									
Peak Hour for Entire I	ntersection I	Begins at 07	:45 AM										
07:45 AM	1	1	2	0	0	0	0	1	1	0	2	2	5
08:00 AM	1	0	1	0	0	0	0	0	0	0	2	2	3
08:15 AM	0	1	1	0	1	1	0	1	1	0	1	1	4
08:30 AM	0	0	0	0	1	1	0	0	0	0	2	2	3
Total Volume	2	2	4	0	2	2	0	2	2	0	7	7	15
% App. Total	50	50		0	100		0	100		0	100		
PHF	.500	.500	.500	.000	.500	.500	.000	.500	.500	.000	.875	.875	.750



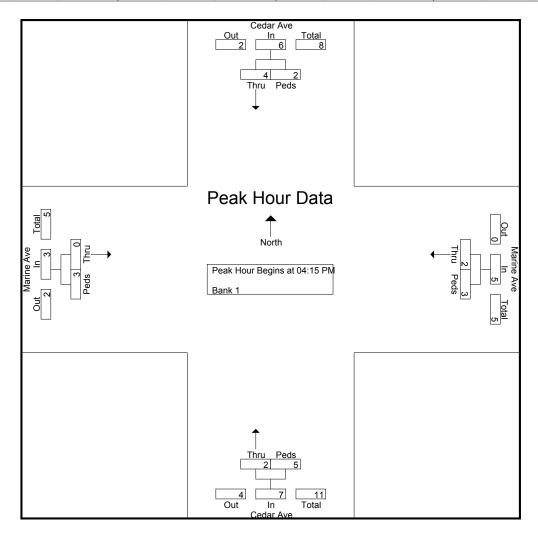
626.991.7522

www.ctcounters.com

File Name : Cedar_Marine_BP Site Code : 00000000

Start Date : 6/10/2014

		Cedar Ave	:	N	Iarine Av	ve		Cedar Av	e	N	Marine Av	ve	
	S	outhbound]	v	Vestboun	d	N	orthbour	ıd	1	Eastbound	i	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 12:00 I	PM to 05:45	PM - Peak	1 of 1									
Peak Hour for Entire I	Intersection	Begins at 0	4:15 PM										
04:15 PM	2	1	3	1	1	2	1	0	1	0	1	1	7
04:30 PM	1	1	2	0	0	0	0	0	0	0	0	0	2
04:45 PM	0	0	0	0	2	2	0	2	2	0	2	2	6
05:00 PM	1	0	1	1	0	1	1	3	4	0	0	0	6
Total Volume	4	2	6	2	3	5	2	5	7	0	3	3	21
% App. Total	66.7	33.3		40	60		28.6	71.4		0	100		
PHF	.500	.500	.500	.500	.375	.625	.500	.417	.438	.000	.375	.375	.750



626.991.7522

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File Name : Cedar_22nd Site Code : 00000000

Start Date : 6/10/2014

Page No : 1

Groups Printed- Unshifted

					Groups 1	rintea- U	nsniitea						
	Ce	edar Ave		2	22nd St		Co	edar Ave		2	22nd St		
	Sou	thbound		We	estbound		No	rthbound		Ea	stbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	1	5	1	2	1	3	1	5	0	2	0	1	22
07:15 AM	2	8	1	1	3	5	1	6	0	0	0	0	27
07:30 AM	0	11	5	0	3	6	3	9	0	2	2	0	41
07:45 AM	2	16	6	0	4	3	1	17	0	6	1	1	57_
Total	5	40	13	3	11	17	6	37	0	10	3	2	147
08:00 AM	3	14	6	2	6	11	3	21	0	7	0	2	75
08:15 AM	3	22	10	0	5	8	0	15	0	6	0	0	69
08:30 AM	0	13	9	0	4	6	2	12	0	4	1	2	53
08:45 AM	1	13	6	0	2	6	2	9	0	5	2	2	48
Total	7	62	31	2	17	31	7	57	0	22	3	6	245
04:00 PM	0	18	8	0	0	1	1	15	0	4	1	5	53
04:15 PM	0	22	10	0	3	4	4	14	0	7	3	4	71
04:30 PM	0	23	4	0	3	5	2	18	0	12	1	2	70
04:45 PM	0	26	9	1	4	2	11	18	0	8	2	3	74_
Total	0	89	31	1	10	12	8	65	0	31	7	14	268
												1	
05:00 PM	0	23	8	0	1	5	1	16	0	11	1	6	72
05:15 PM	0	22	7	1	2	2	3	18	0	9	2	3	69
05:30 PM	0	26	5	0	0	5	1	16	0	5	1	7	66
05:45 PM	0	21	9	0	2	2	1	11	0	7	0	4	57
Total	0	92	29	1	5	14	6	61	0	32	4	20	264
Grand Total	12	283	104	7	43	74	27	220	0	95	17	42	924
Apprch %	3	70.9	26.1	5.6	34.7	59.7	10.9	89.1	0	61.7	11	27.3	
Total %	1.3	30.6	11.3	0.8	4.7	8	2.9	23.8	0	10.3	1.8	4.5	

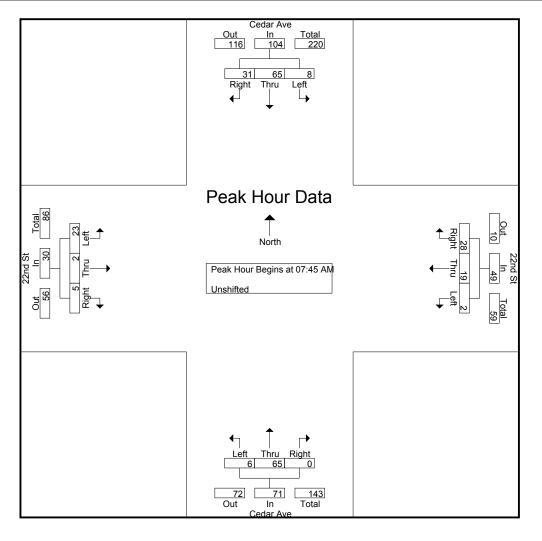
626.991.7522

www.ctcounters.com

File Name : Cedar_22nd Site Code : 00000000

Start Date : 6/10/2014

		Ceda	r Ave			22n	d St			Ceda	r Ave			22n	d St		
		South	oound			Westl	ound			North	bound			Eastb	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 0'	7:00 AM	to 11:45	AM - Peak	1 of 1		_				_				_		
Peak Hour for Ent	ire Interse	ction Beg	gins at 07	:45 AM													
07:45 AM	2	16	6	24	0	4	3	7	1	17	0	18	6	1	1	8	57
08:00 AM	3	14	6	23	2	6	11	19	3	21	0	24	7	0	2	9	75
08:15 AM	3	22	10	35	0	5	8	13	0	15	0	15	6	0	0	6	69
08:30 AM	0	13	9	22	0	4	6	10	2	12	0	14	4	1	2	7	53_
Total Volume	8	65	31	104	2	19	28	49	6	65	0	71	23	2	5	30	254
% App. Total	7.7	62.5	29.8		4.1	38.8	57.1		8.5	91.5	0		76.7	6.7	16.7		
PHF	.667	.739	.775	.743	.250	.792	.636	.645	.500	.774	.000	.740	.821	.500	.625	.833	.847



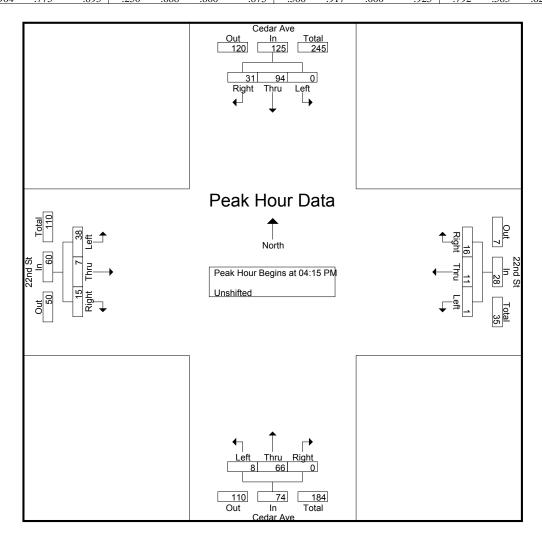
626.991.7522

www.ctcounters.com

File Name : Cedar_22nd Site Code : 00000000

Start Date : 6/10/2014

		Ceda	r Ave			22n	d St			Ceda	r Ave			22n	d St		
		South	oound			Westl	oound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 12	2:00 PM	to 05:45	PM - Peak	1 of 1												
Peak Hour for Ent	ire Interse	ction Beg	gins at 04	:15 PM													
04:15 PM	0	22	10	32	0	3	4	7	4	14	0	18	7	3	4	14	71
04:30 PM	0	23	4	27	0	3	5	8	2	18	0	20	12	1	2	15	70
04:45 PM	0	26	9	35	1	4	2	7	1	18	0	19	8	2	3	13	74
05:00 PM	0	23	8	31	0	1	5	6	1	16	0	17	11	1	6	18	72
Total Volume	0	94	31	125	1	11	16	28	8	66	0	74	38	7	15	60	287
% App. Total	0	75.2	24.8		3.6	39.3	57.1		10.8	89.2	0		63.3	11.7	25		
PHF	.000	.904	.775	.893	.250	.688	.800	.875	.500	.917	.000	.925	.792	.583	.625	.833	.970



626.991.7522

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File Name : Cedar_22nd_BP

Site Code : 00000000 Start Date : 6/10/2014

Page No : 1

Groups Printed- Bank 1

			Oroup	, i i iiitcu- Daiii	7 1				
	Cedar Av	re	22nd St		Cedar Av	re	22nd St		
	Southboun		Westboun		Northbou	nd	Eastbound		
Start Time	Thru	Peds	Thru	Peds	Thru	Peds	Thru	Peds	Int. Total
07:00 AM	1	0	1	1	0	1	1	1	6
07:30 AM	2	0	1	0	0	2	0	1	6
07:45 AM	0	0	3	4	0	0	4	3	14_
Total	3	0	5	5	0	3	5	5	26
		_ 1	_	. 1	_	- 1		- 1	_
08:00 AM	0	0	0	1	0	0	1	0	2
08:15 AM	0	0	2	2	0	0	2	2	8
08:30 AM	0	0	2	0	0	0	0	2	4
08:45 AM	0	1	0	1	1	0	1	0	4
Total	0	1	4	4	1	0	4	4	18
04:00 PM	1	1	0	1	1	1	1	0	6
04:15 PM	0	0	1	2	0	0	2	1	6
04:30 PM	0	1	1	0	1	0	0	1	4
04:45 PM	0	0	0	3	0	0	3	0	6_
Total	1	2	2	6	2	1	6	2	22
05.1579.6			0	ا م	0	. 1	0	ا م	0
05:15 PM	4	0	0	0	0	4	0	0	8
05:30 PM	3	0	5	2	0	3	2	5	20
05:45 PM	<u>0</u> 7	0	5	6	0	7	4	0	8
Total	/	0	5	6	0	/	6	5	36
Grand Total	11	3	16	21	3	11	21	16	102
Apprch %	78.6	21.4	43.2	56.8	21.4	78.6	56.8	43.2	
Total %	10.8	2.9	15.7	20.6	2.9	10.8	20.6	15.7	

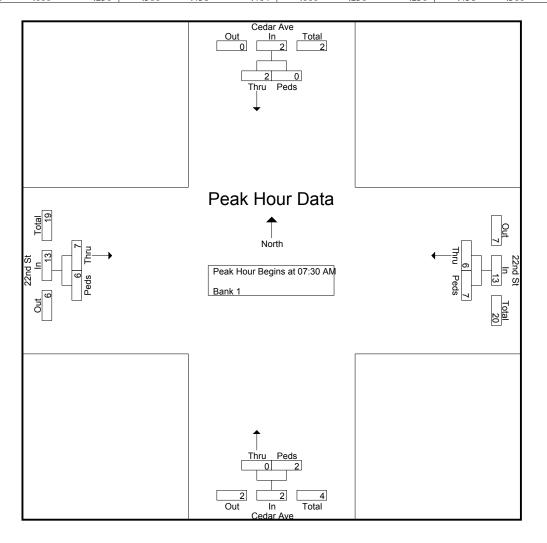
626.991.7522

www.ctcounters.com

File Name: Cedar_22nd_BP

Site Code : 00000000Start Date : 6/10/2014

	(Cedar Ave			22nd St			Cedar Av	e		22nd St		
	So	uthbound		V	Vestboun	d	ľ	Northbour	ıd	:	Eastbound		
Start Time	Thru	Peds A	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 07:00 A	M to 11:45	AM - Peak	1 of 1									
Peak Hour for Entire I	ntersection I	Begins at 07	:30 AM										
07:30 AM	2	0	2	1	0	1	0	2	2	0	1	1	6
07:45 AM	0	0	0	3	4	7	0	0	0	4	3	7	14
08:00 AM	0	0	0	0	1	1	0	0	0	1	0	1	2
08:15 AM	0	0	0	2	2	4	0	0	0	2	2	4	8
Total Volume	2	0	2	6	7	13	0	2	2	7	6	13	30
% App. Total	100	0		46.2	53.8		0	100		53.8	46.2		
PHF	.250	.000	.250	.500	.438	.464	.000	.250	.250	.438	.500	.464	.536



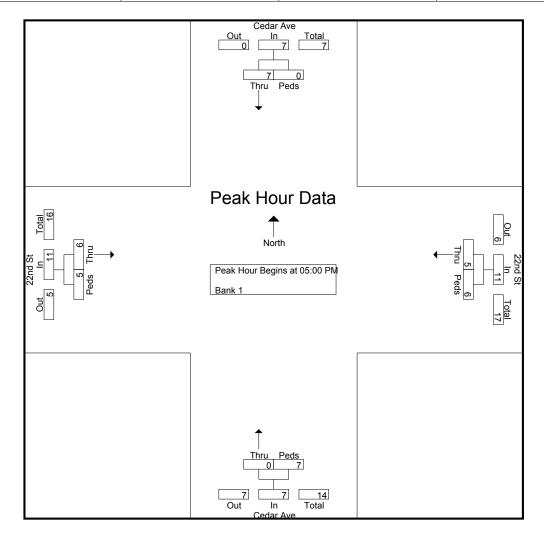
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File Name : Cedar_22nd_BP

Site Code : 00000000 Start Date : 6/10/2014

	(Cedar Ave			22nd St			Cedar Av	e		22nd St		
	So	outhbound		V	Vestboun	d	N	Northbour	nd	F	Eastbound	1	
Start Time	Thru	Peds A	pp. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 12:00 P	M to 05:45 P	M - Peak	l of 1									
Peak Hour for Entire I	ntersection I	Begins at 05:	00 PM										
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	4	0	4	0	0	0	0	4	4	0	0	0	8
05:30 PM	3	0	3	5	2	7	0	3	3	2	5	7	20
05:45 PM	0	0	0	0	4	4	0	0	0	4	0	4	8_
Total Volume	7	0	7	5	6	11	0	7	7	6	5	11	36
% App. Total	100	0		45.5	54.5		0	100		54.5	45.5		
PHF	.438	.000	.438	.250	.375	.393	.000	.438	.438	.375	.250	.393	.450



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File Name : Meadows_Marine Site Code : 00000000

Site Code : 00000000 Start Date : 6/10/2014

Page No : 1

Groups Printed- Unshifted

						Groups I	rinted- U	nshifted						
		Me	adows Ave		M	arine Ave		Mea	adows Ave		M	arine Ave		
		So	uthbound		W	estbound		No	rthbound		Ea	stbound		
	Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
	07:00 AM	32	2	5	4	115	11	21	1	2	4	70	6	273
	07:15 AM	2	0	11	6	101	4	51	0	5	1	71	6	258
	07:30 AM	3	1	7	13	137	1	23	0	23	1	89	8	306
	07:45 AM	2	11	2	9	167	2	48	0	11	3	169	6	420_
	Total	39	4	25	32	520	18	143	1	41	9	399	26	1257
	08:00 AM	2	0	2	7	199	1	55	1	11	0	152	7	437
	08:15 AM	3	0	4	6	147	1	75	1	14	2	107	12	372
	08:30 AM	0	0	9	14	180	7	43	0	5	5	87	17	367
	08:45 AM	3	0	10	10	185	1	48	0	10	2	92	23	384
	Total	8	0	25	37	711	10	221	2	40	9	438	59	1560
	04:00 PM	2	0	5	14	130	6	15	0	7	4	120	11	314
	04:15 PM	2	0	4	26	133	0	17	1	8	8	161	25	385
	04:30 PM	2	0	0	23	151	1	16	0	4	7	164	19	387
	04:45 PM	2	11	4	17	170	1	16	0	5	4	165	18	403
	Total	8	1	13	80	584	8	64	1	24	23	610	73	1489
	05:00 PM	1	0	1	30	125	2	18	0	4	2	204	17	404
	05:15 PM	1	0	2	29	132	0	5	0	4	6	179	21	379
	05:30 PM	3	1	2	22	174	3	6	0	8	4	180	18	421
	05:45 PM	2	0	2	25	162	1	12	0	3	4	188	25	424
	Total	7	1	7	106	593	6	41	0	19	16	751	81	1628
G	Grand Total	62	6	70	255	2408	42	469	4	124	57	2198	239	5934
	Apprch %	44.9	4.3	50.7	9.4	89	1.6	78.6	0.7	20.8	2.3	88.1	9.6	
	Total %	1	0.1	1.2	4.3	40.6	0.7	7.9	0.1	2.1	1	37	4	

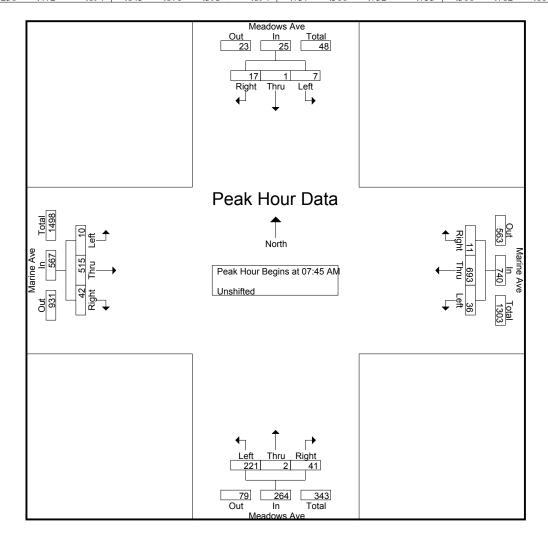
626.991.7522

www.ctcounters.com

File Name: Meadows_Marine

Site Code : 00000000 Start Date : 6/10/2014

		Meadov	vs Ave			Mari	ne Ave			Meado	ws Ave			Mari	ne Ave		
		Southb	ound			Westl	bound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 0'	7:00 AM	to 11:45	AM - Peak	1 of 1		_				_				_		
Peak Hour for Ent	ire Interse	ction Beg	gins at 07	7:45 AM													
07:45 AM	2	1	2	5	9	167	2	178	48	0	11	59	3	169	6	178	420
08:00 AM	2	0	2	4	7	199	1	207	55	1	11	67	0	152	7	159	437
08:15 AM	3	0	4	7	6	147	1	154	75	1	14	90	2	107	12	121	372
08:30 AM	0	0	9	9	14	180	7	201	43	0	5	48	5	87	17	109	367
Total Volume	7	1	17	25	36	693	11	740	221	2	41	264	10	515	42	567	1596
% App. Total	28	4	68		4.9	93.6	1.5		83.7	0.8	15.5		1.8	90.8	7.4		
PHF	.583	.250	.472	.694	.643	.871	.393	.894	.737	.500	.732	.733	.500	.762	.618	.796	.913



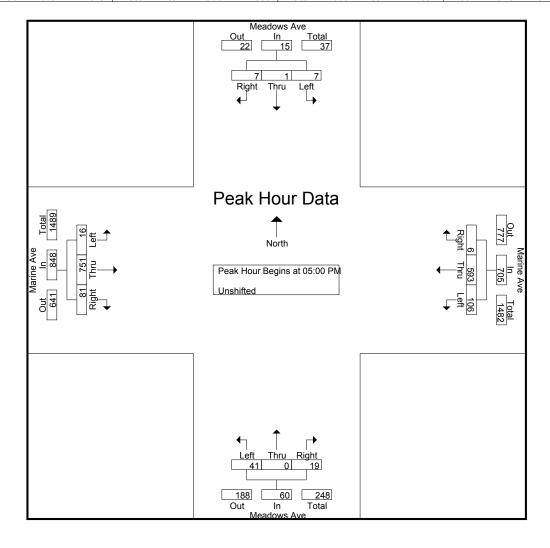
626.991.7522

www.ctcounters.com

File Name: Meadows_Marine

Site Code : 00000000 Start Date : 6/10/2014

		Meadov	ws Ave			Mari	ne Ave			Meado	ws Ave			Mari	ne Ave		
		Southb	ound			Westl	oound			North	bound			Easth	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	is From 12	2:00 PM	to 05:45 I	PM - Peak	1 of 1												
Peak Hour for Ent	ire Interse	ction Beg	gins at 05:	:00 PM													
05:00 PM	1	0	1	2	30	125	2	157	18	0	4	22	2	204	17	223	404
05:15 PM	1	0	2	3	29	132	0	161	5	0	4	9	6	179	21	206	379
05:30 PM	3	1	2	6	22	174	3	199	6	0	8	14	4	180	18	202	421
05:45 PM	2	0	2	4	25	162	1	188	12	0	3	15	4	188	25	217	424
Total Volume	7	1	7	15	106	593	6	705	41	0	19	60	16	751	81	848	1628
% App. Total	46.7	6.7	46.7		15	84.1	0.9		68.3	0	31.7		1.9	88.6	9.6		
PHF	.583	.250	.875	.625	.883	.852	.500	.886	.569	.000	.594	.682	.667	.920	.810	.951	.960



626.991.7522

www.ctcounters.com

File Name : Meadows_Marine_BP Site Code : 00000000

Site Code : 00000000 Start Date : 6/10/2014

Page No : 1

Groups Printed- Bank 1

			Groups	rrinteu- Dani	\ 1				
	Meadows A	ve	Marine Av	ve	Meadows A	ve	Marine Av	re	
	Southboun	ıd	Westboun	d	Northbour	nd	Eastbound	l	
Start Time	Thru	Peds	Thru	Peds	Thru	Peds	Thru	Peds	Int. Total
07:00 AM	0	1	0	0	0	0	0	0	1
		·							
07:30 AM	1	0	0	1	0	1	0	0	3
07:45 AM	11	3	0	0	0	1	11	2	8
Total	2	4	0	1	0	2	1	2	12
08:00 AM	1	0	0	0	1	0	0	0	2
08:15 AM	0	0	0	3	0	0	0	0	3
08:30 AM	0	0	0	3	1	2	0	0	6
08:45 AM	0	0	0	1	0	0	0	0	1_
Total	1	0	0	7	2	2	0	0	12
0.4.5.77.5				ا م		ا ه			
04:15 PM	2	0	0	0	1	0	0	0	3
04:30 PM	1	1	0	1	0	0	0	0	3
m . 1	2	1		1		0		0	
Total	3	1	0	1	1	0	0	0	6
05:00 PM	1	2	0	3	1	1	0	0	8
05:00 FM 05:15 PM	0	0	0	0	1	0	0	0	0
05:30 PM	0	0	0	1	0	0	0	0	1
05:45 PM	0	0	0	0	0	1	0	2	3
Total	1	2	0	4	2	2	0	2	13
Total	1	2	O	7	2	2	· ·	2	13
Grand Total	7	7	0	13	5	6	1	4	43
Apprch %	50	50	ő	100	45.5	54.5	20	80	.5
Total %	16.3	16.3	0	30.2	11.6	14	2.3	9.3	
						'		1	

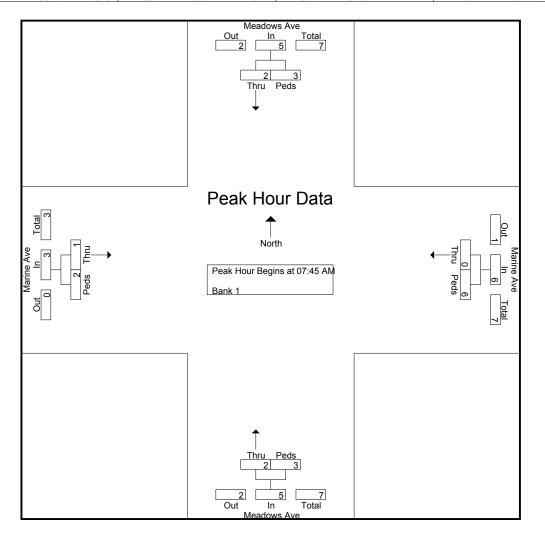
626.991.7522

www.ctcounters.com

File Name : Meadows_Marine_BP Site Code : 00000000

Start Date : 6/10/2014

	Mo	eadows Ave	:	N	Marine Av	ve	Mo	eadows A	ve		Marine Av	ve	
	So	outhbound		V	Vestboun	d	N	orthbour	ıd		Eastbound	ì	
Start Time	Thru	Peds A	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 07:00 A	M to 11:45	AM - Peak	1 of 1									
Peak Hour for Entire I	Intersection 1	Begins at 07	:45 AM										
07:45 AM	1	3	4	0	0	0	0	1	1	1	2	3	8
08:00 AM	1	0	1	0	0	0	1	0	1	0	0	0	2
08:15 AM	0	0	0	0	3	3	0	0	0	0	0	0	3
08:30 AM	0	0	0	0	3	3	1	2	3	0	0	0	6_
Total Volume	2	3	5	0	6	6	2	3	5	1	2	3	19
% App. Total	40	60		0	100		40	60		33.3	66.7		
PHF	.500	.250	.313	.000	.500	.500	.500	.375	.417	.250	.250	.250	.594



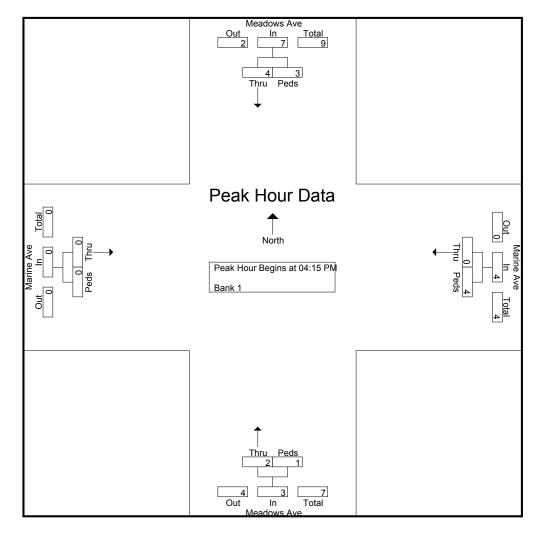
626.991.7522

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File Name : Meadows_Marine_BP Site Code : 00000000

Start Date : 6/10/2014

	M	eadows Av	e	1	Marine A	ve	M	leadows A	ve	I	Marine Av	ve	
	S	outhbound		1	Westboun	d	1	Northbour	nd]	Eastbound	1	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis F	rom 12:00 F	M to 05:45	PM - Peak	1 of 1									
Peak Hour for Entire I	ntersection	Begins at 04	4:15 PM										
04:15 PM	2	0	2	0	0	0	1	0	1	0	0	0	3
04:30 PM	1	1	2	0	1	1	0	0	0	0	0	0	3
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	1	2	3	0	3	3	1	1	2	0	0	0	8
Total Volume	4	3	7	0	4	4	2	1	3	0	0	0	14
% App. Total	57.1	42.9		0	100		66.7	33.3		0	0		
PHF	.500	.375	.583	.000	.333	.333	.500	.250	.375	.000	.000	.000	.438



CITY TRAFFIC COUNTERS 626.991.7522 www.ctcounters.com

22nd St Bt Sepulveda & Cedar

Start	11-Jun-14	W	est		Totals		ast	Hour	Totals		ed Totals
Time	Wed	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoor
12:00		0	4			2	9				
12:15		0	6			3	10				
12:30		0	5			1	10				
12:45		0	5	0	20	2	16	8	45	8	65
01:00		0	6			1	14				
01:15		1	10			1	20				
01:30		0	3			0	12				
01:45		0	6	1	25	0	12	2	58	3	83
02:00		0	8			0	12				
02:15		0	4			1	12				
02:30		1	3			4	9				
02:45		0	6	1	21	0	12	5	45	6	66
03:00		0	8			0	16				
03:15		0	6			0	20				
03:30		0	7			0	10				
03:45		0	4	0	25	0	15	0	61	0	86
04:00		0	5			0	11				
04:15		0	8			2	16				
04:30		0	6			0	12				
04:45		0	3	0	22	0	14	2	53	2	75
05:00		0	0			2	16				
05:15		0 2	3			2	14				
05:30		2	3			4	12				
05:45		1	7	5	13	1	11	9	53	14	66
06:00		1	1	-		1	8				
06:15		1	1			5	8				
06:30		3	1			4	11				
06:45		1	4	6	7	2	6	12	33	18	40
07:00		1	1	-		2	1				
07:15		3	1			4	10				
07:30		7	3			6	12				
07:45		6	4	17	9	5	13	17	36	34	45
08:00		5	2			10	11	• •	00	0.1	
08:15		10	4			10	10				
08:30		6	3			8	7				
08:45		8	5	29	14	6	7	34	35	63	49
09:00		8	2	20		12	5	0.	00	00	
09:15		4	0			16	0				
09:30		8	2			6	4				
09:45		7	0	27	4	8	2	42	11	69	1
10:00		4	3		-	8	4	72		00	
10:15		4	0			4	5				
10:30		2	1			9	4				
10:30		2 6	0	16	4	7	2	28	15	44	19
11:00		3	3	10	4	9	3	20	13	44	18
11:15			0			12					
11:13		8 5				14	3				
11:45		6	0 2	22	5	8	2	43	11	65	1,
		124	169	22	ວ	202		43	11	326	16
Total							456				625
Percent		42.3%	57.7%			30.7%	69.3%			34.3%	65.7%
Grand		124	169			202	456			326	62
Total											
Percent		42.3%	57.7%			30.7%	69.3%			34.3%	65.7%

ADT

ADT 2,656

AADT 2,656

CITY TRAFFIC COUNTERS 626.991.7522 www.ctcounters.com

Cedar Ave Bt 21st St & 22nd St

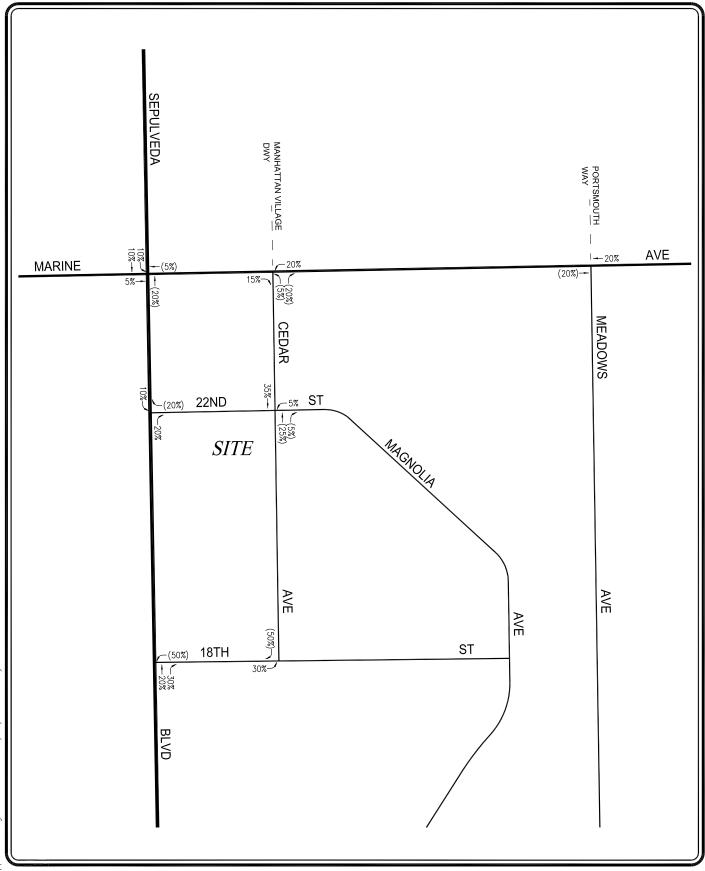
Start	11-Jun-14		uth	Hour	Totals		orth		Totals	Combine	
Time	Wed	Morning	Afternoon								
12:00		1	28			4	44				
12:15		2	19			2	32				
12:30		4	26			6	36				
12:45		2	25	9	98	4	38	16	150	25	248
01:00		0	18			0	54				
01:15		0	14			0	20				
01:30		1	22			1	40				
01:45		0	20	1	74	0	28	1	142	2	216
02:00		2	17			6	46				
02:15		0	24			0	41				
02:30		0	13			1	30				
02:45		0	16	2	70	0	24	7	141	9	211
03:00		2	26	_	, 0	2	37	•		Ū	
03:15		0	30			0	37				
03:30		1	24			1	34				
03:45		0	22	3	102	0	32	3	140	6	242
04:00		2	24	3	102	3	20	3	140	U	272
04:00		2	24			2	21				
04:13		0	23			0	20				
04:45		0	28	4	99	0	19	5	80	9	179
				4	99			5	80	9	179
05:00		0	28			0	18				
05:15		0	24			1	24				
05:30		3	31	_		4	18	_			
05:45		0	24	3	107	4	14	9	74	12	181
06:00		6	35			6	19				
06:15		4	18			7	17				
06:30		1	17			4	17				
06:45		0	21	11	91	4	22	21	75	32	166
07:00		2 7	22			3	10				
07:15		7	21			9	14				
07:30		5	20			14	14				
07:45		6	14	20	77	12	14	38	52	58	129
08:00		10	12			28	10				
08:15		9	8			24	14				
08:30		21	10			47	9				
08:45		18	5	58	35	32	8	131	41	189	76
09:00		12	8	00	00	35	3	101	• •	100	, ,
09:15		18	7			31	4				
09:30		16	7			34	8				
09:45		16	7	62	29	23	4	123	19	185	48
10:00		23		02	29	33		123	19	100	40
		14	3			24	5				
10:15			3				8				
10:30		17	2 7	70	4.5	24	2	440	40	400	
10:45		16	7	70	15	32	4	113	19	183	34
11:00		13	2			34	1				
11:15		15	1			30	1				
11:30		17	1			32	1				
11:45		24	3	69	7	38	3	134	6	203	13
Total		312	804			601	939			913	1743
Percent		28.0%	72.0%			39.0%	61.0%			34.4%	65.6%
Grand		312	804			601	939			913	1743
Total											1743
Percent		28.0%	72.0%			39.0%	61.0%			34.4%	65.6%

APPENDIX B

EXISTING USES TRIP DISTRIBUTION WEEKDAY AM AND PM PEAK HOURS

LINSCOTT, LAW & GREENSPAN, engineers

LLG Ref. 1-14-4083-1





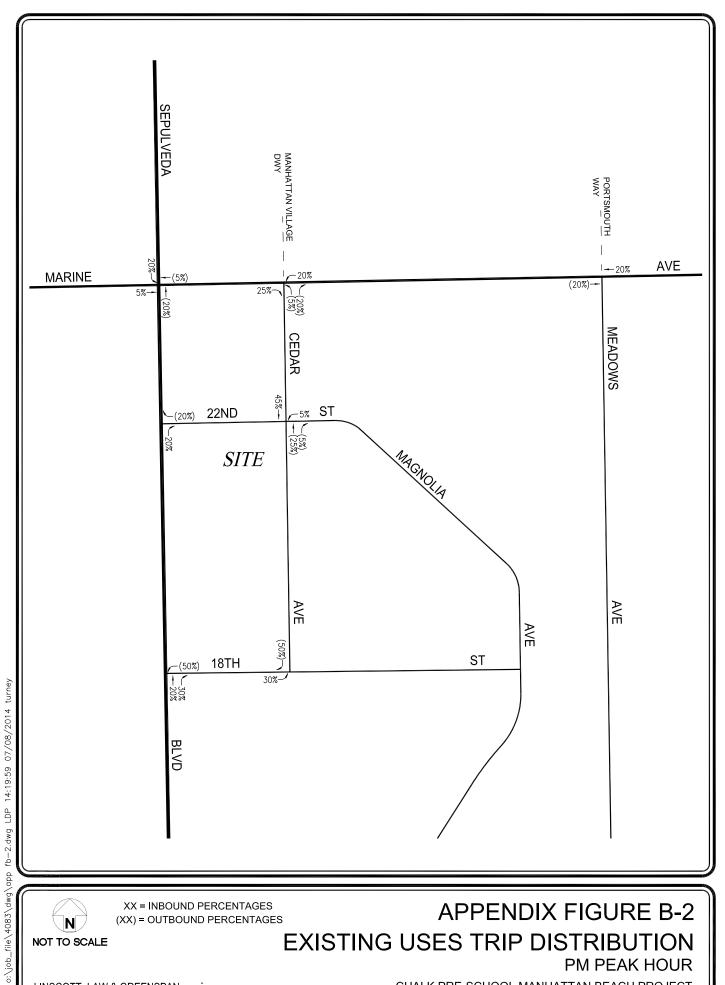
LINSCOTT, LAW & GREENSPAN, engineers

XX = INBOUND PERCENTAGES

(XX) = OUTBOUND PERCENTAGES

APPENDIX FIGURE B-1 EXISTING USES TRIP DISTRIBUTION AM PEAK HOUR

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT





XX = INBOUND PERCENTAGES

(XX) = OUTBOUND PERCENTAGES

APPENDIX FIGURE B-2 EXISTING USES TRIP DISTRIBUTION PM PEAK HOUR

LINSCOTT, LAW & GREENSPAN, engineers

CHALK PRE-SCHOOL MANHATTAN BEACH PROJECT

APPENDIX C

ICU/HCM AND LEVELS OF SERVICE EXPLANATION

ICU/HCM DATA WORKSHEETS – WEEKDAY AM AND PM PEAK HOURS

LINSCOTT, LAW & GREENSPAN, engineers

LLG Ref. 1-14-4083-1

INTERSECTION CAPACITY UTILIZATION (ICU) DESCRIPTION

Level of Service is a term used to describe prevailing conditions and their effect on traffic. Broadly interpreted, the Levels of Service concept denotes any one of a number of differing combinations of operating conditions which may occur as a roadway is accommodating various traffic volumes. Level of Service is a qualitative measure of the effect of such factors as travel speed, travel time, traffic interruptions, freedom to maneuver, safety, driving comfort and convenience.

Six Levels of Service, A through F, have been defined in the current version of the *Highway Capacity Manual*, published by the Transportation Research Board. Level of Service A describes a condition of free flow, with low traffic volumes and relatively high speeds, while Level of Service F describes forced traffic flow at low speeds with jammed conditions and queues which cannot clear during the green phases.

The Intersection Capacity Utilization (ICU) method of intersection capacity analysis has been used in our studies. It directly relates traffic demand and available capacity for key intersection movements, regardless of present signal timing, The capacity per hour of green time for each approach is calculated based on the methods of the *Highway Capacity Manual*. The proportion of total signal time needed by each key movement is determined and compared to the total time available (100 percent of the hour). The result of summing the requirements of the conflicting key movements plus an allowance for clearance times is expressed as a decimal fraction. Conflicting key traffic movements are those opposing movements whose combined green time requirements are greatest.

The resulting ICU represents the proportion of the total hour required to accommodate intersection demand volumes if the key conflicting traffic movements are operating at capacity. Other movements may be operating near capacity, or may be operating at significantly better levels. The ICU may be translated to a Level of Service as tabulated below.

The Levels of Service (abbreviated from the *Highway Capacity Manual*) are listed here with their corresponding ICU and Load Factor equivalents. Load Factor is that proportion of the signal cycles during the peak hour which are fully loaded; i.e. when all of the vehicles waiting at the beginning of green are not able to clear on that green phase.

Intersect	ion Capacity Utilization Char	acteristics
Level of Service	Load Factor	Equivalent ICU
A	0.0	0.00 - 0.60
В	0.0 - 0.1	0.61 - 0.70
C	0.1 - 0.3	0.71 - 0.80
D	0.3 - 0.7	0.81 - 0.90
E	0.7 - 1.0	0.91 - 1.00
F	Not Applicable	Not Applicable

SERVICE LEVEL A

There are no loaded cycles and few are even close to loaded at this service level. No approach phase is fully utilized by traffic and no vehicle waits longer than one red indication.

SERVICE LEVEL B

This level represents stable operation where an occasional approach phase is fully utilized and a substantial number are approaching full use. Many drivers begin to feel restricted within platoons of vehicles.

SERVICE LEVEL C

At this level stable operation continues. Loading is still intermittent but more frequent than at Level B. Occasionally drivers may have to wait through more than one red signal indication and backups may develop behind turning vehicles. Most drivers feel somewhat restricted, but not objectionably so.

SERVICE LEVEL D

This level encompasses a zone of increasing restriction approaching instability at the intersection. Delays to approaching vehicles may be substantial during short peaks within the peak hour, but enough cycles with lower demand occur to permit periodic clearance of queues, thus preventing excessive backups. Drivers frequently have to wait through more than one red signal. This level is the lower limit of acceptable operation to most drivers.

SERVICE LEVEL E

This represents near capacity and capacity operation. At capacity (ICU = 1.0) it represents the most vehicles that the particular intersection can accommodate. However, full utilization of every signal cycle is seldom attained no matter how great the demand. At this level all drivers wait through more than one red signal, and frequently through several.

SERVICE LEVEL F

Jammed conditions. Traffic backed up from a downstream location on one of the street restricts or prevents movement of traffic through the intersection under consideration.

Marine Avenue

N-S St:

E-W St:

Project: File:

LINSCOTT, LAW & GREENSPAN, ENGINEERS 600 S. Lake Avenue, Suite 500, Pasadena CA 91106 (626) 796.2322 Fax (626) 792.0941

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Sepulveda Boulevard @ Marine Avenue

Peak hr: Annual Growth: 1.00% Date: 07/08/2014 2014 Date of Count: Projection Year: 2015

	2014	EXIST. TRA	AFFIC	2014	EXISTING	PLUS PRO	JECT	2014	EXIST. W	PROJECT +	MITIGATIO	2015	FUTURE V	/ITHOUT PI	ROJECT	2015	FUTURE \	NITH PROJE	CT	2015	FUTURE V	//PROJECT	+ MITIGAT
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	25	1600	0.016	0	25	1600	0.016	0	25	1600	0.016	0	25	1600		0	25		0.016	0	25	1600	0.016
Nb Thru	2582	4800	0.548 *	4	2586	4800	0.549 *	0	2586	4800	0.549 *	86	2694	4800	0.572 *	4	2698	4800	0.573 *	0	2698	4800	0.573 *
Nb Right	48	0	-	0	48	0	-	0	48	0	-	3	51	0	-	0	51	0	-	0	51	0	-
Sb Left	178	2880	0.062 *	4	182	2880	0.063 *	0	182	2880	0.063 *	8	188	2880	0.065 *	4	192	2880	0.067 *	0	192	2880	0.067 *
Sb Thru	908	4800	0.189	4	912	4800	0.190	0	912	4800	0.190	78	995	4800	0.207	4	999	4800	0.208	0	999	4800	0.208
Sb Right	66	1600	0.041	0	66	1600	0.041	0	66	1600	0.041	0	67	1600	0.042	0	67	1600	0.042	0	67	1600	0.042
Eb Left	85	1600	0.053 *	0	85	1600	0.053 *	0	85	1600	0.053 *	0	86	1600	0.054 *	0	86	1600	0.054 *	0	86	1600	0.054 *
Eb Thru	321	3200	0.112	3	324	3200	0.113	0	324	3200	0.113	6	330	3200	0.115	3	333	3200	0.116	0	333	3200	0.116
Eb Right	38	0	-	0	38	0	-	0	38	0	-	0	38	0	-	0	38	0	-	0	38	0	-
Wb Left	128	1600	0.080	0	128	1600	0.080	0	128	1600	0.080	7	136	1600	0.085	0	136	1600	0.085	0	136	1600	0.085
Wb Thru	296	1600	0.185 *	2	298	1600	0.186 *	0	298	1600	0.186 *	1	300	1600	0.187 *	2	302	1600	0.189 *	0	302	1600	0.189 *
Wb Right [3	293	1600	0.121	5	298	1600	0.123	0	298	1600	0.123	12	308	1600	0.127	5	313	1600	0.129	0	313	1600	0.129
Yellow Allow	/ance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.948 E				0.951 E				0.951 E				0.978 E			E	0.982				0.982 E

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters

Capacity expressed in veh/hour of green
 The westbound right-turn lane has an overlapping phase with the southbound left-turn phase.

Marine Avenue

ICU1

N-S St:

E-W St:

Project: File:

LINSCOTT, LAW & GREENSPAN, ENGINEERS 600 S. Lake Avenue, Suite 500, Pasadena CA 91106 (626) 796.2322 Fax (626) 792.0941

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Sepulveda Boulevard @ Marine Avenue

Peak hr: Annual Growth: 1.00%

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	DJECT	2014	EXIST. W	PROJECT +	MITIGATIO	2015	FUTURE W	ITHOUT PI	ROJECT	2015	FUTURE V	VITH PROJE	CT	2015	FUTURE V	V/PROJECT	+ MITIGAT
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	97	1600	0.061 *	0	97	1600	0.061 *	0	97	1600	0.061 *	0	98	1600	0.061 *	0	98	1600	0.061 *	0	98	1600	0.061 *
Nb Thru	1372	4800	0.309	2	1374	4800	0.309	0	1374	4800	0.309	113	1499	4800	0.338	2	1501	4800	0.338	0	1501	4800	0.338
Nb Right	109	0	-	0	109	0	-	0	109	0	-	12	122	0	-	0	122	0	-	0	122	0	-
Sb Left	208	2880	0.072	6	214	2880	0.074	0	214	2880	0.074	15	225	2880	0.078	6	231	2880	0.080	0	231	2880	0.080
Sb Thru	2077	4800	0.433 *	0	2077	4800	0.433 *	0	2077	4800	0.433 *	106	2204	4800	0.459 *	0	2204	4800	0.459 *	0	2204	4800	0.459 *
Sb Right	121	1600	0.076	0	121	1600	0.076	0	121	1600	0.076	-1	121	1600	0.076	0	121	1600	0.076	0	121	1600	0.076
Eb Left	76	1600	0.048	0	76	1600	0.048	0	76	1600	0.048	1	78	1600	0.049	0	78	1600	0.049	0	78	1600	0.049
Eb Thru	272	3200	0.103 *	1	273	3200	0.104 *	0	273	3200	0.104 *	2	277	3200	0.105 *	1	278	3200	0.105 *	0	278	3200	0.105 *
Eb Right	59	0	-	0	59	0	-	0	59	0	-	0	60	0	-	0	60	0	-	0	60	0	-
Wb Left	205	1600	0.128 *	0	205	1600	0.128 *	0	205	1600	0.128 *	2	209	1600	0.131 *	0	209	1600	0.131 *	0	209	1600	0.131 *
Wb Thru	268	1600	0.168	2	270	1600	0.169	0	270	1600	0.169	4	275	1600	0.172	2	277	1600	0.173	0	277	1600	0.173
Wb Right [3 141	1600	0.016	5	146	1600	0.017	0	146	1600	0.017	14	156	1600	0.020	5	161	1600	0.021	0	161	1600	0.021
Yellow Allo	wance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.825 D				0.825 D				0.825 D				0.856 D			[0.856				0.856 D

^{*} Key conflicting movement as a part of ICU

¹ Counts conducted by City Traffic Counters
2 Capacity expressed in veh/hour of green
3 The westbound right-turn lane has an overlapping phase with the southbound left-turn phase.

22nd Street

N-S St:

E-W St:

Project: File:

LINSCOTT, LAW & GREENSPAN, ENGINEERS 600 S. Lake Avenue, Suite 500, Pasadena CA 91106 (626) 796.2322 Fax (626) 792.0941

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Sepulveda Boulevard @ 22nd Street

Peak hr: Annual Growth: 1.00%

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	DJECT	2014	EXIST. W	PROJECT +	MITIGATIO	2015	FUTURE V	/ITHOUT P	ROJECT	2015	FUTURE V	VITH PROJE	ЕСТ	2015	FUTURE V	V/PROJECT	+ MITIGATION
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Nb Thru	2640	4800	0.551 *	0	2640	4800	0.553 *	Ō	2640		0.553 *	89	2755	4800	0.575 *	0	2755		0.577 *	0	2755	4800	0.577 *
Nb Right	6	0	-	9	15	0	-	0	15	0	-	0	6	0	-	9	15	0	-	0	15	0	-
Sb Left [3]	7	1600	0.004 *	4	11	1600	0.007 *	0	11	1600	0.007 *	0	7	1600	0.004 *	4	11	1600	0.007 *	0	11	1600	0.007 *
Sb Thru	999	4800	0.208	0	999	4800	0.208	0	999	4800	0.208	86	1095	4800	0.228	0	1095	4800	0.228	0	1095	4800	0.228
Sb Right	0	0	-	0	0	0	-	0	0		-	0	0	0	-	0	0	0	-	0	0	0	-
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Eb Right	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000
Wb Left [3]	1	0	0.001	0	1	0	0.001	0	1	0	0.001	0	1	0	0.001	0	1	0	0.001	0	1	0	0.001
Wb Thru	0	1600	0.007 *	0	0	1600	0.009 *	0	0	1600	0.009 *	0	0	1600	0.007 *	0	0	1600	0.009 *	0	0	1600	0.009 *
Wb Right	10	0	-	4	14	0	-	0	14	0	-	0	10	0	-	4	14	0	-	0	14	0	-
Yellow Allow	ance:		0.100 *	ı			0.100 *				0.100 *	I			0.100 *				0.100 *	ı			0.100 *
ICU LOS			0.663 B				0.669 B				0.669 B				0.687 B			E	0.694 3				0.694 B

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters

² Capacity expressed in veh/hour of green 3 No Left-Turn, Mon-Fri 3-7 PM.

22nd Street

ICU2

N-S St:

E-W St:

Project: File:

LINSCOTT, LAW & GREENSPAN, ENGINEERS 600 S. Lake Avenue, Suite 500, Pasadena CA 91106 (626) 796.2322 Fax (626) 792.0941

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Sepulveda Boulevard @ 22nd Street

Peak hr: Annual Growth: 1.00% Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PR	DJECT	2014	EXIST. W	PROJECT +	MITIGATION	2015	FUTURE W	/ITHOUT PI	ROJECT	2015	FUTURE \	NITH PROJE	CT	2015	FUTURE \	V/PROJECT	+ MITIGAT
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *
Nb Thru	1705		0.360	ا آ	1705			0	1705		0.361	124	1846	4800	0.389	0	1846		0.390	0	1846	4800	0.390
Nb Right	21	0	-	6	27			ő	27		-	0	21	0	-	6	27		-	ő	27	0	
Sb Left [3]	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004
Sb Thru	2307	4800	0.482 *	0	2307	4800	0.482 *	0	2307	4800	0.482 *	107	2437	4800	0.509 *	0	2437	4800	0.509 *	0	2437	4800	0.509 *
Sb Right	6	0	-	0	6	0	-	0	6	0	-	0	6	0	-	0	6	0	=	0	6	0	-
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Eb Right	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000	0	0	1600	0.000
Wb Left [3]	3	0	0.002	0	3	0	0.002	0	3	0	0.002	0	3	0	0.002	0	3	0	0.002	0	3	0	0.002
Wb Thru	0	1600	0.013 *	0	0	1600	0.014 *	0	0	1600	0.014 *	0	0	1600	0.013 *	0	0	1600	0.015 *	0	0	1600	0.015 *
Wb Right	18	0	-	2	20	0	-	0	20	0	-	0	18	0	-	2	20	0	-	0	20	0	-
Yellow Allov	wance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.595 A				0.596 A				0.596 A				0.622 B				0.623				0.623 B

^{*} Key conflicting movement as a part of ICU

¹ Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green 3 No Left-Turn, Mon-Fri 3-7 PM.

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INTERSECTION CAPACITY UTILIZATION

Sepulveda Boulevard @ 18th Street

Peak hr: Annual Growth: 1.00%

N-S St: Sepulveda Boulevard E-W St: 18th Street

Project: File: Chalk Pre-School Manhattan Beach Project/1-14-4083-1

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PR	DJECT	2014	EXIST. W	PROJECT 4	MITIGATIO	2015	FUTURE V	/ITHOUT PI	ROJECT	2015	FUTURE \	NITH PROJE	CT	2015	FUTURE V	W/PROJECT	+ MITIGATION
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000
Nb Thru	2593	4800	0.548 *	9	2602	4800	0.552 *	0	2602	4800	0.552 *	89	2708	4800	0.572 *	9	2717	4800	0.576 *	0	2717	4800	0.576 *
Nb Right	35	0	-	13	48	0	-	0	48	0	-	0	35	0	-	13	48	0	-	0	48	0	-
Sb Left [3]	39	1600	0.024 *	0	39	1600	0.024 *	0	39	1600	0.024 *	0	39	1600	0.025 *	0	39	1600	0.025 *	0	39	1600	0.025 *
Sb Thru	986	4800	0.207	0	986	4800	0.207	0	986	4800	0.207	86	1082	4800	0.227	0	1082	4800	0.227	0	1082	4800	0.227
Sb Right	9	0	-	0	9	0	-	0	9	0	-	0	9	0	-	0	9	0	-	0	9	0	- '
Eb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Eb Thru	0	0	0.000	0	0	0	0.000	0	0		0.000	0	0	0		0	0		0.000	0	0	0	0.000
Eb Right	13	1600	0.008 *	0	13	1600	0.008 *	0	13	1600	0.008 *	0	13	1600	0.008 *	0	13	1600	0.008 *	0	13	1600	0.008 *
Wb Left	48	0	0.030 *	21	69	0	0.043 *	0	69	0	0.043 *	0	48	0	0.030 *	21	69	0	0.043 *	0	69	0	0.043 *
Wb Thru	1	1600	0.034	0	1	1600	0.048	0	1	1600	0.048	0	1	1600	0.035	0	1	1600	0.048	0	1	1600	0.048
Wb Right [4	6	0	-	0	6	0	-	0	6	0	-	0	6	0	-	0	6	0	-	0	6	0	-
Yellow Allow	/ance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.710 C				0.728 C				0.728 C				0.735 C			(0.753				0.753 C

Date:

Date of Count:

Projection Year:

07/08/2014

2014

2015

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green 3 No Left-turn, Mon-Fri 3-7 PM. 4 No Right-turn on red.

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INTERSECTION CAPACITY UTILIZATION

Sepulveda Boulevard @ 18th Street

Peak hr: Annual Growth: 1.00%

N-S St: Sepulveda Boulevard E-W St: 18th Street

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

Project: File: ICU3

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	JECT	2014	EXIST. W	PROJECT +	MITIGATIO	2015	FUTURE V	ITHOUT P	ROJECT	2015	FUTURE V	WITH PROJE	CT	2015	FUTURE \	W/PROJECT	+ MITIGATI
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Nb Thru	1542	4800	0.331	6	1548	4800	0.335	0	1548	4800	0.335	124	1681	4800	0.361	6	1687		0.364	0	1687		0.364
Nb Right	49	0	-	9	58	0	-	0	58	0	-	0	49	0	-	9	58	0	-	0	58	0	-
Sb Left [3]	3	1600	0.002	0	3	1600	0.002	0	3	1600	0.002	0	3	1600	0.002	0	3	1600	0.002	0	3	1600	0.002
Sb Thru	2344	4800	0.490 *	0	2344	4800	0.490 *	0	2344	4800	0.490 *	107	2474	4800	0.517 *	0	2474	4800	0.517 *	0	2474	4800	0.517 *
Sb Right	7	0	-	0	7	0	-	0	7	0	-	0	7	0	-	0	7	0	-	0	7	0	-
Eb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Eb Right	5	1600	0.003 *	0	5	1600	0.003 *	0	5	1600	0.003 *	0	5	1600	0.003 *	0	5	1600	0.003 *	0	5	1600	0.003 *
Wb Left	68	0	0.043 *	18	86	0	0.054 *	0	86	0	0.054 *	0	69	0	0.043 *	18	87	0	0.054 *	0	87	0	0.054 *
Wb Thru	1	1600	0.045	0	1	1600	0.056	0	1	1600	0.056	0	1	1600	0.045	0	1	1600	0.057	0	1	1600	0.057
Wb Right [4	3	0	-	0	3	0	-	0	3	0	-	0	3	0	-	0	3	0	-	0	3	0	-
Yellow Allov	vance:		0.100 *				0.100 *				0.100 *	•			0.100 *				0.100 *				0.100 *
ICU LOS			0.635 B				0.647 B				0.647 B				0.663 B			E	0.674				0.674 B

Date:

Date of Count:

Projection Year:

07/08/2014

2014

2015

^{*} Key conflicting movement as a part of ICU

¹ Counts conducted by City Traffic Counters
2 Capacity expressed in veh/hour of green
3 No Left-turn, Mon-Fri 3-7 PM.
4 No Right-turn on red.

N-S St:

E-W St:

Project: File:

Cedar Avenue

Marine Avenue

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Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Cedar Avenue @ Marine Avenue

Peak hr: 1.00% Annual Growth:

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	JECT	2014	EXIST. W/I	PROJECT +	MITIGATIO	2015	FUTURE V	/ITHOUT PI	ROJECT	2015	FUTURE V	VITH PROJE	CT	2015	FUTURE V	V/PROJECT	+ MITIGATION
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	35	0	0.022	7	42		0.026	0	42	0	0.026		35	0	0.022	7	42	0	0.026	0	42	0	0.026
Nb Thru	66	1600	0.079 *	, ,	66		0.028 *	0	66	1600	0.020	0	67	1600	0.022	'n	67	1600	0.020	0	67		0.020
Nb Right	25	0	-	8	33		-	0	33	0	-	0	25	0	-	8	33		-	0	33		-
Sb Left	92	0	0.058	0	92		0.058	0	92	0	0.058	_	93	0	0.058	0	93	0	0.058	0	93	0	0.058
Sb Leit Sb Thru	27	1600	0.036	0	27		0.036	0	27	1600	0.036	0	27	1600	0.036	0	93 27	1600	0.036	0	93 27	1600	0.036
Sb Right	55	1600	0.074	0	55		0.074	0	55	1600	0.074	0	57	1600	0.075	0	57		0.075	0	57	1600	0.075
SD FIGUR	33	1600	0.034	"	55	1600	0.034	"	55	1600	0.034	'	37	1600	0.033		37	1600	0.033	0	37	1600	0.033
Eb Left	57	1600	0.036 *	0	57	1600	0.036 *	0	57	1600	0.036 *	0	58	1600	0.036 *	0	58	1600	0.036 *	0	58	1600	0.036 *
Eb Thru	378	3200	0.125	0	378	3200	0.127	0	378	3200	0.127	17	399	3200	0.131	0	399	3200	0.133	0	399	3200	0.133
Eb Right	21	0	-	7	28	0	-	0	28	0	-	0	21	0	-	7	28	0	-	0	28	0	-
Wb Left	33	1600	0.021	9	42	1600	0.026	0	42	1600	0.026	0	33	1600	0.021	9	42	1600	0.026	0	42	1600	0.026
Wb Thru	599	3200	0.187	0	599		0.187	l ö	599	3200	0.187	20	625	3200	0.195	0	625		0.195	0	625		0.195
Wb Right	319	1600		0	319		0.199 *	ō	319	1600	0.199 *	0	322	1600	0.201 *	0	322		0.201 *	0	322		0.201 *
Yellow Allow	ance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.471 A				0.481 A				0.481 A				0.475 A			A	0.484 A				0.484 A

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green

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INTERSECTION CAPACITY UTILIZATION

Cedar Avenue @ Marine Avenue

Peak hr: 1.00% Annual Growth:

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

N-S St: Cedar Avenue E-W St: Marine Avenue

Project: File: Chalk Pre-School Manhattan Beach Project/1-14-4083-1

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	JECT	2014	EXIST. W	PROJECT +	MITIGATIO	2015	FUTURE V	VITHOUT PI	ROJECT	2015	FUTURE \	NITH PROJE	:CT	2015	FUTURE V	V/PROJECT	+ MITIGATIO
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	27	0	0.017	7	34	0	0.021	0	34	0	0.021	0	27	0	0.017	7	34	0	0.021	0	34	0	0.021
Nb Thru	32	1600	0.058 *	0	32	1600	0.067 *	0	32	1600	0.067 *	1	33	1600	0.059 *	0	33	1600	0.068 *	0	33	1600	0.068 *
Nb Right	34	0	-	7	41	0	-	0	41	0	-	0	34	0	-	7	41	0	-	0	41	0	-
Sb Left	340	0	0.213 *	0	340	0	0.213 *	0	340	0	0.213 *	-5	338	0	0.212 *	0	338	0	0.212 *	0	338	0	0.212 *
Sb Thru	51	1600	0.244	0	51	1600	0.244	0	51	1600	0.244	-1	51	1600	0.243	0	51	1600	0.243	0	51	1600	0.243
Sb Right	185	1600	0.116	0	185	1600	0.116	0	185	1600	0.116	-8	179	1600	0.112	0	179	1600	0.112	0	179	1600	0.112
Eb Left	51	1600	0.032	0	51	1600	0.032	0	51	1600	0.032	5	57	1600	0.035	0	57	1600	0.035	0	57	1600	0.035
Eb Thru	526	3200	0.175 *	0	526	3200	0.178 *	0	526	3200	0.178 *	24	555	3200	0.185 *	0	555	3200	0.187 *	0	555	3200	0.187 *
Eb Right	35	0	-	7	42	0	-	0	42	0	-	0	35	0	-	7	42	0	-	0	42	0	-
Wb Left	33	1600	0.021 *	6	39	1600	0.024 *	0	39	1600	0.024 *	0	33	1600	0.021 *	6	39		0.025 *	0	39	1600	0.025 *
Wb Thru	413	3200	0.129	0	413	3200	0.129	0	413	3200	0.129	28	445	3200	0.139	0	445	3200	0.139	0	445	3200	0.139
Wb Right	198	1600	0.124	0	198	1600	0.124	0	198	1600	0.124	1	201	1600	0.126	0	201	1600	0.126	0	201	1600	0.126
Yellow Allo	wance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.567 A				0.581 A				0.581 A				0.576 A			ļ	0.591				0.591 A

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green

Cedar Avenue

N-S St:

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INTERSECTION CAPACITY UTILIZATION

Cedar Avenue @ 22nd Street Peak hr:

1.00% Annual Growth:

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

E-W St: 22nd Street Project: File: Chalk Pre-School Manhattan Beach Project/1-14-4083-1

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	DJECT	2014	EXIST. W/	PROJECT +	MITIGATIO	2015	FUTURE V	/ITHOUT PI	ROJECT	2015	FUTURE V	VITH PROJE	СТ	2015	FUTURE \	V/PROJECT	+ MITIGATIO
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	6	0	0.004 *	5	11	0	0.007 *	0	11	0	0.007 *	0	6	0	0.004 *	5	11	0	0.007 *	0	11	0	0.007 *
Nb Thru	65	1600	0.044	15	80	1600	0.058	0	80	1600	0.058	0	66	1600	0.045	15	81	1600	0.059	0	81	1600	0.059
Nb Right	0	0	-	2	2	0	-	0	2	0	-	0	0	0	-	2	2	0	-	0	2	0	-
Sb Left	8	0	0.005	0	8	0	0.005	0	8	0	0.005	0	8	0	0.005	0	8	0	0.005	0	8	0	0.005
Sb Thru	65	1600	0.065 *	16	81	1600	0.075 *	0	81	1600	0.075 *	0	66	1600	0.066 *	16	82	1600	0.076 *	0	82	1600	0.076 *
Sb Right	31	0	-	0	31	0	-	0	31	0	-	0	31	0	-	0	31	0	-	0	31	0	-
Eb Left	23	0	0.014 *	0	23	0	0.014 *	0	23	0	0.014 *	0	23	0	0.015 *	0	23	0	0.015 *	0	23	0	0.015 *
Eb Thru	2	1600	0.019	0	2	1600	0.028	0	2	1600	0.028	0	2	1600	0.019	0	2		0.028	0	2	1600	0.028
Eb Right	5	0	-	15	20	0	-	0	20	0	-	0	5	0	-	15	20	0	-	0	20	0	-
Wb Left	2	0	0.001	3	5	0	0.003	0	5	0	0.003	0	2	0	0.001	3	5	0	0.003	0	5	0	0.003
Wb Thru	19	1600	0.031 *	0	19	1600	0.033 *	0	19	1600	0.033 *	0	19	1600	0.031 *	0	19		0.033 *	0	19	1600	0.033 *
Wb Right	28	0	-	0	28	0	-	0	28	0	-	0	28	0	-	0	28	0	-	0	28	0	-
Yellow Allow	ance:		0.100 *	•			0.100 *				0.100 *				0.100 *	'			0.100 *	'			0.100 *
ICU LOS			0.214 A				0.229 A				0.229 A				0.215 A			,	0.230 A				0.230 A

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green

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INTERSECTION CAPACITY UTILIZATION

1.00%

Cedar Avenue @ 22nd Street Peak hr:

Annual Growth:

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

N-S St: Cedar Avenue E-W St: 22nd Street

Project: File: Chalk Pre-School Manhattan Beach Project/1-14-4083-1

ICU5

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	JECT	2014	EXIST. W	PROJECT +	MITIGATIO	2015	FUTURE W	/ITHOUT PI	ROJECT	2015	FUTURE V	WITH PROJE	СТ	2015	FUTURE \	V/PROJECT	+ MITIGATI
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	8	0	0.005 *	5	13	0	0.008 *	0	13	0	0.008 *	0	8	0	0.005 *	5	13	0	0.008 *	0	13	0	0.008 *
Nb Thru	66	1600	0.046	14	80	1600	0.059	0	80	1600	0.059	1	68	1600	0.047	14	82	1600	0.060	0	82	1600	0.060
Nb Right	0	0	-	2	2	0	-	0	2	0	-	0	0	0	-	2	2	0	-	0	2	0	-
Sb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Sb Thru	94	1600	0.078 *	13	107	1600	0.086 *	0	107	1600	0.086 *	-1	94	1600	0.078 *	13	107	1600	0.086 *	0	107	1600	0.086 *
Sb Right	31	0	-	0	31	0	-	0	31	0	-	0	31	0	-	0	31	0	-	0	31	0	-
Eb Left	38	0	0.024 *	0	38	0	0.024	0	38	0	0.024	0	38	0	0.024 *	0	38	0	0.024	0	38	0	0.024
Eb Thru	7	1600	0.038	0	7	1600	0.043 *	0	7		0.043 *	0	7	1600	0.038	0	7	1600	0.044 *	0	7	1600	0.044 *
Eb Right	15	0	-	9	24	0	-	0	24	0	-	0	15	0	-	9	24	0	-	0	24	0	-
Wb Left	1	0	0.001	1	2	0	0.001 *	0	2	0	0.001 *	0	1	0	0.001	1	2	0	0.001 *	0	2	0	0.001 *
Wb Thru	11	1600	0.018 *	0	11	1600	0.018	0	11	1600	0.018	0	11	1600	0.018 *	0	11	1600	0.018	0	11	1600	0.018
Wb Right	16	0	-	0	16	0	-	0	16	0	-	0	16	0	-	0	16	0	-	0	16	0	-
Yellow Allov	vance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.224 A				0.239 A				0.239 A				0.225 A			ļ	0.239 A				0.239 A

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green

Meadows Avenue

Marine Avenue

N-S St:

E-W St:

Project: File:

LINSCOTT, LAW & GREENSPAN, ENGINEERS 600 S. Lake Avenue, Suite 500, Pasadena CA 91106 (626) 796.2322 Fax (626) 792.0941

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Meadows Avenue @ Marine Avenue

Peak hr: Annual Growth:

1.00%

Date:

Date of Count:

Projection Year:

07/08/2014

2014

2015

	2014	EXIST. TR	AFFIC	2014	EXISTING P	LUS PRO	JECT	2014	EXIST. W	PROJECT +	- MITIGATIO	2015	FUTURE V	/ITHOUT PI	ROJECT	2015	FUTURE V	VITH PROJE	СТ	2015	FUTURE V	V/PROJECT	+ MITIGATIO
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume C	apacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	221	0	0.138	0	221	0	0.138		221	0	0.138	0	223	0	0.140	0	223	0	0.140	_	223	0	0.140
Nb Thru	2	1600	0.165 *	0	2	1600	0.165 *	0	221	-	0.165 *	0	223	1600		0	223	1600	0.170 *	0	223		0.140
Nb Right	41	0	-	0	41	0	-	0	41		-	6	47	0	-	0	47	0	-	0	47	0	-
Sb Left	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *
Sb Thru	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000
Sb Right	17	1600	0.011	0	17	1600	0.011	0	17	1600	0.011	0	17	1600	0.011	0	17	1600	0.011	0	17	1600	0.011
Eb Left	10	1600	0.006 *	0	10	1600	0.006 *	0	10		0.006 *	0	10	1600	0.006 *	0	10	1600	0.006 *	0	10	1600	0.006 *
Eb Thru	515	3200	0.174	8	523	3200	0.177	0			0.177	17	537	3200	0.181	8	545	3200	0.184	0	545		0.184
Eb Right	42	0	-	0	42	0	-	0	42	. 0	-	0	42	0	-	0	42	0	-	0	42	0	-
Wb Left	36	1600	0.023	0	36	1600	0.023	0	36		0.023	1	37	1600		0	37	1600	0.023	0	37	1600	0.023
Wb Thru	693	3200	0.220 *	9	702	3200	0.223 *	0			0.223 *	20	720	3200	0.228 *	9	729	3200	0.231 *	0	729	3200	0.231 *
Wb Right	11	0	-	0	11	0	-	0	11	0	-	0	11	0	-	0	11	0	-	0	11	0	-
Yellow Allow	ance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.496 A				0.498 A				0.498 A				0.510 A			F	0.512 A				0.512 A

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green

N-S St:

E-W St:

Project: File:

Meadows Avenue

Marine Avenue

ICU6

LINSCOTT, LAW & GREENSPAN, ENGINEERS 600 S. Lake Avenue, Suite 500, Pasadena CA 91106 (626) 796.2322 Fax (626) 792.0941

Chalk Pre-School Manhattan Beach Project/1-14-4083-1

INTERSECTION CAPACITY UTILIZATION

Meadows Avenue @ Marine Avenue

Peak hr: Annual Growth: 1.00%

Date: 07/08/2014 Date of Count: 2014 Projection Year: 2015

	2014	EXIST. TR	AFFIC	2014	EXISTING	PLUS PRO	DJECT	2014	EXIST. W	PROJECT +	MITIGATION	2015	FUTURE \	VITHOUT P	ROJECT	2015	FUTURE V	VITH PROJE	CT	2015	FUTURE V	V/PROJECT	+ MITIGAT
	1	2	V/C	Added	Total		V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
Movement	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	41	0	0.026	0	41	0	0.026	0	41	0	0.026	0	41	0	0.026	0	41	0	0.026	0	41	0	0.026
Nb Thru	0	1600	0.038 *	0	0	1600	0.038 *	0	0	1600	0.038 *	0	0	1600	0.039 *	0	0	1600	0.039 *	0	0	1600	0.039 *
Nb Right	19	0	-	0	19	0	-	0	19	0	-	1	20	0	-	0	20	0	-	0	20	0	-
Sb Left	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *	0	7	1600	0.004 *
Sb Thru	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000	0	1	0	0.000
Sb Right	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004	0	7	1600	0.004
Eb Left	16	1600	0.010	0	16	1600	0.010	0	16	1600	0.010	0	16	1600	0.010	0	16	1600	0.010	0	16	1600	0.010
Eb Thru	751	3200	0.260 *	7	758	3200	0.262 *	0	758	3200	0.262 *	19	778	3200	0.269 *	7	785	3200	0.271 *	0	785	3200	0.271 *
Eb Right	81	0	-	0	81	0	-	0	81	0	-	0	82	0	-	0	82	0	-	0	82	0	-
Wb Left	106	1600	0.066 *	0	106	1600	0.066 *	0	106		0.066 *	5	112	1600	0.070 *	0	112		0.070 *	0	112		0.070 *
Wb Thru	593	3200	0.187	6	599	3200	0.189	0	599	3200	0.189	29	628	3200	0.198	6	634	3200	0.200	0	634	3200	0.200
Wb Right	6	0	=	0	6	0	=	0	6	0	=	0	6	0	=	0	6	0	-	0	6	0	-
Yellow Allo	wance:		0.100 *				0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU LOS			0.468 A				0.470 A				0.470 A				0.481 A				0.484 A				0.484 A

^{*} Key conflicting movement as a part of ICU 1 Counts conducted by City Traffic Counters 2 Capacity expressed in veh/hour of green

LEVEL OF SERVICE FOR UNSIGNALIZED INTERSECTIONS

In the *Highway Capacity Manual (HCM)*, published by the Transportation Research Board, 2010, level of service for unsignalized intersections is defined in terms of delay, which is a measure of driver discomfort, frustration, fuel consumption, and lost travel time. The delay experienced by a motorist is made up of a number of factors that relate to control, geometrics, traffic, and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions, in the absence of incidents, control, traffic, or geometric delay. Only the portion of total delay attributed to the traffic control measures, either traffic signals or stop signs, is quantified. This delay is called *control delay*. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay.

Level of Service criteria for unsignalized intersections are stated in terms of the average control delay per vehicle. The level of service is determined by the computed or measured control delay and is defined for each minor movement. Average control delay for any particular minor movement is a function of the service time for the approach and the degree of utilization. (Level of service is not defined for the intersection as a whole for two-way stop controlled intersections.)

Level of Service Criteria f	for TWSC/AWSC Intersections
Level of Service	Average Control Delay (Sec/Veh)
A	≤ 10
В	$> 10 \text{ and} \le 15$
C	$> 15 \text{ and } \le 25$
D	$> 25 \text{ and} \le 35$
E	$> 35 \text{ and} \le 50$
F	> 50

Level of Service (LOS) values are used to describe intersection operations with service levels varying from LOS A (free flow) to LOS F (jammed condition). The following descriptions summarize *HCM* criteria for each level of service:

- LOS A describes operations with very low control delay, up to 10 seconds per vehicle.
- LOS B describes operations with control delay greater than 10 and up to 15 seconds per vehicle.
- LOS C describes operations with control delay greater than 15 and up to 25 seconds per vehicle.
- LOS D describes operations with control delay greater than 25 and up to 35 seconds per vehicle.
- LOS E describes operations with control delay greater than 35 and up to 50 seconds per vehicle.

LOS F describes operations with control delay in excess of 50 seconds per vehicle. For two-way stop controlled intersections, LOS F exists when there are insufficient gaps of suitable size to allow side-street demand to safely cross through a major-street traffic stream. This level of service is generally evident from extremely long control delays experienced by side-street traffic and by queuing on the minor-street approaches.

Intersection												
Int Delay, s/veh 0.5												
,												
Movement	EBL	EBT	EBR	WB	L WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0		1 0		0		6	7	999	C
Conflicting Peds, #/hr	0	0	0		0 0		0	0	0	0	0	C
Sign Control	Stop	Stop	Stop	Sto	p Stop	Stop	Free	Free	Free	Free	Free	
RT Channelized	-	-	None			None	-		None	-	_	None
Storage Length	-	-	0			_	-	-	-	55	-	-
Veh in Median Storage, #	-	0	-		- 0	-	-	0	-	-	0	-
Grade, %	-	0	-		- 0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	Ç	3 93	93	93	93	93	93	93	93
Heavy Vehicles, %	2	2	2		2 2	2	2	2	2	2	2	2
Mymt Flow	0	0	0		1 0	11	0	2839	6	8	1074	
Major/Minor N	Inor2			Mino	.1		Major1			Major2		
	2224	2024	537			1423		0	0		0	C
Conflicting Flow All		1089	331	328 284			1074	-	-	2845	-	
Stage 1	1135	2845		44		-						
Stage 2	6.44	6.54	7.14	6.4			5.34	-	-	5.34	-	
Critical Hdwy	7.34	5.54	7.14	7.3			3.34		-	3.34	-	
Critical Hdwy Stg 1 Critical Hdwy Stg 2	6.74	5.54	-	6.7			-	_	-		-	
Follow-up Hdwy	3.82	4.02	3.92	3.8			3.12	-	-	3.12	-	
Pot Cap-1 Maneuver	3.82	4.02	418	3.0	9 3		360		_	3.12	_	
Stage 1	173	290	410		9 37	107	300		_	40	_	
Stage 1 Stage 2	194	37	-	51			-	_	_	-	_	
Platoon blocked, %	174	31	-	<i>J</i> 1	4 290	-		_	-	_		
Mov Cap-1 Maneuver	36	2	418		8 2	107	360	_	_	46	_	
Mov Cap-2 Maneuver	36	2	410		8 2		300		-	-		
Stage 1	173	240	_		9 37		_		_	_	_	
Stage 2	175	37		42				_		-	_	
Stage 2	173	31	_	72	.5 240	_	_	-	_	-	-	_
Approach	EB			W			NB			SB		
HCM Control Delay, s	0			Ò	8		0			0.7		
HCM LOS	A				F							
Minor Lane/Major Mvmt	NBL	NBT	NBREI	BLn W BLr	1 SBL	SBT	SBR					
Capacity (veh/h)	360	-	-	- 5	0 46	-	-					
HCM Lane V/C Ratio	-	-	-	- 0.23	7 0.164	-	-					
HCM Control Delay (s)	0	-	-	0 9	8 98	-	-					
HCM Lane LOS	A	-	-	A	F F	-	-					

Intersection												
Int Delay, s/veh 0.	7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	1	0	14	0	2640	15	11	999	C
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	C
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-		None	-	_	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	-	-	-	55	-	-
Veh in Median Storage, #	-	0	_	-	0	-	-	0	_	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	1	0	15	0	2839	16	12	1074	0
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	2233	3053	537	3300	3945	1427	1074	0	0	2855	0	0
Stage 1	1098	1098	<i>331</i>	2847	2847	1427	1074	-	U	2633	U	U
Stage 2	1135	2855	_	453	1098	_		_	_	-	_	-
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	_	_	5.34	_	_
Critical Hdwy Stg 1	7.34	5.54	7.17	7.34	5.54	7.17	-	_	_	-	_	
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	_	_	_	_	_	
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	_	_	3.12	_	
Pot Cap-1 Maneuver	45	3	418	9.02	3	107	360	_	_	45	_	_
Stage 1	170	287	-	9	37	-	-	_	_	-	_	
Stage 2	194	37	_	508	287	_	_	_	_	_	_	_
Platoon blocked, %				200				_	_		_	_
Mov Cap-1 Maneuver	31	2	418	7	2	107	360	_	_	45	_	_
Mov Cap-2 Maneuver	31	2	-	7	2		-	-	-	-	-	
Stage 1	170	210	_	9	37	_	-	-	_	-	-	_
Stage 2	167	37	-	373	210	_	-	-	-	-	-	_
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			95.6			0			1.2		
HCM LOS	A			F			- U			1.2		
HEW LOS	71			1								
Minor Lane/Major Mymt	NRI	NRT	NRRE	BLn W BLn1	SBL	SBT	SRR					
Capacity (veh/h)	360	NDI	- NDINE	- 55	45	- 100	-					
HCM Lane V/C Ratio	300	-	-	- 0.293		-	<u>-</u>					
HCM Control Delay (s)	0	- -	_		111.4	_	- -					
HCM Lane LOS	A	_	-	A F	F	_	_					
HCM 95th %tile Q(veh)	0	_	_	- 1	0.9	_	-					
TICIVI 95th 70the Q(vell)	U	-	-	- 1	0.7	-	-					

Intersection												
).6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	1	0	10	0	2755	6	7	1095	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	-	-	-	55	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	1	0	11	0	2962	6	8	1177	0
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	2377	4161	589	3452	4158	1484	1177	0	0	2969	0	0
Stage 1	1192	1192	-	2966		-	-	-	_		-	-
Stage 2	1185	2969	_	486		_	_	_	_	_	-	_
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	_	_	5.34	_	_
Critical Hdwy Stg 1	7.34	5.54	_	7.34	5.54	_	_	-	_	_	-	_
Critical Hdwy Stg 2	6.74	5.54	-	6.74	5.54	-	-	_	_	-	-	_
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	-	-	3.12	-	_
Pot Cap-1 Maneuver	37	2	387	7	2	98	321	-	-	39	-	-
Stage 1	146	259	-	7	32	-	-	-	-	-	-	-
Stage 2	180	32	-	486	259	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	28	2	387	6	2	98	321	-	-	39	-	-
Mov Cap-2 Maneuver	28	2	-	6	2	-	-	-	-	-	-	-
Stage 1	146	206	-	7	32	-	-	-	-	-	-	-
Stage 2	160	32	-	386	206	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			125.1			0			0.8		
HCM LOS	A			F								
Minor Lane/Major Mvm	nt NRI	NBT	NBREE	Ln W BLn1	SBL	SBT	SBR					
Capacity (veh/h)	321	-	_	- 41	39	- DD 1	-					
HCM Lane V/C Ratio	-		_	- 0.288			-					
HCM Control Delay (s)	0	_	_	0.266		_	_					
HCM Lane LOS	A	_	_	A F	F	_	_					
HCM 95th %tile Q(veh)		_	_	- 1	0.6	_	_					
Tient /Jun /June Q(Ven)		_		1	0.0	•						

Intersection												
).9											
int Belay, 57 ven	,. <i>,</i>											
Movement	EBL	EBT	EBR	WBI.	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	1	0	14	0	2755	15	11	1095	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop		Stop	Free	Free	Free	Free	Free	Free
RT Channelized	- Stop	-	None	- Stop		None	-		None	-		None
Storage Length	-	-	0	_	-	-	_	_	-	55	-	-
Veh in Median Storage,	# -	0	-	_	0	_	_	0	_	-	0	_
Grade, %	-	0	-	-	0	-	-	0	-	-	0	_
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	1	0	15	0	2962	16	12	1177	0
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	2386	4170	589		4171	1490	1177	0	0	2978	0	0
Stage 1	1201	1201	309	2970		1409	11//	-	-	2916	-	U
Stage 2	1185	2978	-	495	1201	_	-	_	-	-	_	-
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	_	-	5.34	_	_
Critical Hdwy Stg 1	7.34	5.54	7.14	7.34	5.54	7.14	J.J 4	_	_	J.J4 -	_	-
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	_	_	_	_	_	_
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	_		3.12		
Pot Cap-1 Maneuver	36	2	387	7.02	2	97	321	_	_	39	_	
Stage 1	144	256	-	7	32	-	321	_	_	-	_	
Stage 2	180	32	_	480	256	_	_	_	_	_	_	_
Platoon blocked, %	100	32		100	230			_	_		_	_
Mov Cap-1 Maneuver	23	1	387	5	1	97	321	_	_	39	_	_
Mov Cap-2 Maneuver	23	1	-	5	1	-	-	_	_	-	-	_
Stage 1	144	177	_	7	32	_	_	_	_	_	_	-
Stage 2	152	32	_	332	177	_	_	-	_	-	-	_
Approach	ЕВ			WB			NB			SB		
HCM Control Delay, s	0			128.2			0			1.3		
HCM LOS	A			120.2 F			U			1.5		
HCWI LOS	A			Г								
M: 1 26: 15	, NDI	NIDE	NID PET	N WITH A	ODI	(IDE	CDD					
Minor Lane/Major Mvm				BLnWBLn1	SBL	SBT						
Capacity (veh/h)	321	-	-	- 44		-	-					
HCM Lane V/C Ratio	-	-	-	- 0.367		-	=					
HCM Control Delay (s)	0	-	-	0 128.2		-	-					
HCM Lane LOS	A	-	-	A F		-	-					
HCM 95th %tile Q(veh)	0	-	-	- 1.3	1	-	-					

Intersection												
Int Delay, s/veh 0.4	4											
·												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	3	0	18	0	1705	21	7	2307	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	C
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-		None	-	_	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	-	-	-	55	-	
Veh in Median Storage, #	<u> </u>	0	-	-	0	-	-	0	-	-	0	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	3	0	19	0	1795	22	7	2428	6
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All		4263	1217		4255	908	2435	0	0	1817	0	0
Stage 1	2446	2446	1217	1806	1806	-	2433	-	U	1017	U	U
Stage 2	718	1817	_	986		_	-	_	_	-	_	_
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	_	_	5.34	_	_
Critical Hdwy Stg 1	7.34	5.54	7.17	7.34	5.54	7.17	-	_	_	J.J .	_	
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	_	_	_	_	_	
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	_	_	3.12	_	_
Pot Cap-1 Maneuver	11	2	148	20	2	239	75	_	_	155	_	_
Stage 1	18	61	-	53	129		-	_	_	-	_	
Stage 2	351	128	_	240	60	_	_	_	_	_	_	_
Platoon blocked, %		120						_	_		_	_
Mov Cap-1 Maneuver	10	2	148	19	2	239	75	_	_	155	_	_
Mov Cap-2 Maneuver	10	2	-	19	2	-	-	_	_	-	_	-
Stage 1	18	58	_	53	129	_	_	_	_	_	_	
Stage 2	323	128	_	229	57	-	-	-	-	_	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			57.6			0			0.1		
HCM LOS	A			57.6 F			<u> </u>			0.1		
Helli Lob	7.1			•								
Minor Lane/Major Mvmt	NRI	NRT	NRRE	BLn W BLn1	SBL	SBT	SBR					
Capacity (veh/h)	75	-	-	- 90		<u>5D1</u>	-					
HCM Lane V/C Ratio	-	-		- 0.246		_	<u>-</u>					
HCM Control Delay (s)	0	- -	_		29.4	_	-					
HCM Lane LOS	A	_	_	A F	29.4 D	_	_					
HCM 95th %tile Q(veh)	0	_	_	- 0.9		_	-					
TICIVI 95tii /0tile Q(vell)	U	-	-	- 0.9	0.1	-	-					

Intersection												
Int Delay, s/veh).4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	3	0	20	0	1705	27	7		6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	- July -		None			None	-		None	-		None
Storage Length	_	_	0	_	_	-	_	_	-	55	-	-
Veh in Median Storage,	# -	0	-	_	0	_	_	0	_	-	0	_
Grade, %	-	0	-	_	0	_	_	0	_	_	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	0	0	0	3	0	21	0	1795	28	7	2428	6
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All		4269	1217	2795	4258	912	2435	0	0	1823	0	0
	2446	2446	1217	1809	1809	912	2433	-	-	1823	U	U
Stage 1 Stage 2	718	1823	-	986	2449	-	-	-	_	-	-	-
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	_	_	5.34	-	_
Critical Hdwy Stg 1	7.34	5.54	7.14	7.34	5.54	7.14	3.34			5.54		_
Critical Hdwy Stg 2	6.74	5.54	-	6.74	5.54	-	-	-	-	-	-	_
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	-	_	3.12	_	_
Pot Cap-1 Maneuver	11	2	148	20	2	237	75	_	_	154	_	
Stage 1	18	61	-	53	129	231	-	_	_	134	_	
Stage 2	351	127	_	240	60	_	_	_	_	_	_	
Platoon blocked, %	331	1,27		240	00		_	_	_		_	
Mov Cap-1 Maneuver	10	2	148	19	2	237	75	_	_	154	_	_
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	10	2	-	19	2	231	-	_	_	154	_	
Stage 1	18	58	_	53	129	_	_	_	_	_	_	
Stage 2	320	127	_	229	57	_	_	_	_	_	_	_
Stage 2	320	12/		22)	37							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			55.4			0			0.1		
HCM LOS	A			55.4 F			U			0.1		
TICM LOS	A			Г								
Minor Lane/Major Mvn	at NDI	NDT	MDDD	BLn W BLn1	SBL	SBT	SBR					
Capacity (veh/h)	75	-	-	- 95	154	-	-					
HCM Cantrol Polov (s)	-	-	-	- 0.255	29.5	-	-					
HCM Long LOS		-	-			-	-					
HCM O5th O(tile O(treb)	A	-	-	A F	D	-	-					
HCM 95th %tile Q(veh)) 0	-	-	- 0.9	0.1	-	-					

Intersection												
	.4											
J .												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	3	0	18	0	1846	21	7	2437	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	-	-	-	55	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	3	0	19	0	1943	22	7	2565	6
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	3360	4548	1286	2995	4540	983	2572	0	0	1965	0	0
Stage 1	2583	2583	_	1954		_	_	_	_	_	_	_
Stage 2	777	1965	-	1041	2586	-	-	-	-	-	-	-
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	-	_	5.34	-	_
Critical Hdwy Stg 1	7.34	5.54	-	7.34	5.54	_	-	-	-	-	-	-
Critical Hdwy Stg 2	6.74	5.54	-	6.74	5.54	_	-	-	-	-	-	-
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	-	-	3.12	-	-
Pot Cap-1 Maneuver	8	1	133	15	1	213	64	-	-	130	-	-
Stage 1	14	51	-	42	109	-	-	-	-	-	-	-
Stage 2	323	107	-	222	51	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	7	1	133	14	1	213	64	-	-	130	-	-
Mov Cap-2 Maneuver	7	1	-	14	1	-	-	-	-	-	-	-
Stage 1	14	48	-	42	109	-	-	-	-	-	-	-
Stage 2	294	107	-	210	48	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			78.6			0			0.1		
HCM LOS	A			F								
Minor Lane/Major Mvm	t NBL	NBT	NBREF	BLn W BLn1	SBL	SBT	SBR					
Capacity (veh/h)	64	_	-	- 70			-					
HCM Lane V/C Ratio	-	_	_	- 0.316		_	-					
HCM Control Delay (s)	0	_	_	0.316		_	_					
HCM Lane LOS	A	_	_	A F	D	_	-					
HCM 95th %tile Q(veh)		_	_	- 1.2	0.2	_	_					
	3			1.2	J. <u>_</u>							

Intersection												
Int Delay, s/veh 0.	5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	3	0	20		1846	27	7		6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	-	-	-	55	-	-
Veh in Median Storage, #	<u> </u>	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	3	0	21	0	1943	28	7	2565	6
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All		4555	1286	2998	4543	986	2572	0	0	1972	0	0
Stage 1	2583	2583	1200	1957	1957	700	2312	-	-	17/2	-	-
Stage 2	777	1972	_	1041	2586	_	_	_	_	_	_	_
Critical Hdwy	6.44	6.54	7.14	6.44	6.54	7.14	5.34	_	_	5.34	_	_
Critical Hdwy Stg 1	7.34	5.54	7.17	7.34	5.54	7,17	3.34	_	_	-	_	_
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	_	_	_	_	_	_
Follow-up Hdwy	3.82	4.02	3.92	3.82	4.02	3.92	3.12	_	_	3.12	_	_
Pot Cap-1 Maneuver	8	1	133	15	1	212	64	_	_	129	_	_
Stage 1	14	51	-	41	109	-	-	_	_	-	_	_
Stage 2	323	107	_	222	51	_	_	_	_	_	_	_
Platoon blocked, %	020	10,						_	_		_	_
Mov Cap-1 Maneuver	7	1	133	14	1	212	64	_	_	129	_	_
Mov Cap-2 Maneuver	7	1	-	14	1		-	_	_	-	_	_
Stage 1	14	48	_	41	109	_	_	_	_	_	_	_
Stage 2	291	107	_	210	48	_	_	-	_	_	_	_
3.11.61	_, _											
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			74.4			0			0.1		
HCM LOS	A			, F			- O			0.1		
Helli Lob	П			1								
Minor Lane/Major Mvmt	MDI	NDT	NID DIC	BLn W BLn1	SBL	SBT	CDD					
	64			- 75	129							
Capacity (veh/h) HCM Lane V/C Ratio		-	-	- 0.323		-	-					
	0	-	-			-	-					
HCM Control Delay (s) HCM Lane LOS			-			-						
HCM Lane LOS HCM 95th %tile Q(veh)	A	-	-	A F - 1.2		-	-					
ncivi 95th %the Q(ven)	0	-	-	- 1.2	0.2	-	-					

Intersection												
Intersection Delay, s/veh	7.6											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	23	2	5	0	2	19	28	0	6	65	0
						_			~	U	O.C	-
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Peak Hour Factor Heavy Vehicles, %	0.85	0.85	0.85	0.85	0.85	_		0.85	0.85	~		0.85
						0.85	0.85			0.85		0.85 2 0

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.7	7.4	7.7
HCM LOS	A	A	A

Lane	NBLn1	EBLn1 V	WBLn1	SBLn1	
Vol Left, %	8%	77%	4%	8%	
Vol Thru, %	92%	7%	39%	62%	
Vol Right, %	0%	17%	57%	30%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	71	30	49	104	
LT Vol	65	2	19	65	
Through Vol	0	5	28	31	
RT Vol	6	23	2	8	
Lane Flow Rate	84	35	58	122	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.098	0.044	0.065	0.136	
Departure Headway (Hd)	4.205	4.498	4.088	3.994	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	842	801	881	887	
Service Time	2.283	2.499	2.089	2.068	
HCM Lane V/C Ratio	0.1	0.044	0.066	0.138	
HCM Control Delay	7.7	7.7	7.4	7.7	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.3	0.1	0.2	0.5	

Lane

Intersection				
Intersection Delay, s/veh				
Intersection LOS				
Movement	SBU	SBL	SBT	SBR
Vol, veh/h	0	8	65	31
Peak Hour Factor	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2
Mvmt Flow	0	9	76	36
Number of Lanes	0	0	1	0
Approach		SB		
Opposing Approach		NB		
Opposing Lanes		1		
Conflicting Approach Left		WB		
Conflicting Lanes Left		1		
Conflicting Approach Righ	ıt	EB		
Conflicting Lanes Right		1		
HCM Control Delay		7.7		
HCM LOS		A		

Intersection												
Intersection Delay, s/veh	7.9											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	23	2	20	0		10	20	^	4.4	0.0	
, oi, , oii, ii	U	23	2	20	0	5	19	28	0	11	80	2
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
	~					-				0.85 2		0.85
Peak Hour Factor	~	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85			0.85 2 2

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.7	7.6	8
HCM LOS	A	A	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	12%	51%	10%	7%	
Vol Thru, %	86%	4%	37%	67%	
Vol Right, %	2%	44%	54%	26%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	93	45	52	120	
LT Vol	80	2	19	81	
Through Vol	2	20	28	31	
RT Vol	11	23	5	8	
Lane Flow Rate	109	53	61	141	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.129	0.065	0.072	0.16	
Departure Headway (Hd)	4.354	4.389	4.241	4.074	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	828	821	849	865	
Service Time	2.354	2.391	2.244	2.173	
HCM Lane V/C Ratio	0.132	0.065	0.072	0.163	
HCM Control Delay	8	7.7	7.6	8	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.4	0.2	0.2	0.6	

Intersection				
Intersection Delay, s/veh				
Intersection LOS				
Movement	SBU	SBL	SBT	SBR
Vol, veh/h	0	8	81	31
Peak Hour Factor	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2
Mvmt Flow	0	9	95	36
Number of Lanes	0	0	1	0
Approach		SB		
Opposing Approach		NB		
Opposing Lanes		1		
Conflicting Approach Left		WB		
Conflicting Lanes Left		1		
Conflicting Approach Right		EB		
Conflicting Lanes Right		1		
HCM Control Delay		8		
HCM LOS		A		
_				
Lane				

Intersection												
Intersection Delay, s/veh	7.7											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	23	2	5	0	2	19	28	0	6	66	0
						_			•	U	00	-
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Peak Hour Factor Heavy Vehicles, %	0.85	0.85	0.85	0.85	0.85	_		0.85		~		0.85
						0.85	0.85		0.85	0.85		0.85 2 0

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.7	7.4	7.8
HCM LOS	A	A	A

Lane	NBLn1	EBLn1 V	WBLn1	SBLn1	
Vol Left, %	8%	77%	4%	8%	
Vol Thru, %	92%	7%	39%	63%	
Vol Right, %	0%	17%	57%	30%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	72	30	49	105	
LT Vol	66	2	19	66	
Through Vol	0	5	28	31	
RT Vol	6	23	2	8	
Lane Flow Rate	85	35	58	124	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.099	0.044	0.066	0.137	
Departure Headway (Hd)	4.206	4.502	4.092	3.997	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	841	800	880	886	
Service Time	2.285	2.504	2.093	2.073	
HCM Lane V/C Ratio	0.101	0.044	0.066	0.14	
HCM Control Delay	7.8	7.7	7.4	7.7	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.3	0.1	0.2	0.5	

SBU	SBL	SBT	SBR
0	8	66	31
0.85	0.85	0.85	0.85
2	2	2	2
0	9	78	36
0	0	1	0
	SB		
	NB		
	1		
	WB		
	1		
t	EB		
	1		
	7.7		
	Α		
	0 0.85 2 0	0 8 0.85 0.85 2 2 0 9 0 0 SB NB 1 WB 1 EB 1 7.7	0 8 66 0.85 0.85 0.85 2 2 2 2 0 9 78 0 0 1 SB NB 1 WB 1 EB 1 7.7

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Intersection												
Intersection Delay, s/veh	7.9											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	23	2	20	0	5	19	28	0	11	81	2
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	27	2	24	0	6	22	33	0	13	95	2
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.7	7.6	8
HCM LOS	A	A	A

Lane	NBLn1	EBLn1 V	WBLn1	SBLn1	
Vol Left, %	12%	51%	10%	7%	
Vol Thru, %	86%	4%	37%	68%	
Vol Right, %	2%	44%	54%	26%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	94	45	52	121	
LT Vol	81	2	19	82	
Through Vol	2	20	28	31	
RT Vol	11	23	5	8	
Lane Flow Rate	111	53	61	142	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.131	0.065	0.072	0.161	
Departure Headway (Hd)	4.354	4.395	4.248	4.076	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	828	820	848	864	
Service Time	2.354	2.397	2.25	2.176	
HCM Lane V/C Ratio	0.134	0.065	0.072	0.164	
HCM Control Delay	8	7.7	7.6	8	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.4	0.2	0.2	0.6	

Lane

Intersection Delay, s/veh				
Intersection LOS				
Movement	SBU	SBL	SBT	SBR
Vol, veh/h	0	8	82	31
Peak Hour Factor	0.85	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2
Mvmt Flow	0	9	96	36
Number of Lanes	0	0	1	0
Approach		SB		
Opposing Approach		NB		
Opposing Lanes		1		
Conflicting Approach Left	t	WB		
Conflicting Lanes Left		1		
Conflicting Approach Rigi	ht	EB		
Conflicting Lanes Right		1		
TICK C . ID 1		8		
HCM Control Delay HCM LOS				

Intersection												
Intersection Delay, s/veh	7.7											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
X 7 - 1 1. /L.	^	• •										
Vol, veh/h	0	38	7	15	0	1	11	16	0	8	66	0
Peak Hour Factor	0.97	38 0.97	7 0.97	15 0.97	0.97	0.97	11 0.97	16 0.97	0 0.97	8 0.97	66 0.97	0.97
· · · · · · · · · · · · · · · · · · ·	~		7 0.97 2			1 0.97 2				_		~
Peak Hour Factor	~		7 0.97 2 7	0.97	0.97			0.97	0.97	0.97		~

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.7	7.3	7.7
HCM LOS	A	A	A

Lane	NBLn1	EBLn1 V	WBLn1	SBLn1	
Vol Left, %	11%	63%	4%	0%	
Vol Thru, %	89%	12%	39%	75%	
Vol Right, %	0%	25%	57%	25%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	74	60	28	125	
LT Vol	66	7	11	94	
Through Vol	0	15	16	31	
RT Vol	8	38	1	0	
Lane Flow Rate	76	62	29	129	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.089	0.074	0.033	0.143	
Departure Headway (Hd)	4.213	4.288	4.113	4.002	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	840	821	876	887	
Service Time	2.29	2.388	2.113	2.071	
HCM Lane V/C Ratio	0.09	0.076	0.033	0.145	
HCM Control Delay	7.7	7.7	7.3	7.7	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.3	0.2	0.1	0.5	

SBU	SBL	SBT	SBR
0	0	94	31
0.97	0.97	0.97	0.97
2	2	2	2
0	0	97	32
0	0	1	0
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Intersection												
Intersection Delay, s/veh	7.8											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	38	7	24	0	2	11	16	0	13	80	2
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	39	7	25	0	2	11	16	0	13	82	2
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.8	7.4	7.9
HCM LOS	A	A	A

Lane	NBLn1	EBLn1 V	WBLn1	SBLn1	
Vol Left, %	14%	55%	7%	0%	
Vol Thru, %	84%	10%	38%	78%	
Vol Right, %	2%	35%	55%	22%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	95	69	29	138	
LT Vol	80	7	11	107	
Through Vol	2	24	16	31	
RT Vol	13	38	2	0	
Lane Flow Rate	98	71	30	142	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.115	0.087	0.035	0.16	
Departure Headway (Hd)	4.234	4.394	4.223	4.05	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	833	820	852	872	
Service Time	2.328	2.395	2.226	2.138	
HCM Lane V/C Ratio	0.118	0.087	0.035	0.163	
HCM Control Delay	7.9	7.8	7.4	7.9	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.4	0.3	0.1	0.6	

T				
Intersection Delay, s/veh				
Intersection LOS				
Movement	SBU	SBL	SBT	SBR
Vol, veh/h	0	0	107	31
Peak Hour Factor	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	2	2
Mvmt Flow	0	0	110	32
Number of Lanes	0	0	1	0
Approach			SB	
Opposing Approach			NB	
Opposing Lanes			1	
Conflicting Approach Lef	ìt		WB	
Conflicting Lanes Left			1	
Conflicting Approach Rig	ght		EB	
Conflicting Lanes Right			1	
HCM Control Delay			7.9	
HCM LOS			A	

Intersection												
Intersection Delay, s/veh	7.7											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	38	7	15	0	1	11	16	0	8	68	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	39	7	15	0	1	11	16	0	8	70	0
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.7	7.3	7.7
HCM LOS	A	A	A

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	11%	63%	4%	0%	
Vol Thru, %	89%	12%	39%	75%	
Vol Right, %	0%	25%	57%	25%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	76	60	28	125	
LT Vol	68	7	11	94	
Through Vol	0	15	16	31	
RT Vol	8	38	1	0	
Lane Flow Rate	78	62	29	129	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.092	0.075	0.033	0.143	
Departure Headway (Hd)	4.213	4.393	4.118	4.004	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	840	820	874	885	
Service Time	2.291	2.393	2.119	2.076	
HCM Lane V/C Ratio	0.093	0.076	0.033	0.146	
HCM Control Delay	7.7	7.7	7.3	7.8	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.3	0.2	0.1	0.5	

SBU	SBL	SBT	SBR
0	0	94	31
0.97	0.97	0.97	0.97
2	2	2	2
0	0	97	32
0	0	1	0
		SB	
		NB	
		1	
		WB	
		1	
nt		EB	
		1	
		7.8	
		Α	
	0 0.97 2 0	0 0 0.97 0.97 2 2 0 0 0 0	0 0 94 0.97 0.97 0.97 2 2 2 2 0 0 97 0 0 1 SB NB 1 WB 1 EB 1 7.8

Intersection												
Intersection Delay, s/veh	7.8											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	38	7	24	0	2	11	16	0	13	82	2
D I II E .												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97 2	0.97 2	0.97	0.97 2
	0.97 2 0										0.97 2 85	

Approach	EB	WB	NB
Opposing Approach	WB	EB	SB
Opposing Lanes	1	1	1
Conflicting Approach Left	SB	NB	EB
Conflicting Lanes Left	1	1	1
Conflicting Approach Right	NB	SB	WB
Conflicting Lanes Right	1	1	1
HCM Control Delay	7.8	7.4	7.9
HCM LOS	A	A	A

Lane	NBLn1	EBLn1 V	WBLn1	SBLn1	
Vol Left, %	13%	55%	7%	0%	
Vol Thru, %	85%	10%	38%	78%	
Vol Right, %	2%	35%	55%	22%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	97	69	29	138	
LT Vol	82	7	11	107	
Through Vol	2	24	16	31	
RT Vol	13	38	2	0	
Lane Flow Rate	100	71	30	142	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.118	0.087	0.035	0.16	
Departure Headway (Hd)	4.234	4.4	4.229	4.052	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	833	819	851	872	
Service Time	2.328	2.401	2.232	2.141	
HCM Lane V/C Ratio	0.12	0.087	0.035	0.163	
HCM Control Delay	7.9	7.8	7.4	7.9	
HCM Lane LOS	A	A	A	A	
HCM 95th-tile Q	0.4	0.3	0.1	0.6	

Intersection Delay, s/veh				
Intersection LOS				
intersection LOS				
Movement	SBU	SBL	SBT	SBR
Vol, veh/h	0	0	107	31
Peak Hour Factor	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	2	2
Mvmt Flow	0	0	110	32
Number of Lanes	0	0	1	0
Approach			SB	
Opposing Approach			NB	
Opposing Lanes			1	
Conflicting Approach Left			WB	
Conflicting Lanes Left			W D	
Conflicting Approach Right	t		EB	
Conflicting Lanes Right	L		1	
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HCM Control Delay			Δ	
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CITY OF MANHATTAN BEACH

DEPARTMENT OF COMMUNITY DEVELOPMENT

TO: Angelica Ochoa, Assistant Planner

FROM: Erik Zandvliet, Traffic Engineer

DATE: September 2, 2014

SUBJECT: Site Plan Review-1114 22nd Street

Traffic Engineering Comments (Revised 9-2-2014)

The following comments have been prepared to address traffic engineering concerns for the proposed chalk Preschool at 1114 22nd Street based on plans prepared by Studio 9one2Architecture dated June 1, 2014 and the Traffic Impact Study prepared by Linscott, Law and Greenspan Engineers, dated July 10, 2014.

- 1. The applicant shall prepare and maintain a Traffic Operations and Management Plan (TOMP) as summarized in the Traffic Impact Study to be followed by faculty, staff, students and parents/guardians. The TOMP shall include, but not be limited to, the following requirements:
 - a. School staff shall be directed to arrive at the on-site parking lot prior to commencement of student drop-off operations and park within designated spaces.
 - b. One to two staff members or volunteers will be positioned within the site parking lot to direct parent/guardian drop-off and pick-up operations and assist during the morning drop-off and afternoon pick-up peak periods.
 - c. Staff or volunteers shall wear safety gear including reflective vests at all times when performing traffic control operations within the parking lot.
 - d. Parents and guardians shall park their vehicles on-site for short-term parking and then escort their pre-school child/children to the appropriate building entrance.
 - e. School-related vehicles (e.g., parents/guardians dropping off students, etc.) will also be directed to travel to the site via Sepulveda Boulevard, Cedar Avenue, and 22nd Street so as to result in a greater disbursement of trips.
 - f. Upon entering the project site, parents and guardians will be encouraged to have their student(s) ready to exit and enter the vehicle safely and efficiently.
 - g. The parking lot gate will remain open during student drop-off and pick-up times.
 - h. School-related vehicles will be directed to not park, drop-off, or pick-up students anywhere along 22nd Street or Cedar Avenue.
 - i. The TOMP should include information on parking operations, site access and circulation, and pre-school student drop-off/pick-up operations. The goal of maintaining and reinforcing the TOMP is to facilitate site access and circulation to/from the site, minimize impacts to the neighborhood surrounding the site, and efficiently manage parking facilities provided on the site.
 - j. The parking and student drop-off/pick-up operations contained in the TOMP shall be included in Chalk pre-school policies. These school policies should be

EXHIBIT D PC MTG 9-10-14 communicated to faculty, staff, students and parents/guardians at the beginning of the school year and be reinforced throughout the school year.

- 2. Any vehicle gates shall remain open during business hours. (COA)
- 3. All two-way driveways and approaches shall be as wide as the aisle it serves. The driveway approach must be at least 24 feet wide (W=24'), not including approach wings. (**COA and revise plans**)
- 4. All parking spaces shall remain unrestricted for all users during business hours. (COA)
- 5. Parking stall cross-slope shall not exceed 5%. (COA)
- 6. Doors, gates and staircases shall not swing into a vehicle aisle or public sidewalk. (COA)
- 7. Provide unobstructed triangle of sight visibility (5' x 15') adjacent to each driveway and behind the ultimate property line when exiting the parking areas without walls, columns or landscaping over 36 inches high. (MBMC 10.64.150) (**COA** and shown on plans)
- 8. All parking spaces adjacent to a vertical obstruction, except columns, must be at least one foot wider than a standard space. **(COA)**
- 9. Wheel stops are necessary for all parking spaces inside a parking lot or structure except those spaces abutting a masonry wall or protected by a 6-inch high curb. (MBMC 10.64.100D) (COA)
- 10. At least three feet is required beyond the end of an aisle to provide sufficient back-up space for vehicles in the last space of the aisle. (**COA and shown on plans**)
- 11. Disabled parking must comply with current standards. One or more van size spaces may be required in parking lot with sufficient height clearance. See 2013 CBC Chapter 11B, Div II and other ADA requirements. (**Revise plans**)
- 12. All unused driveways and undeveloped property frontages shall be reconstructed with curb, gutter and sidewalk. The existing driveway approaches shall be removed and replaced with curb, gutter and sidewalk. (**COA**)
- 13. Any compact spaces shall be labeled with a sign and a stencil marking at the back of each space. (COA)
- 14. All outside site lighting shall be directed away from the public right-of-way and shall minimize spill-over onto the sidewalks and street. Shields and directional lighting shall be used where necessary to prevent spillover onto adjacent properties. (**COA**)
- 15. Bicycle parking shall be provided at a rate of five percent (5%) of all parking spaces. (MBMC 10.64.80) (**COA**)
- 16. The parking lot shall be signed and marked to the satisfaction of the City Traffic Engineer. **(COA)**.

COA – Condition of Approval

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CITY OF MANHATTAN BEACH DEPARTMENT OF COMMUNITY DEVELOPMENT

TO:

See distribution below

FROM:

Angela Soo, Executive Secretary

(c/o Planner TBD)

DATE:

June 12, 2014

SUBJECT:

Review Request for Proposed Project at:

1114 22ND STREET

(Chalk Preschool

Commercial Use Permit / Environmental Assessment /
Remodel/Change Existing 8,212 sq ft single story commercial building
to a 119 max child preschool /

Remove and Reconstruction of existing non-conforming parking lot to 17 space lot

The subject application has been submitted to the Planning Division. Please review the attached material(s) and provide specific comments and/or conditions you recommend to be incorporated into the draft Resolution for the project. Conditions should be primarily those which are not otherwise addressed by a City Ordinance.

If no response is received <u>JUNE 25, 2014</u> we will conclude there are no conditions from your department.

Comments/Conditions (attach additional shorts as necessary):

Date 8-28-14 NO

PUBLIC WORKS - ENGINEERING CITY OF MANHATTAN BEACH

구청 및 4별당

Yes/No Building Div.
Yes/No Fire Dept
Yes/No Public Works (Carl Blank)
Yes/No Engineering (Joe Parco)
Yes/No Waste Mgmnt (Anna LJ)
Yes/No Traffic Engr.(Erik Z)

Yes No City Attorney
Yes / No Police Dept.: — TBD

___Traffic

___ Detectives

___ Crime Prevention ___ Alcohol License (Chris Vargas)

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(c/o Planner TBD)

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1114 22ND STREET

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Comments/Conditions (attach additional sheets as necessary):

TBD —	Yes/No Building Div. Yes/No Fire Dept Yes/No Public Works (Carl Blank) Yes/No Engineering (Joe Parco) Yes/No Waste Mgmnt (Anna LJ) Yes/No Traffic Engr.(Erik Z)	Yes No City Attorney Yes / No Police Dept.: — TBD Traffic Detectives Crime Prevention Alcohol License (Chris Vargas)
	Test item Engr.(Enk 2)	Alcohol License (Chris vargas)

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CITY OF MANHATTAN BEACH DEPARTMENT OF COMMUNITY DEVELOPMENT

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See distribution below

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(c/o Planner TBD)

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June 12, 2014

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Commercial Use Permit / Environmental Assessment /
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Remove and Reconstruction of existing non-conforming parking lot to 17 space lot

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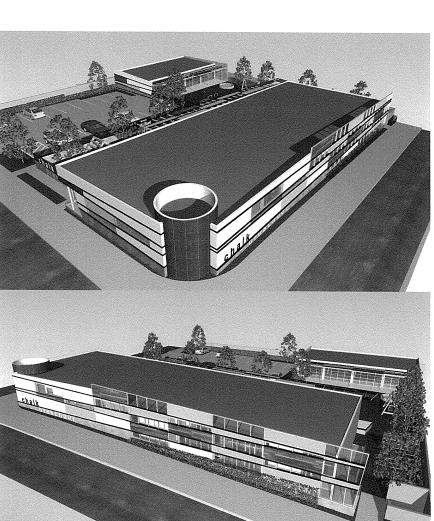
If no response is received <u>JUNE 25, 2014</u> we will conclude there are no conditions from your department.

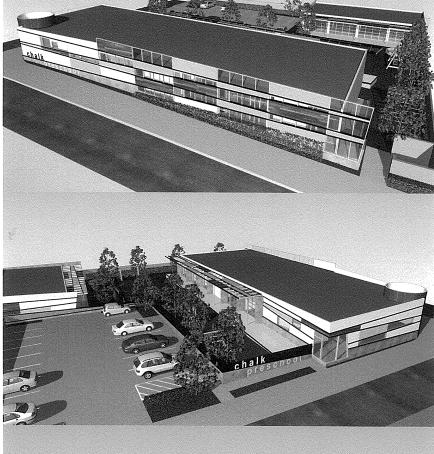
Comments/Conditions (attach additional sheets as necessary):

Comply with 2013 California Codes and City of Manhattan Beach Amendments.

Yes/No Building Div. Yes/No Fire Dept Yes/No Public Works (Carl Blank) Yes/No Engineering (Joe Parco) Yes/No Waste Mgmnt (Anna LJ) Yes/No Traffic Engr.(Erik Z)	Yes (No City Attorney Yes / No Police Dept.: TBDTraffic Detectives Crime Prevention Alcohol License (Chris Vargas)
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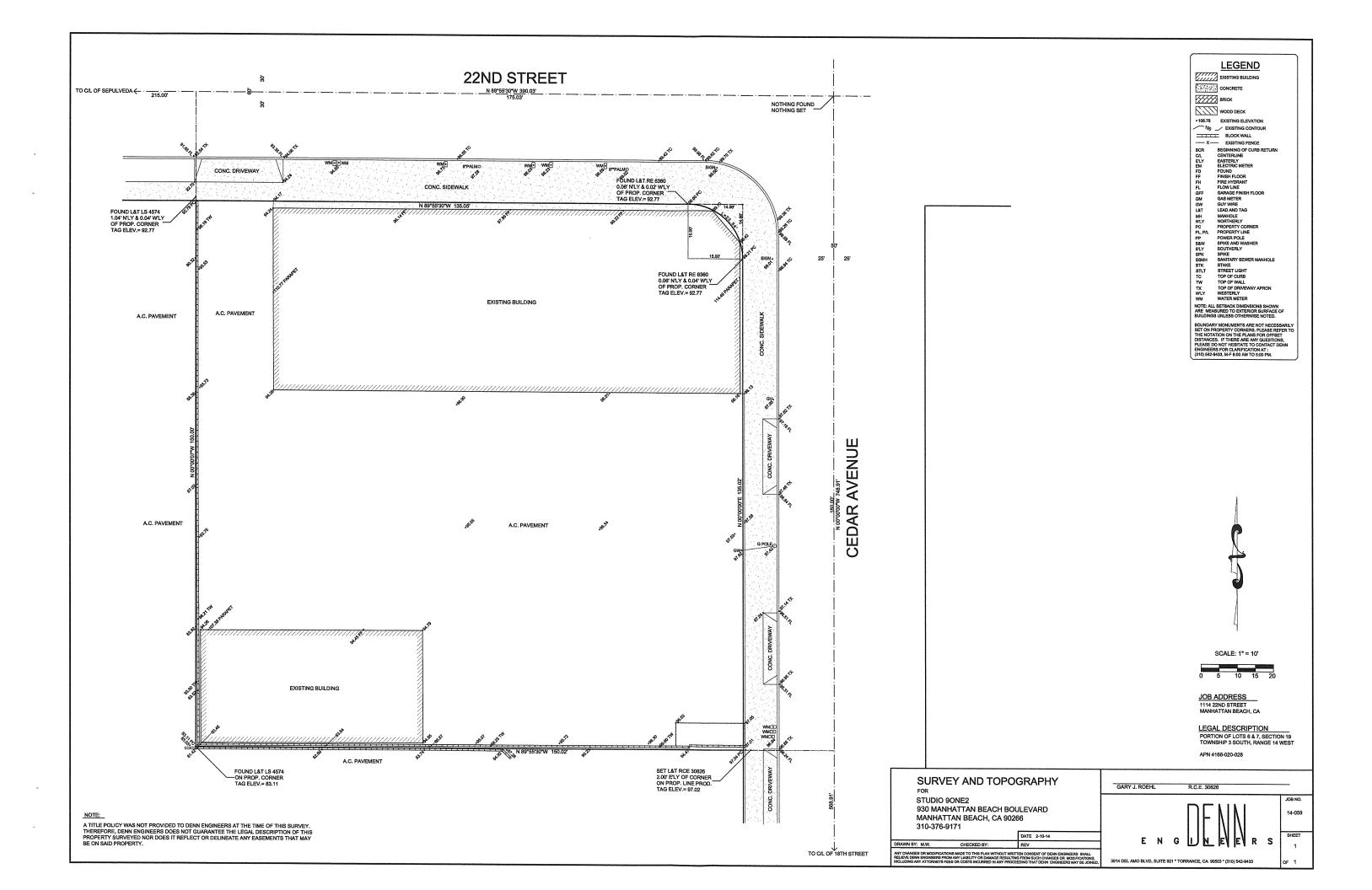


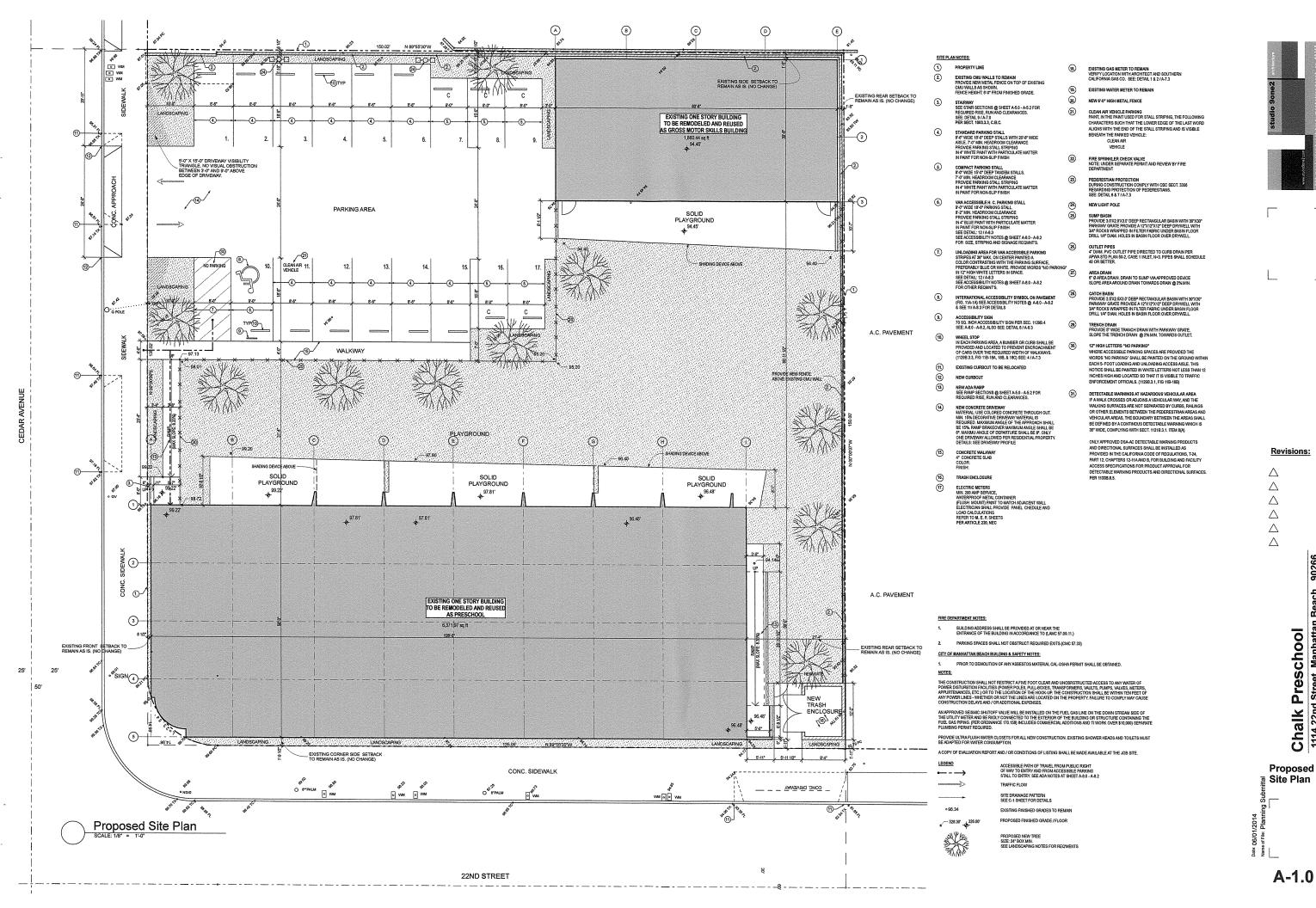


PROJECT DATA		ABBREVIATION			SHEET SCHE	DULE
OWNER: PROJECT ADDRESS: LEGAL DESCRIPTION: ACCESSOR'S PARCEL #: OCCUPANCY: FIRE SPRINKLERS: CONSTRUCTION: ZONING: NO. OF STORIES: SCOPE OF WORK: GENERAL BUILDING INFORMATION AREA TABULATION: LOT AREA: ALLOWABLE FLOOR AREA: (100 % OF LOT AREA) PRESCHOOL BUILDING: GROSS MOTOR SKILLS BUILDING: TOTAL FLOOR AREA: REQUIRED AREA OF CLASSROOMS: (35 SQ FT / CHILD) TOTAL AREA OF CLASSROOMS: REQUIRED AREA OF PLAYGROUND: (75 SQ FT / CHILD) PLAYGROUND PROVIDED: PARKING REQUIREMENTS: REQUIRED PARKING SPACES: (1 PARKING SPACE S'- ADA ACCESSIBLE SPACES: COMPACT PARKING SPACES: COMPACT PARKING SPACES: COMPACT PARKING SPACES: (1 TOTAL PARKING SPACES: COMPACT PARKING SPACES: (1 TOTAL PARKING SPACES: (1 TREE / 6 PARKING SPACES) HEIGHT LIMIT: (199.21+99.70 / 2) + 92.79 '+ 83.11' + 97.04') / 4 PROPOSED BUILDING HEIGHT: THIS PROJECT SHALL COMPLY WITH:	17/6 = 2.83 (3 TREES) 5	AB. AB. ANCHOR BOLT ACOUS. ASPAILITICA ACOUS. ASPAILITICA ACOUS. ACRES AD. ACRES AD. ACRES BO. ASOVE RISHHED SURFACED ASOVE BD. BO. BO. BO. BO. BO. BO. BO.	U. URINAL VERT. VERTICAL	L COUST OF THE STATE AND THE	GENERAL NOTES DOOR SCHEDULE WINDOW SCHEDULE ENVELOPE TITLE:24 PC 1 SITE SURVEY GRADING AND DRAINA 1.0 SITE PLAN 2.0 FIRST FLOOR PLAN 2.1 GROSS MOTOR SKILLS 3.0 ROOF PLAN 3.1 ROOF PLAN MOTOR S 3.0 ROOF PLAN 3.1 ROOF PLAN MOTOR S 4.0 BUILDING SECTIONS 5.0 FURNITURE PLAN 6.1 EXTERIOR ELEVATION: 6.2 EXTERIOR ELEVATION: 6.3 EXTERIOR ELEVATION: 6.4 EXTERIOR ELEVATION: 6.5 EXTERIOR ELEVATION: 6.6 EXTERIOR ELEVATION: 6.1 EXTERIOR ELEVATION: 6.2 EXTERIOR ELEVATION: 6.3 EXTERIOR ELEVATION: 6.4 EXTERIOR ELEVATION: 6.5 EXTERIOR ELEVATION: 6.6 EXTERIOR ELEVATION: 6.1 EXTERIOR ELEVATION: 6.2 EXTERIOR ELEVATION: 6.3 EXTERIOR ELEVATION: 6.4 EXTERIOR ELEVATION: 6.5 EXTERIOR ELEVATION: 6.6 EXTERIOR ELEVATION: 6.1 EXTERIOR ELEVATION: 6.2 EXTERIOR ELEVATION: 6.3 ACCHESCIBLE RESTROY 6.4 ACCESSIBLE RESTROY 6.5 ACCESSIBLE RESTROY 6.6 EXTERIOR ELEVATION: 6.7 EXAMING DETAILS 6.8 SOUNDATION DETAILS 6.9 FRAMING DETAILS 6.9 FRAMING DETAILS 6.9 FRAMING DETAILS 6.0 FRAMING DETAILS 6.1 FRAMING DETAILS 6.1 FRAMING DETAILS 6.2 FRAMING DETAILS 6.3 FRAMING DETAILS 6.4 FRAMING DETAILS 6.5 FRAMING DETAILS 6.6 FRAMING DETAILS 6.7 FRAMING DETAILS 6.7 FRAMING DETAILS 6.7 FRAMING DETAILS 6.8 SOUTHERN CALIFORN 6.0 ELECTRICAL TITLE 24 6.0 ELECTRICAL TITLE 24 6.0 SOUTHERN CALIFORN 6.0 ELECTRICAL TITLE 24 6.0 SOUTHERN CALIFORN 6.0 SUCHERN CALIFORN 6.0 ELECTRICAL TITLE 24 6.0 SOUTHERN CALIFORN 6.0 SUCHERN CALIFORN 6.0 ELECTRICAL TITLE 24 6.0 SOUTHERN CALIFORN	S BUILDING PLAN EKILLS BUILDING) S S S S S S S S S S S S S S S S S S S
SEPARATE PERMITS SHALL BE OBTAINED I SITE WORK, ELECTRICAL, MECHANICAL, PL DEMOLITION WORKS. ENCROACHMENT PERMIT IS REQUIRED FO	ALIFORNIA FIRE CODE, IS CALIFORNIA GREEN BUILDING STANDARDS CODE, IANHATTAN BEACH AMENDMENTS FOR: UMBING, SIGNAGE, LANDSCAPE, SPRINKLERS AND R WORK IN PUBLIC RIGHT OF WAY. CHANICAL EQUIPMENT SCREENING, PLUMBING, IS SEE NOTE #19 @ A-2.0) IS SEPARATE PLAN REVIEW AND APPROVAL	CONSULTANTS ARCHITECT: Studio 9 One 2 Patrick J. K illen Architect 930 Manhattan Beach Blwd. Manhattan Beach, Ca. 902f Ph: (310) 376-9171 SURVEY: DENN Engineers Gary J. Roehl 3914 Del Arno Bld, Suite 92 Torrance, Ca. 90503 Ph: (310) 542-9433 STRUCTURAL: DCI Engineers Craig Crowley 601 West Riverside, Suite 6 Spokane, Washington 9920 Ph: (509) 455-4448 ELECTRICAL: Moses and Associates, Inc. Ray Moses 447 S. Robertson Blwd, Su Beverly Hills, Ca. 90211 Ph: (310) 859-7715 MECHANICAL: PLUMBING: SOIL/GEC: Western Laboratories Geotechnical Engineering 2501 W. 237th Street, Suite Torrance, Ca. 9005 Ph: (310) 530-8900	1 00 1 te 201		SYMBOLS O O O O O O O O O O O O O O O O O O	ROOM NAME SECTION DESIGNATIOSHEET NUMBER DETAIL NUMBER WINDOW LETTER DOOR NUMBER KEY NOTE



T-1

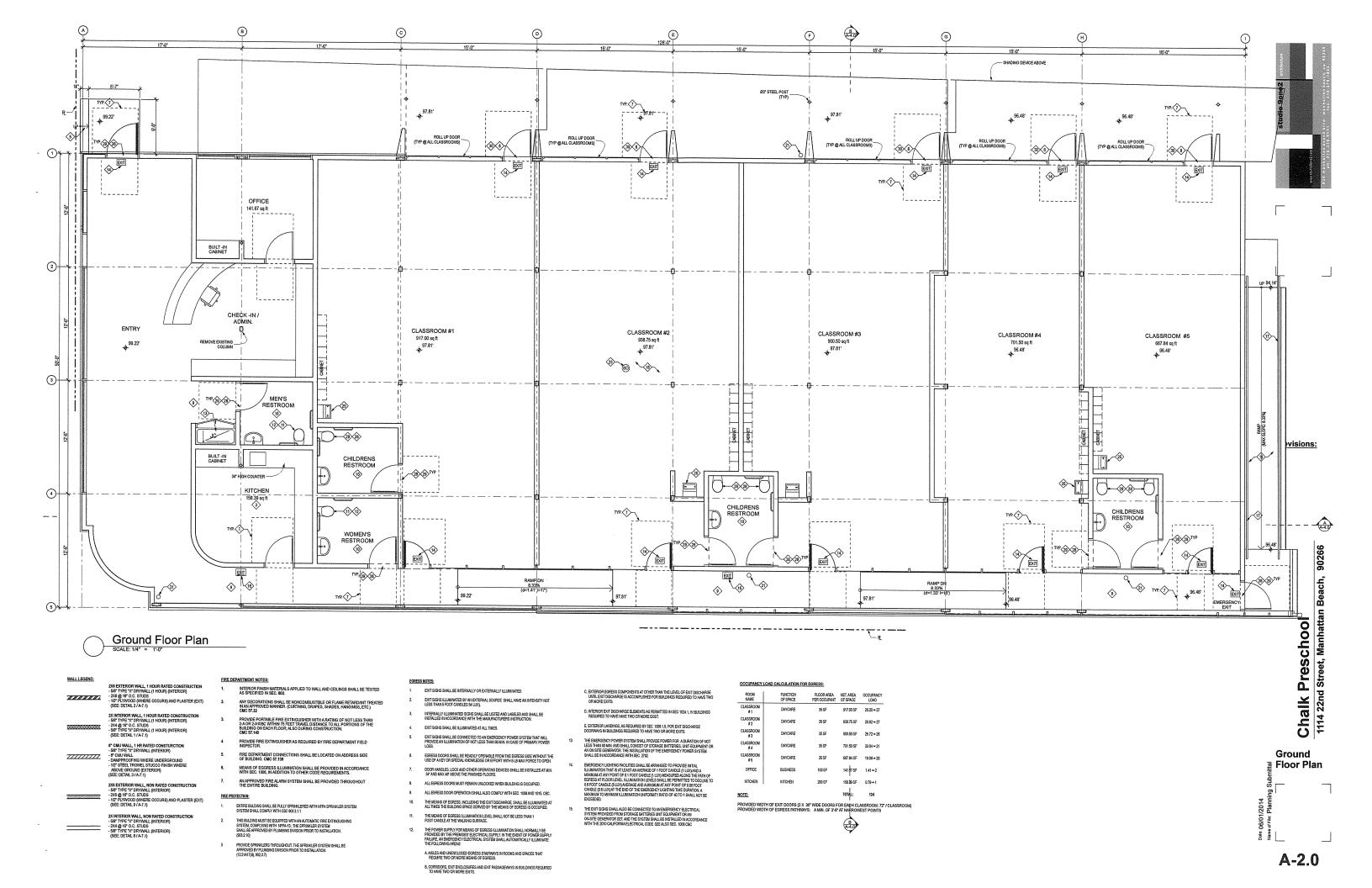


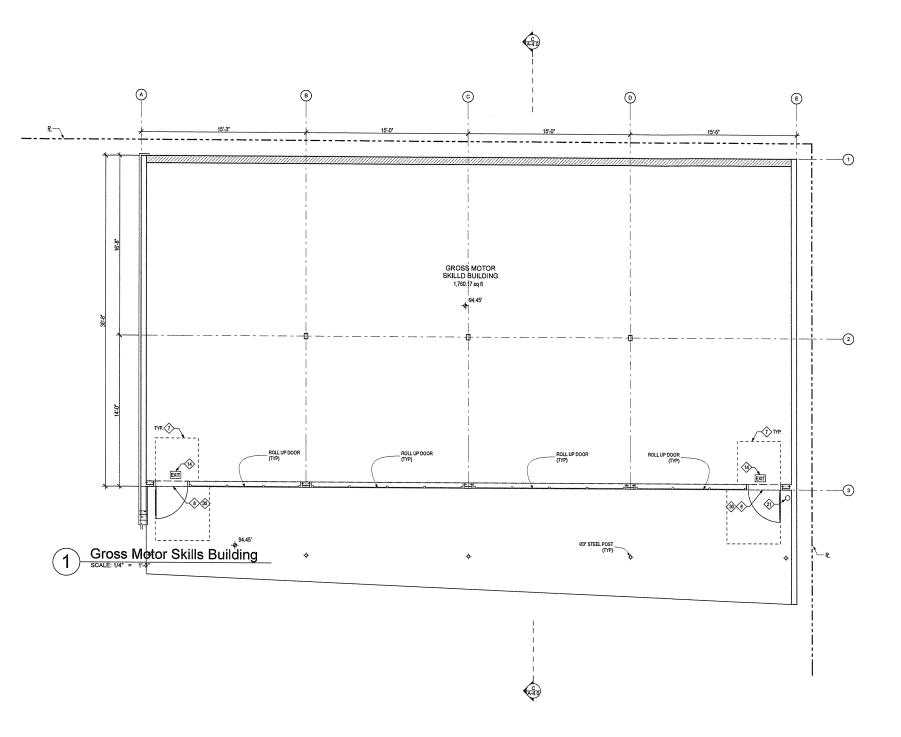


A-1.0

Chalk Preschool
1114 22nd Street, Manhattan Beach,

Revisions:





2X6 EXTERIOR WALL, 1 HOUR RATED CONSTRUCTION
- 5/8" TYPE "X" DRYWALL (1 HOUR) (INTERIOR)
- 2X6 @ 16" O.C. STUDS
- 1/2" PLYWGOD (WHERE OCCURS) AND PLASTER (EXT) (SEE: DETAIL 2 / A-7.1) 2X INTERIOR WALL, 1 HOUR RATED CONSTRUCTION - SIST TYPE "X" DRYWALL (1 HOUR) (INTERIOR)
- 2X4 @ 16" O.C. STUDS
- 56" TYPE "X" DRYWALL (1 HOUR) (INTERIOR)
(SEE: DETAIL 1 / A-7.1) 8" CMU WALL, 1 HR RATED CONSTURCTION - 5/8" TYPE "X" DRYWALL (INTERIOR) - 8" CMU WALL - DAMPPROOFING WHERE UNDERGROUND - 1/2" STEEL TROWEL STUCCO FINISH WHERE - AROVAE GROUND CHYSTERION BOOK STORY 7//////// ABOVE GROUND (EXTERIOR) (SEE: DETAIL 3 / A-7.1)

2X6 EXTERIOR WALL, NON RATED CONSTRUCTION - 58° TYPE "X" DRYWALL (INTERIOR) - 2X6 @ 16° O.C. STUDS - 122° PLYWOOD (WHERE OCCURS) AND PLASTER (EXT) 1/2" PLYWOOD (WHERE (SEE: DETAIL 9 / A-7.1) 2X INTERIOR WALL, NON RATED CONSTRUCTIO

- 58° TYPE "X" DRYWALL (INTERIOR)
- 2X4 @ 16° O.C. STUDS
- 58° TYPE "X" DRYWALL (INTERIOR)

FIRE DEPARTMENT NOTES:

INTERIOR FINISH MATERIALS APPLIED TO WALL AND CEILINGS SHALL BE TESTED AS SPECIFIED IN SEC. 802

ANY DECORATIONS SHALL BE NONCOMBUSTIBLE OR FLAME RETARDANT TREATED IN AN APPROVED MANNER. (CURTAINS, DRAPES, SHADES, HANGINGS, ETC.)

PROVIDE PORTABLE FRE EXTINGUISHER WITH A RATING OF NOT LESS THAN 2-A OR 2-A108C WITHIN 75 FEET TRAVEL DISTANCE TO ALL PORTIONS OF THE BUILDING ON EACH FLOOR, ALSO DURING CONSTRUCTION. CMC 57:140

PROVIDE FIRE EXTINGUISHER AS REQUIRED BY FIRE DEPARTMENT FIELD INSPECTOR.

FIRE DEPARTMENT CONNECTIONS SHALL BE LOCATED ON ADDRESS SIDE OF BUILDING, CMC 57.138

MEANS OF EGGRESS ILLUMINATION SHALL BE PROVIDED IN ACCORDANCE WITH SEC. 1006, IN ADDITION TO OTHER CODE REQUIREMENTS.

AN APPROVED FIRE ALARM SYSTEM SHALL BE PROVIDED THROUGHOUT THE ENTIRE BUILDING.

THIS BILDING MUST BE ECLAPPED WITH AN AUTOMATIC FIRE EXTINGUISHING SYSTEM, COMPLYING WITH INFPA-13; THE SPRINKLER SYSTEM SHALL BE APPROVED BY PLUMBING DIMISON PRIOR TO INSTALLATION. (2022-10)

EXIT SIGNS ILLUMINATED BY AN EXTERNAL SOURCE SHALL HAVE AN INTENSITY NOT LESS THAN 5 FOOT CANDLES SALUXIX

INTERNALLY ILLUMINATED SIGNS SHALL BE LISTED AND LABELED AND SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTION.

EXIT SIGNS SHALL BE ILLUMINATED AT ALL TIMES. EXIT SIGNS SHALL BE CONNECTED TO AN EMERGENCY POWER SYSTEM THAT WILL PROVIDE AN ILLUMINATION OF NOT LESS THAN 90 MIN. IN CASE OF PRIMARY POWER LOSS.

EGRESS DOORS SHALL BE READILY OPENABLE FROM THE EGRESS SIDE WITHOUT THE USE OF A KEY OR SPECIAL KNOWLEDGE OR EFFORT WITH 5 LB MAX FORCE TO OPEN.

DOOR HANDLES, LOCK AND OTHER OPERATING DEVICES SHALL BE INSTALLED AT MIN. 34" AND MAX 46" ABOVE THE FINISHED FLOORS. ALL EGRESS DOORS MUST REMAIN UNLOCKED WHEN BUILDING IS OCCUPIED.

ALL EGRESS DOOR OPERATION SHALL ALSO COMPLY WITH SEC. 1008 AND 1015. CRC THE MEANS OF EGRESS, INCLUDING THE EXIT DISCHARGE, SHALL BE ILLUMINATED AT ALL TIMES THE BUILDING SPACE SERVED BY THE MEANS OF EGRESS IS OCCUPIED.

THE MEANS OF EGRESS ILLUMINATION LEVEL SHALL NOT BE LESS THAN 1 FOOT CANDUL AT THE WALKING SURFACE. THE POWER SUPPLY FOR MEANS OF EGRESS ILLUMINATION SHALL NORMALLY SE PROVIDED BY THE PREMISES' ELECTRICAL SUPPLY. IN THE EVENT OF POWER SUPPLY PALLIER, AN EMERGENCY ELECTRICAL SYSTEM SHALL AUTOMATICALLY ILLUMINATE THE FOLLOWING FACES.

A, AISLES AND UNENCLOSED EGRESS STARWAYS IN ROOMS AND SPACES THAT REQUIRE TWO OR MORE MEANS OF EGRESS.

B, CORRIDORS, EXIT ENCLOSURES AND EXIT PASSAGEWAYS IN BUILDINGS REQUIRED TO HAVE TWO OR MORE EXITS.

D, INTERIOR EXIT DISCHARGE ELEMENTS AS PERMITTED IN SEC 1024.1, IN BUILDINGS REQUIRED TO HAVE HAVE TWO OR MORE EXIST.

E, EXTERIOR LANDINGS, AS REQUIRED BY SEC. 1008.1.5, FOR EXIT DISCHARGE DOORWAYS IN BUILDINGS REQUIRED TO HAVE TWO OR MORE EXITS.

THE EMERGENCY POWER SYSTEM SHALL PROVIDE POWER FOR A DURATION OF NOT LESS THAN 50 MIN AND SHALL CONSIST OF STORAGE BATTERES, UNIT EQUIPMENT OR AND NISTIG SEMENTAGE. THE INSTALLATION OF THE EMERGENCY POWER SYSTEM SHALL BE IN ACCORDANCE WITH SEC. 2702.

EMERGENCY LIGHTING FACILITIES SHALL BE ARRANGED TO PROMDE INTIAL ELLIMINATION THAT IS AT LESS THANERAGE OF 1 FOOT CANDLE (1 LUIQ) MAINE MINIMAL THAT FOR THE ATT IN AFFERDED FOR THE PROPERTY OF THE THAT FOR THAT IS A THE ATT IN AFFERDED FOR THE PROPERTY OF THE PROPERTY O

SYSTEM PROVIDED FROM STORAGE BATTERIES UNIT EQUIPMENT OR AN
ON-SITE GENERATOR SET, AND THE SYSTEM SHALL BE INSTALLED IN ACCORDANCE
WITH THE 2010 CALIFORNIA ELECTRICAL CODE. SEE ALSO SEC. 1006 CSC

OCCUPANCY LOAD CALCULATION FOR EGRESS

PUNCTION OF SPACE FLOOR AREA NET, AREA OCCUPANCY PER OCCUPANT OF SPACE LOAD GROSS MOTOF SKILLS DAYCARE

CLASSROOMS ARE NOT IN USE.

2. PROVIDED WIDTH OF EXIT DOORS (2 X 36" WIDE DOORS: 72")

3. PROVIDED WIDTH OF EGRESS PATHWAYS: AMIN. OF 3-6" AT NARROWEST POINTS

NOTE: 1. DURING THE TIME WHEN GROSS MOTOR SKILLS AREA IS OCCUPIED ONE OR TWO **(17)**

ELEVATOR DESPREED ITEM

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INSTALLATION (CLEPANCES SHALL COMPLY

WITH 3010 SCC HOHPT 30, THE DISAGLED

ACCESS (7-49) AND 1188 HIGH SINC

LHOUR FIRES RATED MAD LABELED DOOR AND

LHOUR FIRES RATED MAD LABELED DOOR AND

LHOUR FIRES RATED MAD LABELED DOOR AND

ANAILA TALL OPENINS NITH SHAT'S RECURRED.

NOTE: GURNEY CLERANCES NOT REQUIRED

FOR COS 3024 AND EXPENIES.

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②

STAIRWAY
SEE STAIR SECTIONS @ SHEET A 5.0 & A 5.2
FOR REGUIRED RISE, RIN AND CLEARANCES
SEE ALSO DETAIL 9, 10 & 11 / A 7.0
7 / A 7.1, 15 / A 8.3
PER SECT. 1003.3.3, C.B.C.

FIRE BLOCKING
PROVIDE DRAFT STOPS AND OR FIRE BLOCKING
IN ALL SEPARATION WALLS, FLOORICELING
ASSEMBLIES AND COMMON ATTIC SPACE (IF
APPLICABLE) SO THAT NO ENCLOSED SPACE
SHALL MEASURE MORE THAN 10°-0" IN ANY
DIRECTION
PER SECT 708, C.B.C.

GROUND FAULT INTERRUPTER (GFI) OUTLETS SHALL BE PROVIDED IN BATHROOMS, GARAGES EXTERIOR, AND WITHIN 8' OF KITCHEN SINKS REFER TO ELECTRICAL SHEETS PER SECT 210-8a, CEC

6> FIRE DEPARTMENT CHECK VALVE

FLOOR PLAN NOTES

⟨1⟩

⟨2⟩

4>

REQUIRED CLEARANCE AT DOORS
THERE SHALL BE A LEVEL AND CLEAR FLOOR OR LANDING ON
EACH SIDE OF A DOOR. THE LEVEL AREA SHALL HAVE A LEWSTH
IN THE DIRECTION OF DOOR SWINN OF AT LEAST OF AND THE
LENGTH OPPOSITE THE DIRECTION OF DOOR SWING OF 48° AS MEASURED AT RIGHT ANGLES TO THE PLANE OF THE DOOR IN THE CLOSED POSITION MEASURE AN INSUL ANGLES TO THE PLANE OF THE DOOR IN THE CLOSED POSITION, THE CLOSED POSITION, THE THE ANGLES AND THE CLOSED POSITION, THE CLOSED POSITION, THE CLOSED POSITION THE CLOSED

ENTRY DOORS

IN LIEU OF PANIC HARDWARE, PROVIDE A
READLY VISIBLE DURABLE SIGN STATING: "THIS
DOOR MUST REMAIN UNL, OCKED WHEN
BUILDING IS OCCUPIED." PER SC. 1008.1.9.2(2.2)
THE SIGN SHALL BE IN LETTERS OF NOT LESS THAN
'THIGH ON A CONTRASTING BACKGROUND. ⟨8⟩ WHEN UNLOCKED, BOTH DOORS MUST BE FREE
TO SWING WITHOUT OPERATION OF ANY
LATCHING DEVICE

(FIG 11B-26(A))

CORRIDOR AND HALLWAY WIDTH EVERY CORRIDOR AN HALLWAY SEVING AN OCCUPANT LOAD OF 10 OR MORE SHALL NOT BE LESS THAN 44" (1116 MM) IN WIDTH. CORRIDORS SERVING AN COCUPANT LOAD LESS THAN 10 SHALL NOT BELESS THAN 32" (141 MM) IN WIDTH.

(PER 1 1388 J. 1, 0 B.C.) **②**

ACCESSIBLE TOILET ROOM
PROVIDE NEW TOILET ROOMS AS SHOWN ROOMS TO
BE FULLY HANDICAPPED ACCESSIBLE.
SEE TOILET ROOM PLANS & SHEET A8.4 & A.8.5
SEE ACCESSIBILITY DETAILS @ SHEET A8.3 **(10)** AND SEE NOTES @ A-8.0 - 8.2

REQUIRED PLUMBING FIXTURES:
NUMBER OF OCCUPANTS = 137 (PER CPC TABLE A)
118 STUDENTS (60 MAUE, 60 FEMALE): 18 STAFF (0 MALE, 9 FEMALE)
MINIMUM NUMBER OF FIXTURES REQUIRED (CPC TABLE 4-1)

TOILETS LAVATORIES URINALS TOILETS
LAVATORIES
NUMBER OF FIXTURES PROVIL TOILETS TOILETS LAVATORIES

WATER CLOSETS
PROVIDE ULTRA FLUSH WATER CLOSETS FOR ALL NEW
CONSTRUCTION. TOILETS MUST BE ADAPTED FOR
WATER CONSUMPTION. 128 GALLON FLUSH, MAX.
PER CPC AND CAL. GREEN CODE

ACCESS TO TOILETS
THE CONTRELINE OF THE WATER CLOSET FIXTURE SHALL BE 16'
FROM THE SION WALL OR PARTITION. ON THE OTHER SIDE OF
THE WATER CLOSET, PROVIDE A MINIMUM 28' WIDE CLEAR
FLOOR SPACE IF THE WATER CLOSET IS ADJACENT TO A
FIXTURE OR AMENIMUM OF 32' WIDE CLEAR FLOOR SPACE IF
THE WATER CLOSET IS ADJACENT TO A WALL OR PRATITION.
THIS CLEAR SPACE SHALL EXTED FROM THE REPAR WALL TO
HE FRONT O'TE WATER CLOSET, (HISS.4.1, FIG INE.4) 12>

13> MOP SINK SEE PLUMBING PLANS

€

(15) FLOOR LEVEL EXIT SIGN FLOOR LEVEL BUT SIGN
INTERNALLY OR SYSTEMALLY LLUMINATED PHOTOLUMINESCENT
OR SELF-LUMINOUS FLOOR LEVEL BUT SIGNS SHALL BE
PROVIDED IN ALL INTERIOR CORRIDORS. THE BUTTOM OF THE
SIGN SHALL BENOT LESS THAN FO OR MORE THAN F ABOVE
THE FLOOR LEVEL AND SHALL INDICATE THE PATH OF EXIT
TRAVEL FOR BUT AND SHALL INDICATE THE PATH OF EXIT
TRAVEL FOR BUT AND STATE COST BUT SHALL BE
ON THE DOOR OR ADJACENT TO THE DOOR WITH THE OR
ON THE DOOR OR ADJACENT TO THE ORD OWN THE THE CLE
COST OF THE SIGN OR MARKER WITHIN 4" OF THE DOOR FRAME.
(C.B.C. 101.15)

€ UTDOOR CONCRETE RAMP
CLEAR WIDTH: MIN 45°
MAX SLOPE: 1" NI 1-0" (8.33%)
MAX LENGTH: 30-0" (WITH THE MAX SLOPE)
MAX RISE: 2-6"
(PER SECT. 11:33.5 - 0.B.C.)
OUTDOOR RAMPS AND THER APPROACHES OUTDOOR PAMPS AND THER APPROACHES
SHALL BE DESCRIBED ON THAT WITHER WILL NOT ACUMENTE
ON WALKING SURFACES. (PER 11338 5.6, C.B.C.)
TOP LANDING.
MIN WOTH:
60° (IN THE DIRECTION OF RAMP)
BOTTOM LANDING:
MIN LENGTH:
42° (IN THE DIRECTION OF RAMP)
DOORS IN OPEN POSITION SHALL NOT REDUCE
THE LEITH OF THE RECURRED LANDING TO LESS
THAL 22° (PER 150 5.8)

HANDRAILS FOR RAMPS
HANDRAILS SHALL BE FALCED ON EACH SIDE OF THE RAMP, SHALL
BE CONTICUS AND SHALL BE SIT TO 35' ABOVE THE RAMP SUFFACE.
HANDRAILS SHALL EXTEND A MINIMALM OF 1-0' BEYOND THE TOP
AND BOTTOM OF THE RAMP AND SHALL BE PARALLE TO THE
FLOOR HANDRAILS SHALL BE NOT LESS THAN 1-12' IN CROSS
SECTIONAL NORMAN DAMESHOND OF THE SHAPE SHALL PROVIDE
AN EQUIVALENT GREPHON SUFFACE. THE PANDROP PORTION OF
HORDRIGHES SHOWER AS MOOTH DEWYSACE WITH THE SHAPE SHALL PROVIDE
UNINTERSUPTED BY NEWLE, POSTS, OTHER CONSTRUCTION
LELEMENTS, OR OSSTRUCTIONS, ANY WALL OF OTHER SHEPSE
ADJACENT TO THE HANDRAIL SHALL BE FREE OF SHAPP OR
ADBRAICH TIS CHEMISTS. DEGES SHAP OR
ADBRAICH TO THE HANDRAIL SHALL BE FREE OF SHAPP OR
ADBRAICH TIS CHEMISTS. DEGES SHAP IN ADBRAICH SHAPE A
BERSANGE LEBURTS. DEGES SHAP IN ARE AMBRIAND RODUS
OF 16' SEED DETAIL 15/A-7.0 (PER OBC 11338 5.5)

AUTOMATIC FIRE SPRINKLER SYSTEM
ENTIRE BULDING SHALL BE FULLY SPRINKLERED.
(PER 2010 GBC 903.3.1.1)
THE SPRINKLER SYSTEM SHALL BE APPROVED BY PLUMBING
DIVISION PRIOR TO INSTALLATION. (12.21417(d), 902.3.7)

FIRE ALARM SYSTEM - DEFERRED ITEM
A FIRE ALARM SYSTEM SHALL BE INSTALLED THROUGHOUT THE
BUILDING. SYSTEM SHALL BE INSTALLED IN ACCORDANCE WITH
COC CHAPTER AND FIRE DEPARTMENT REQUIREMENTS.
PER SECT. 403.5, C.B.C.

PENETRATION FIRESTOP SYSTEM (C.B.C., 710.23)
PENETRATION SHALL BE PROTECTED BY ANAPPROVED
PENETRATION FIRESTOP SYSTEM INSTALLED AS ISSTED IN
ACCORDANCE WITH UBC STANDARD 7.5. THE SYSTEM SHALL
ANAE AN FERRING AND AT PRATIOS OF INOT LESS THAN ON
HOUR BUT NOT LESS THAN THE REQUIRED RATING OF THE
ELOOK PENETRATED.

THROUGH-PENETRATION FIRE STOPS (C.B.C. 713.3.1) THROUG THROUGH-PENETRATION PIEE STOPS (2G. Z. 713.1) THROUGH-PENETRATION FIRE STOPS RECOURCE BY THIS CODE SHALL HAVE AN FOR THATING AS DETERNINED BY TESTS CONDUCTED IN ACCORDANCE WITH USE STANDARDZ-5. THROUGH-PENETRATION FIRE STOPS MAY BE USED FOR MEMBRANE PENETRATIONS. THE FY RATING SHALL APPLY TO ALL THROUGH-PENETRATIONS AND SHALL NOT BE LESS THAN ALL THROUGH-PENETRATIONS AND SHALL NOT BE LESS THAT THE REQUERD FINER-RESISTANCE RETAINS OF THE ASSEMBLY PENETRATED. THE TRATING SHALL APPLY TO THOSE THROUGH-PENETRATION LOCATIONS REQUIRED TO HAVE TRATINGS AS SPECIFIED IN SECTION 710.2.3 AND SHALL NOT PENETRATION LOCATION OF THE PENETRATION OF THE SHALL PENETRATE HAS ALL PROPERTY ASSEMBLY TO THE ASSEMBLY PENETRATED. ALL PROPERTY PENETRATE IN THE THE INS SERVES SMALLES SELVIERLY PAS IDEN IO INTERIOR SERVES HAT HE TENDED THE HE SERVE HE HE TENDED THE HE SERVE HE SERVE HE HE SERVE HAD THE SLEEVE INSELF AND AN ASSUMENT PROBLEM THE SERVE AND THE SLEEVE HE SERVE HAD THE SERVE HAD AND ASSUMENT PROBLEM TO SHALL BE PROTECTED. INSULATION AND THE SERVE HE SERVE HE SHALL THOUGHT HAD AND THE SERVE HAD AND THE SERVE HAD AND THE SERVE HAD THE SERVE HAD AND THE SERVE HAD THE SERV ASSEMBLY. SEE MECHANICAL & PLUMBING PLANS FOR DETAILS

PORTABLE FIRE EXTINGUISHERS PROVIDE PORTABLE FIRE EXTINGUISHER WITH A RATING OF NOT LESS FRAVOUS PORTABLE FIRE EXTINGUISHER WITH A RATING OF NOT LESS THAN 2.4 OR 3.4 ALIGO WITHIN 15 FEET TRAVEL DISTANCE TO ALL PORTIONS OF THE BULDING ON EACH FLOOR; ALSO DURING CONSTRUCTION, CAMOS 7.140 HOWEVER THIS BULDING MUST BE EQUIPPED WITH AN AUTOMATIC FIRE EXTRIGUISHING SYSTEM, COMPLYING WITH ORPPATS IN PRPATSR); THE SPRANKLER GYSTEM SALL BE APPROVED BY PLUMBING DIVISION PROCK TO INSTALLATION, (603.21).

ROOF LADDER AND HATCH ROOF ACCESS LADDER TO BE: 501 HEAVY DUTY TUBULAR RAIL FIXED & SHALL BE INSTALLED WITH A LADDER-UP SAFETY POST. (OYNEEPES INC. 415 #822-#222) ROOF HATCH TO BE: ACCUDOR (702 7644.871) FRE RATED **②** (1227 094.8710) FIRE RATED
SHALL BE IN COMPLIANCE WITH COR TITLES, SECTION 3277
MANUFACTURER: PRECISION LADDERS, LLC
MODEL:
FLS PRODUCT OR APPROVED EQUAL COMPLYING 2010 CBC
SEE DETAIL: \$/A-7.3

SMOKE DETECTORS
SMOKE DETECTORS SHALL BE INSTALLED IN ACCORDANCE WITH CBC
AND SHALL BE LOCATED AS FOLLOWS: **23**>

A. IN EVERY MECHANICAL EQUIPMENT, ELECTRICAL, TRANSFORMER, TELEPHONE EQUIPMENT, ELEVATOR A. IN EVERY MECHANICAL EQUIPMENT, ELECTRICAL,
TRANSPORMER, TELEPHONE COUPMENT, ELECTROCA,
MACHINE OR BINLAR ROOM AND IN ELEMATOR LOSSIES.
IN THE MAN RETURNAH RAN DEWALUST AND FLOMM OF
EACH AR CONDITIONING SYSTEM, SUCH DETECTOR SHALL
BELOCATED AN A SERVICEABLE AVEADOWN STEMAN OF
THE LAST DUCT MALET.
C. AF EACH CONNECTION TO A VERTICAL DUCT OR RISER
SERVING TWO OR MORE STORIES FROM A RETURNAH DUCT
OR RESINIO FA MACROCOMITORIONS SYSTEM.

NEW FIRE DEPARTMENT CONNECTION (CMC 57.138)

25>

26> FURNITURE ELEMENTS
ALL PURNITURE ELEMENTS (CABINETS, COUNTERS, SINKS, SEATINS,
ETC) SHALL MEET THE ACCESSIBILITY REQUIREMENTS OF
CBC CHAPTER 118.

RESTROOM DOOR SIGNS RESTROOM DOOR SIGNS

DOORWAYS LEDONG TO MEYS SANTARY FACLITIES SHALL BE
IDENTIFIED BY AN ECULLATERA. TRANSIGE 14" THICK WITH EDGES 12"
LONG AND A VERTEX FOORTING UPWAND, WOMEN'S SANTARY
FACLITIES SHALL BE DENTIFIED BY A CIRCLE 14" THICK AND 12" IN
DAMAETER, LITIES SHALL BE DENTIFIED BY A CIRCLE 14"
THICK, 12" AND LIMITES SHALL BE IDENTIFIED BY A CIRCLE 14"
THICK, 12" AND LIMITER, WITH A 14" THICK TRANSIGE
SUPERINPOSED ON THE CIRCLE AND WITHIN THE 12" DAMAETER
(1158 A) SUPERMINOSED ON THE UPPLACE MAY WITHOUT STATEMENT OF THE UPPLACE AT THINKING, SYMBOLS ON SANITARY FACILITY GEOLETIC CIPICAL & TRINKING, SYMBOLS ON SANITARY FACILITY DOORS SHALL BE CENTERED ON THE DOOR AT A HEIGHT OF SY AND THEIR COLOR AND CONTRAST SHALL BE DISTRICTLY DIFFERENT FROM THE COLOR AND CONTRAST OF THE DOOR (11158.8)

FROM THE CULTURAL USERINGS TO THE DUOR. (TITLES)
BRAILE SIGN.
WHEN PERMANENT IDENTIFICATION IS PROVIDED FOR ROOMS AND
SPACES OF A BUILDING OR SITE. PAISED LETTERS SMAIL BE PROVIDED
AND SHALL BE ACCOMPANED BY BRAILE IN CONFORMANCE
WITH SECTION IN THIS 2.2 THROUGH THIS 25.7 SIGNS SHALL BE
DOOR. WHERE THEME IS NO WALL SPACE ON THE LATCH SIDE.
DOOR. WHERE THEME IS NO WALL SPACE ON THE LATCH SIDE.
NOLLUNG AND TOOLE LEEP DOORS, SIGNS SHALL BE FLACED ON THE
NEAREST ADJACCHT WALL, PREFERABLY ON THE RIGHT. MOVERTING
HERALT SHALL BE ON MOVES AND THE PRINCE TOOR TO THE
CENTERLINE OF THE SIGN. MOVINTING LOCATION SHALL BE
CENTERLINE OF THE SIGN. MOVINTING LOCATION SHALL BE
OF SIGNAGE WITHOUT ENCOURTERING PROTEUTION OF SLETCH OF
SIGNAGE WITHOUT ENCOURTERING PROTEUTION OF SLETCH OR
STANDING WITHIN THE SWING OF A DOOR. (11178.5.1, 11178.5.7)

CHARACTERS AND BRAILLE SHALL BE IN A HORIZONTAL FORMAT.
BRALLE SHOULD BE FLACED AMBINISHING 78-8NCH AND A MAXIMUM
OF 122-NICHO DIRECTLY BELOW THE TACTHE CHARACTERS; RLUSH
LEFT OR CENTERED. WHEN TACTLE SIGN IS MULTILINED, ALL
BRALLE SHALL BE FLACED TOGETHER BELOW ALL LINES OF TACTILE
TEXT. (11178-81.55.4)

CONTRACTED GRADE 2 BRAILE SHALL BE USED WHEREVER BRAILE IS REQUIRED IN OTHER PORTIONS OF THESE STANDARDS. DOTS SHALL BE 1/10 INCH ON CENTERS IN EACH CELL WITH 2/10-PCH SPACE BETWIEN CELLS, DOTS SHALL BE FAISED A MINIMUM OF 1/40 INCH AROVE THE BRACKGROUND. (19175.5)

ISA SIGNS AT ENTRANCE DOORS
ALL BUILDING AND FACILITY ENTRANCES THAT ARE ACCESSIBLE TO

Revisions:

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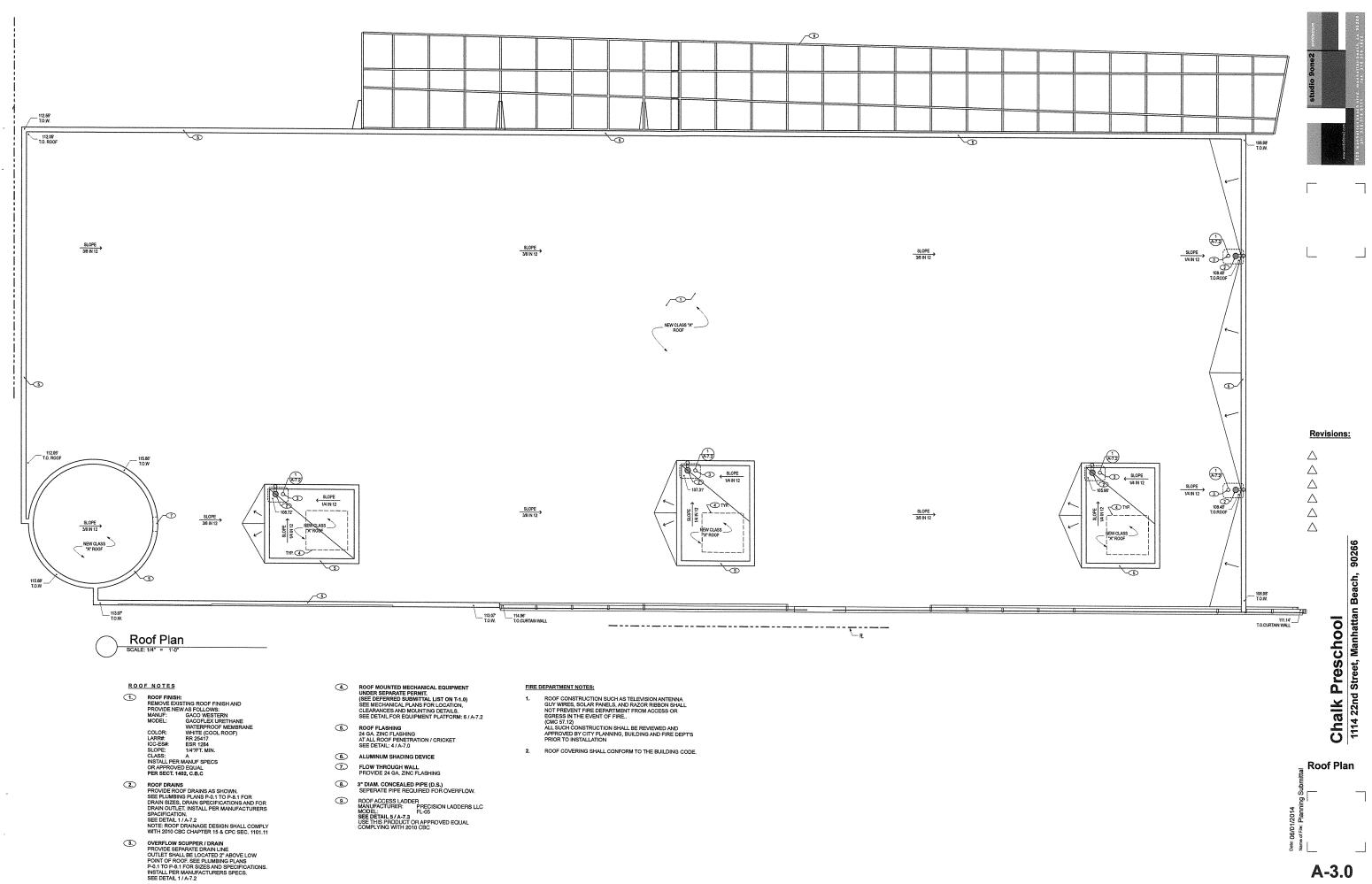
> <u></u> Prescho

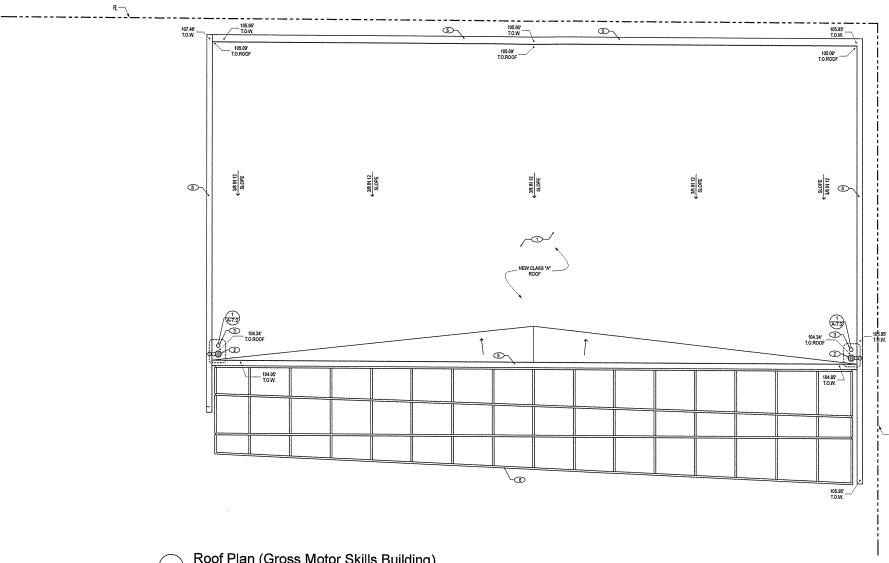
Beach,

Chalk Gross Motor

Skills Building

A-2.1





Roof Plan (Gross Motor Skills Building)

ROOF NOTES

1. ROOF FINISH:
REMOVE EXISTING ROOF FINISH AND PROVIDE NEW AS FOLLOWS:
MANUP: GACO WESTERN
MODEL: GACOFLEX URETHANE
WASTERROOF MEMBRANE
COLOR: WHITE (COOL ROOF)
LARR#: RR 25417
ICC-ES#: ESR 1284
SLOPE: 14"/FT. MIN.
CLASS: A
INSTALL PER MANUF SPECS

WATERPROOF I
COLOR: WHITE (COOL R
LARR#: RR 25417
ICC-E5#: E5R 1284
SLOPE: 1/4"/FT. MIN.
CLASS: A
INSTALL PER MANUF SPECS
OR APPROVED EQUAL
PER SECT. 1402, C.B.C

ROOF DRAINS
PROVIDE ROOF DRAINS AS SHOWN.
SEE PLUMBING PLANS P-0.1 TO P-8.1 FOR
DRAIN SIZES, DRAIN SPECIFICATIONS AND FOR
DRAIN OUTLET. INSTALL PER MANUFACTURERS
SPACIFICATION.
SEE DETAIL 1 / A-7.2
NOTE: ROOF DRAINAGE DESIGN SHALL COMPLY
WITH 2010 CBC CHAPTER 15 & CPC SEC. 1101.11

OVERFLOW SCUPPER / DRAIN
PROVIDE SEPARATE DRAIN LINE
OUTLET SHALL BE LOCATED 2" ABOVE LOW
POINT OF ROOP. SEE PLUMBING PLANS
P-0.1 TO P-8.1 FOR SIZES AND SPECIFICATIONS.
INSTALL PER MANUFACTURERS SPECS.
SEE DETAIL 1 / A-7.2

ROOF MOUNTED MECHANICAL EQUIPMENT UNDER SEPARATE PERMIT. (SEE DEFERRED SUBMITTAL LIST ON T-1.0) SEE MECHANICAL PLANS FOR LOCATION, CLEARANCES AND MOUTING DETAILS. SEE DETAILFOR EQUIPMENT PLATFORM: 6 / A-7.2

5. ROOF FLASHING 24 GA. ZINC FLASHING AT ALL ROOF PENETRATION / CRICKET SEE DETAIL: 4 / A-7.0

6. ALUMINUM SHADING DEVICE

FLOW THROUGH WALL PROVIDE 24 GA. ZINC FLASHING 7.

3" DIAM. CONCEALED PIPE (D.S.) SEPERATE PIPE REQUIRED FOR OVERFLOW.

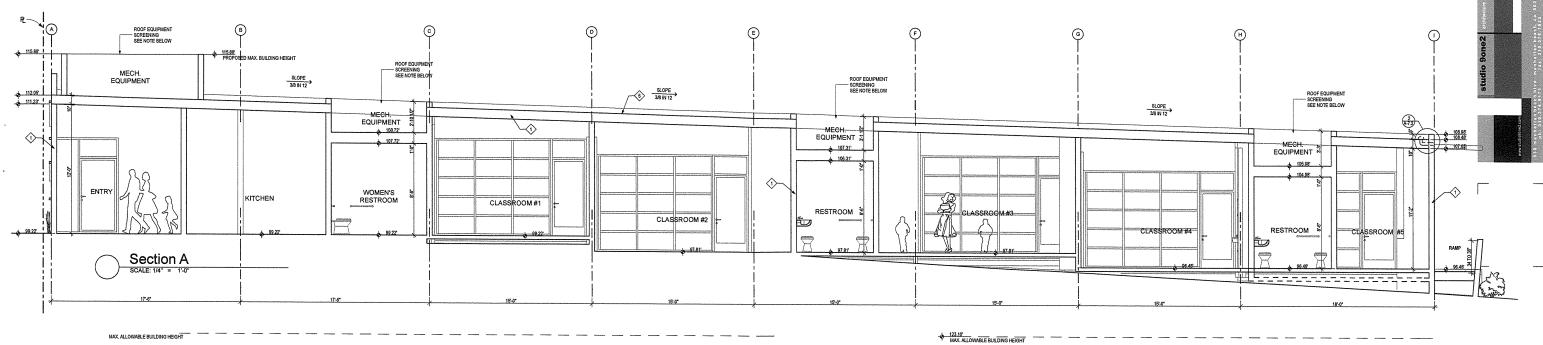
ROOF ACCESS LADDERS
MANUFACTURIER: PRECISION LADDERS LLC
MODEL: FL-05
SEE DETAIL 5 / A-7.3
USE THIS PRODUCT OR APPROVED EQUAL
COMPLYING WITH 2010 CBC

FIRE DEPARTMENT NOTES:

ROOF CONSTRUCTION SUCH AS TELEVISION ANTENNA GUY WIRES, SOLAR PANELS, AND RAZOR RIBBON SHALL NOT PREVENT FIRE DEPARTMENT FROM ACCESS OR ECRESS IN THE EVENT OF FIRE... (CMC 57.12)
ALL SUCH CONSTRUCTION SHALL BE REVIEWED AND APPROVED BY CITY PLANNING, BUILDING AND FIRE DEPT'S PRIOR TO INSTALLATION

2. ROOF COVERING SHALL CONFORM TO THE BUILDING CODE.

Revisions: \triangle \triangle \triangle \triangle \triangle \triangle Chalk Preschool
1114 22nd Street, Manhattan Beach, Roof Plan



SECTION NOTES

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INSULATION REQUIREMENTS
FOIL-BACKED BATT INSULATION
INSTALL PER MANUF. SPECS
PROVIDE: ROOF / CEILING
EXTERIOR WALL
INTERIOR WALL

REFER TO MECHANICAL DRAWINGS

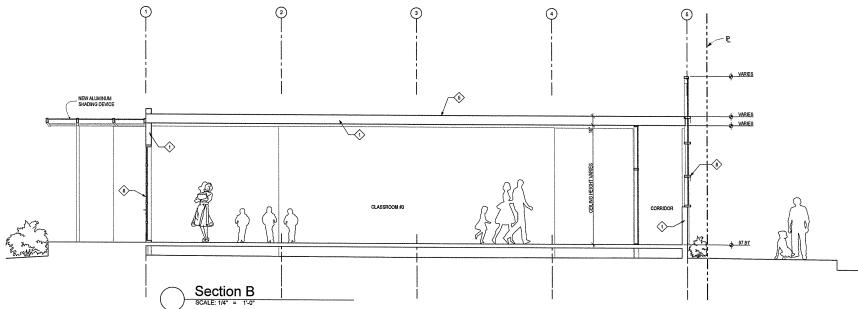
WATERPROOFING @ FOUNDATION WALL

(WHERE SPECIFIED) RAISED WD FLOOR FLR @ OVERHANGS SLAB EDGE INSUL.

MANUF: MAR-FLEX WATER/ROOFINS
MODEL: MAR-FLEX 500° WITH GEO-MAT PLUS BARRIER
EXTERIOR FOUNDATION
WATER/PROOFING SYSTEM
ICC-SSE SESS-3062
PROVIDE 4' DIA PERFORATED PIPE DRAIN WITH
PERFORATIONS PACING DOWN AND DUT TO THE SIDE.
SET PIPE IN 1 CUBIC FOOT OF FREE-DRAINING CLEAN

CRUSHED ROCK PER LINEAL FOOT OF PIPE.
THE ROCK SHOULD BE ENCAPSULATED IN A GEO-FABRIC
KIMRAFLI JABUL ENT OF SUFFICIENT SIZE TO
COMPLETELY ENCLOSED THE ROCK (SEE SOIL REPORT)

R-30 R-19 R-13



MECHANICAL EQUIPMENT SCREENING NOTE: MECHANICAL EQUIPMENT SCREENING IS SUBJECT OF DEFERRED SUBMITTAL. WHEN MECHANICAL EQUIPMENT IS ALREADY INSTALLED A FIELD INSPECION BY PLANNING STAFF WILL BE HELD IN ORDER TO DETERMINE THE NEED OF VISUAL FOUIPMENT SCREENING IF EQUIPMENT SCREENING IS REQUIRED BY THE PLANNIG STAFF CONSULT WITH ARCHITECT PRIOR

	1 HOUR RATED EXTERIOR CONSTRUCTION (EXTERIOR WALLS, FLOORS AND ROOFS) (SEE: DETAIL 2/A-7.1, 4/A-7.1 & 5/A-7.1)
************	2X INTERIOR WALL, 1 HOUR RATED CONSTRUCTION -58° TYPE 'X' DRYWALL (1 HOUR) (INTERIOR) -2X4 @ 16° O.C. STUDS -58° TYPE 'X' DRYWALL (1 HOUR) (INTERIOR) (SEE: DETAIL 1/A-7.1)
77777777	8" CMU WALL, I HR RATED CONSTURCTION - SET TIPE "TRYWALL (INTENDE) - OF CHEWAL - DAMPPROOF WEREE UNDERGROUND - 1/8" STEEL TROWEL STUCOD FINISH WHERE ABOVE GROUND (EXTERIOR) (SEE: DETAL 3/A-7.1)
	NON RATED EXTERIOR CONSTRUCTION (EXTERIOR WALLS, FLOORS AND ROOFS) (SEE: DETAILS AS INDICATED ON PLAN)

- 56" TYPE "X" DRYWALL (INTERIOR)
- 2X4 @ 16" O.C. STUDS
- 56" TYPE "X" DRYWALL (INTERIOR)
(SEE: DETAIL 8/A-7.1)

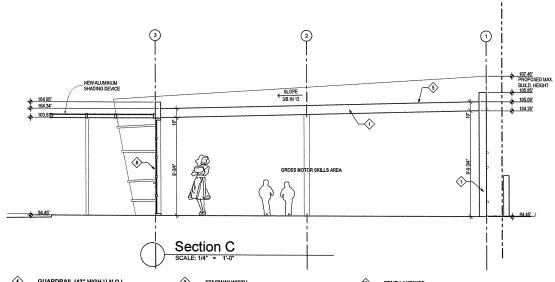
TO INSTALLATION; USE SQUARE-EDGED SHEET METAL SCREEN.

CONSTRUCTION NOTES:

PENETRATIONS

- PENETRATIONS IN WALLS REQUIRING PROTECTED OPENINGS MUST BE FIRE-STOPPED. WITH AN APPROVED MATERIAL RECORDANCE WITH GOS SECTION 713.3.1.

 SPACE BETWEEN PENETRATING MATERIAL'S DESCRIBED BELOW) MUST BE DESIGNED TO PREVENT THE MOVEMENT OF HOT FLAME OR GASSES.
 - STEEL, COPPER OR FERROUS PIPES OR CONDUITS MAY PENETRATE CONCRETE OR MASONRY WALLS WHERE THE PENETRATING ITEM IS AMAXMAND 6 NICH DAMETER AND THE AREA OF THE OPENING THROUGH THE WALL DOES NOT EXCEED 144 SQUAME INCHES.
- MEMBRANE PENETRATIONS OF MAXIMUM 2-HOUR FIRE-RESISTANCE RATED WALL AND PARTITIONS BY STEEL ELECTRICAL OUTLET BOXES NOT EXCEEDING IS SQUARE NOTHES ARE PERMITTED PROVIDING OPENINGS DO NOT EXCEED 100 SQUARE NOTHES FOR ANY 100 SQUARE FRETS OF WALL AREA QUITLET BOXES ON OPPOSITE SIDES OF
- C. WHERE WALLS ARE PENETRATED BY OTHER MATERIALS OR WHERE LARGER OFENINGS ARE REQUIRED THAN FERMITTED IN (B) ABOVE, THEY MUST BE QUALIFIED BY TESTS CONDUCTED IN ACCORDANCE WITH GBC SECTION 713.3.1.1
- OPENINGS THROUGH 1-HOUR FLOOR / CEILING ASSEMBLY SHALL BE PROTECTED BY 1-HOUR SHAFT ENCLOSURE. THE SHAFT ENCLOSURE SHALL BE CONSTRUCTED OF FIRE BARRIERS AND HORIZONTAL ASSEMBLIES PER CBC SECTION 708.



GUARDRAIL (42" HIGH U.N.O.)
PROVIDE 42" HIGH GUARDS (1013) AT DECKS,
LANDINGS, BALCONIES AND WALKWAYS
WHERE THERE A VERTICAL DROP OF >30".
TOP RAIL SHALL SUPPORT 50 LBS. PER
LIN. FT. OPEN RAIL SHALL HAVE INTERMEDIATE
RAILS, SO THAT A4" DIA. SPHERE CANNOT
PASS THRU, PER SECT. 1013 C.B.C.
SEE 7 & 8 / A-7.0, 9 & 10 / A-7.3

ROOF FINISH MANUF: MODEL: GACO WESTERN GACOFLEX URETHAN WATERPROOF MEMB WHITE (COOL ROOF) RR 25417 WATERPROOF
COLOR: WHITE (COOL
LARR#: RR 25417
ICC-ES#: ESR 1284
SLOPE: 1/4"FT. MIN.
CLASS: A
INSTALL PER MANUF SPECS
OR APPROVED EQUAL
PER SECT. 1507, C.B.C.

- FLOOR FRAMING SEE STRUCTURAL DRAWINGS
- ROOF / CEILING FRAMING SEE STRUCTURAL DRAWINGS
- OUTLET DRAIN @ STREET THROUGH APPROVED NON-EROSIVE DEVICE, SEE CIVIL DRWGS. INSTALL PER MANUF SPECS DRAFT STOPS & FIRE BLOCKING PROVIDE DRAFT STOPS AND OR FIRE BLOCKING IN ALL SEPARATION WALLS, FLOORCEILING ASSEMBLIES AND COMMON ATTIC SPACE (IF APPLICABLE) SO THAT NO ENCLOSED SPACE SHALL STOREFRONT WINDOW SYSTEM FRONT GLAZED STOREFRONT WINDOW SYSTEM. SEE WINDOW SCHEDULE MULLION STYLE: MULLION FINISH: WINDOW COLOR: MEASURE MORE THAN 10'-0' IN ANY DIRECTION PER SECT 717, C.B.C.

STAIRWAY WIDTH
THE WIDTH OF STAIRWAYS SHALL BE DETERMINED
AS SPECIFIED IN SEC. 1005.1 GC, BUT SUCH WIDTH
AS SPECIFIED IN SEC. 1005.1 GC, BUT SUCH WIDTH
SHALL NOT BE LESS THAN 4". SEE SEC. 1007.3 GBC
FOR ACCESSIBLE MEANS OF EGGRESS STAIRWAYS.
PER 1009.1 GBC
CLEAR SPACE BETWEEN A HANDRAIL AND A WALL OR
OTHER SURFACE SHALL BE A MINIMUM OF 1 1/2"
PER 1012.7 GBC
PROLECTIONS INTO THE PEOLUPED WIDTH OF STAIR
WAYS AT EACH HANDRAIL SHALL NOT EXCEED 4 1/2"
AT OR BELOW THE HANDRAIL HEIGHT. PROJECTIONS
INTO THE REQUIRED WIDTH SHALL NOT BE LIMITED
ABOVE THE MINIMUM HEADROOM HEIGHT REQUIRED
IN SEC. 1009.2 CBC
PER 1012.8 CBC

RISER REIGHT AND THREAD DEPTH
STAIR RISER HEIGHTS SHALL BE 7" MAX. AND 4" MIN.
HE RISER HEIGHT SHALL BE MEASURED VERTICALLY
BETWEEN THE LEADING EDGES OF ADJACENT TREADS.
RECTANGULAR TREAD DEPTHS SHALL BE 1" MIN.
MEASURED HORIZONIALLY BETWEEN THE VERTICAL
PLANES OF THE FOREMOST PROJECTION OF ADJACENT
TREADS AND AT A RIGHT ANGLE TO THE TREADS
LEADING EDGE. WINDER TREADS SHALL HAVE A MIN.
TREAD DEPTH OF 11" MEASURED BETWEEN THE
VERTICAL PLANES OF THE FOREMOST PROJECTION
OF ADJACENT TREADS AT THE INTERSECTIONS WITH
THE WALKLINE AND AMN. TREAD DEPTH OF 10" WITHIN
THE WALKLINE AND AMN. TREAD DEPTH OF 10" WITHIN OF ADJACENT TREADS AT THE INTERSECTIONS WITH THE WALKING AND AMN TREAD DEPTH OF 10 WITHIN THE CLEAR WIDTH OF THE STAIR. PER 1008.42 CBC
STAIR TREADS AND RISERS SHALL BE OF UNIFORM SIZE AND SHAPE. THE TOLE READ SHALL BE OF UNIFORM SIZE AND SHAPE. THE TOLE READ SHALL BE TO SET WITHIN THE LARGEST AND SMALLEST RISER HEIGHT OR BETWEEN THE LARGEST AND SMALLEST RISER HEIGHT OR BETWEEN THE LARGEST AND SMALLEST RICEAD DEPTH AT SHE WALKINE WITHIN ANY FUIGHT OF STAIRS SHALL NOT EXCEED THE SMALLEST BY MORE THAN 28°.

THE SMALLEST BY MORE THAN 28°.

PER 1008.4.4. CBC STAIR LANDINGS
THERE SHALL BE A FLOOR OR LANDING AT THE TOP
AND BOTTOM OF EACH STAIRWAY. THE WIDTH OF
LANDINGS SHALL NOT BE LESS THAN THE WIDTH OF LANDINGS SHALL NOT BE LESS THAN THE WIDTH OF STARRWAYS THEY SERVE, EVERY LANDING SHALL HAVE A MINIMUM DIMENSION MEASURED IN THE DIRECTION OF TRAVEL EQUAL TO THE WIDTH OF STARRWAY, SOLD DIMENSION SHALL NOT EXCEED 46 "WHERE THE STAR WAY HAS A STRAIGHT RUN, DOORS OPENING TO A LANDING SHALL HOT REDUCE THE THE LANDING TO MORE THAN ONE HALF OF THE REQUIRED WIDTH, PER 1009.5 CBC.

A FLIGHT OF STAIRS SHALL NOT HAVE A VERTICAL RISE GREATER THAN 20 FT BETWEEN FLOOR LEVELS OR LANDINGS.

HEADROOM @ STAIR PROVIDE 5'-8" MIN. CLEAR HEADROOM ABOVE FINISHED TREAD NOSING

FIRE PROTECTION @ STAIR
FIRE PROTECTION REQUIRED AT CONCEALED SPACE
BELOW STAIR PROVIDE 1-HOUR FIRE RESISTIVE WALLS
AND FLOOR / CEILING CONSTRUCTION.
PER SECTION 1000 CBC.
SEE DETAILS 1 / A-7.1 AND 5 / A-7.1

Chalk Preschool
1114 22nd Street, Manhattan Beach, 90266 Building **Sections**

Revisions:

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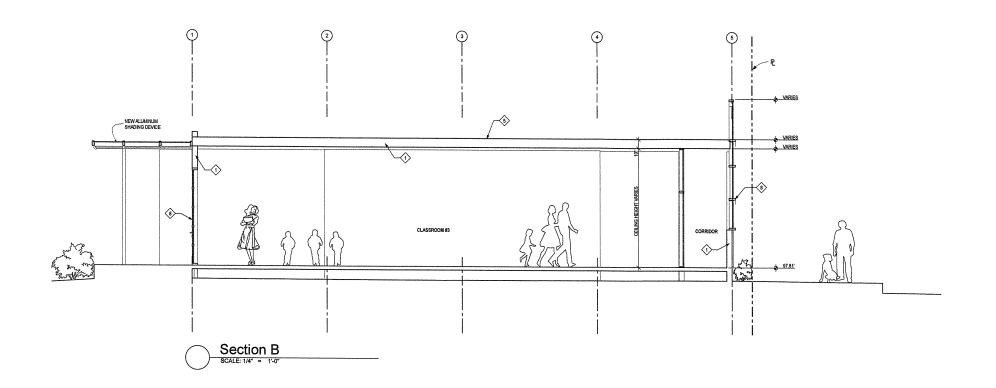
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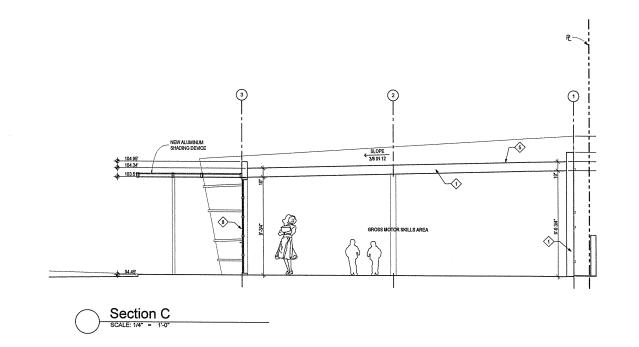
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A-4.0



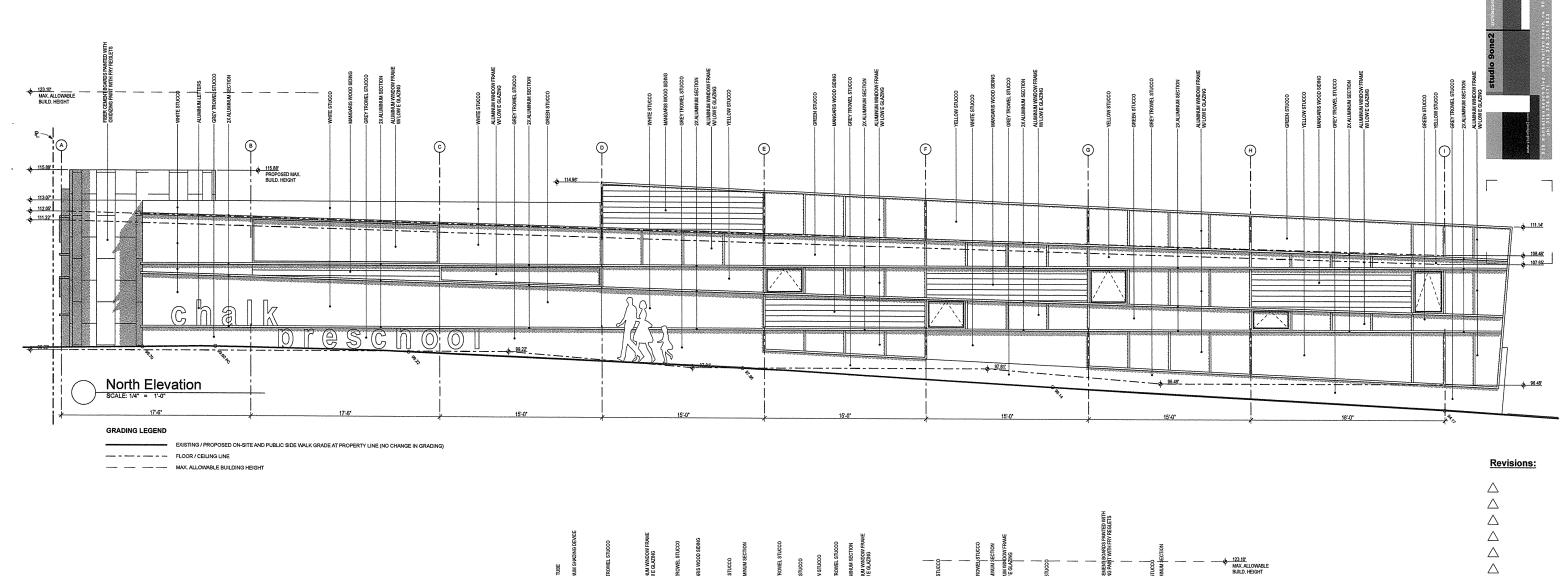


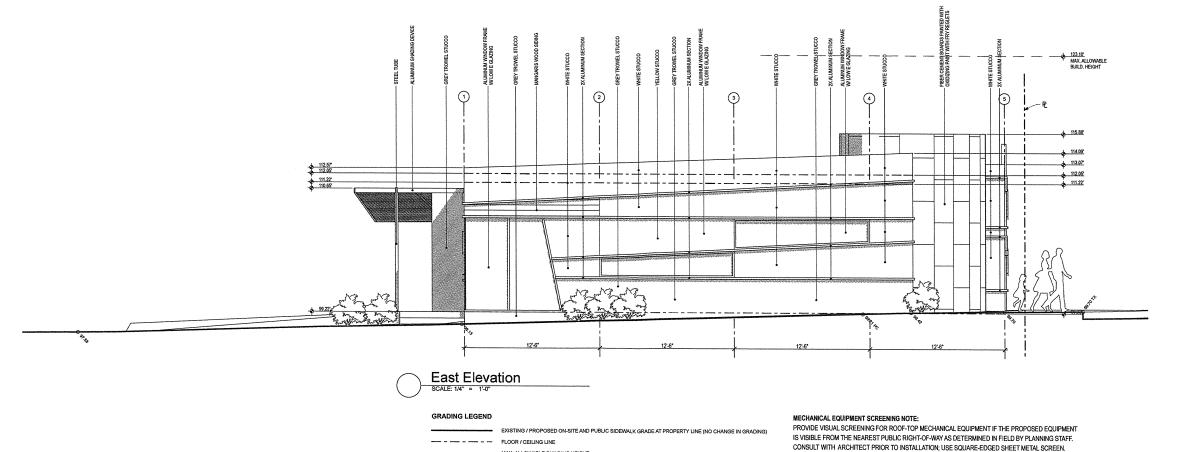


Revisions:

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Building Submittal
Sections
Sections



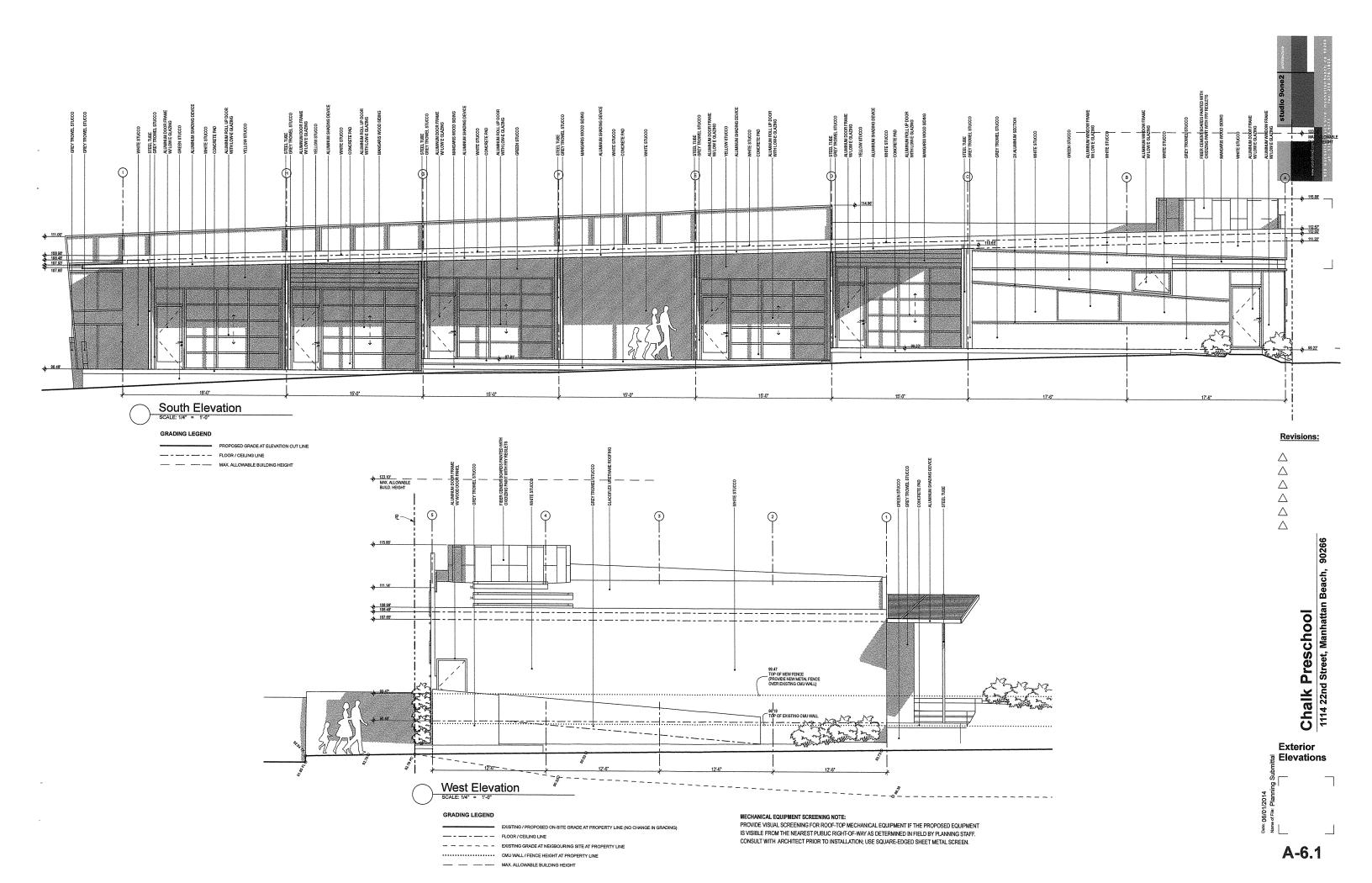


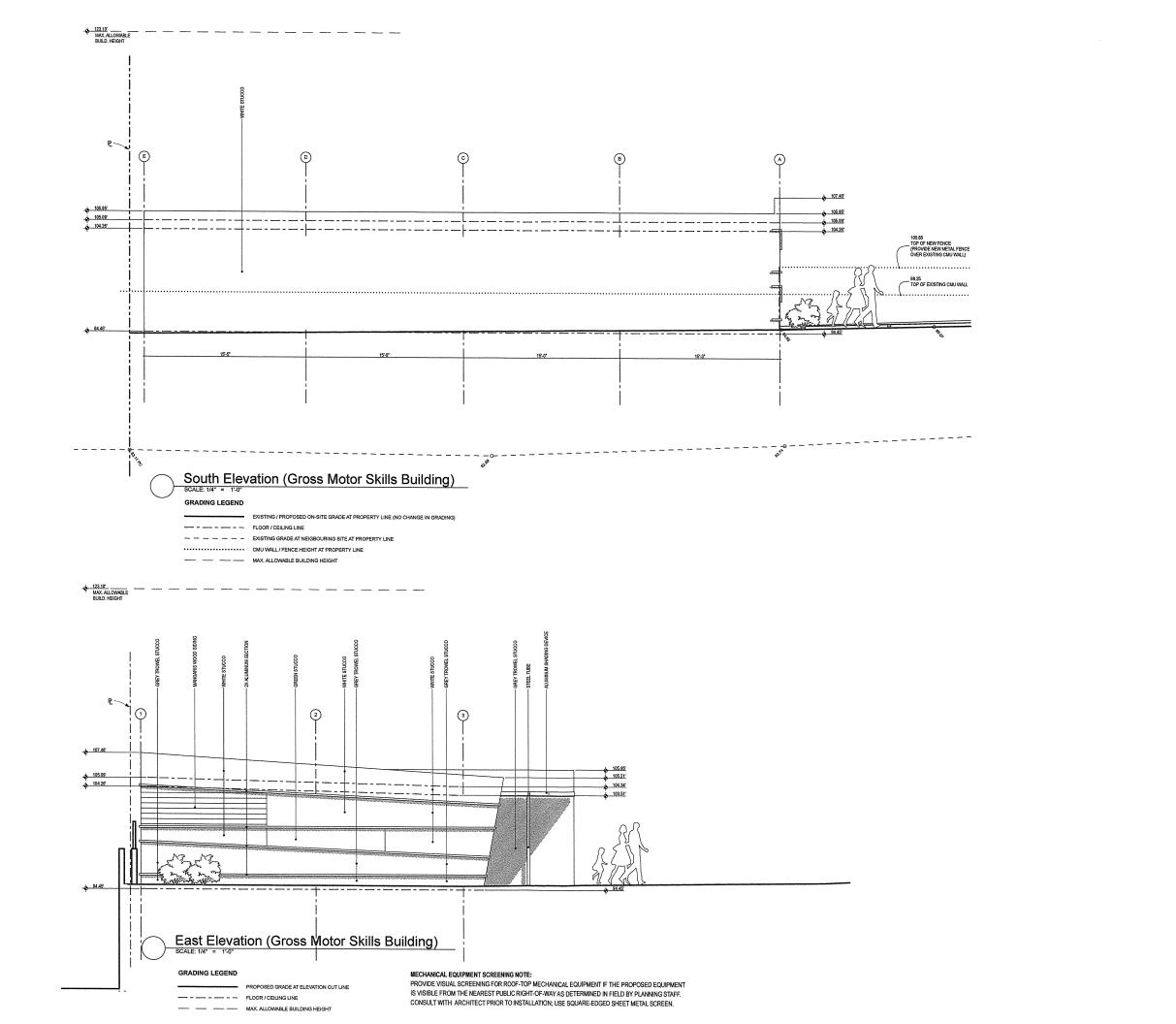
---- MAX. ALLOWABLE BUILDING HEIGHT

A-6.0

Exterior **፱** Elevations

Chalk Preschool
1114 22nd Street, Manhattan Beach, 90266





A-6.2

Exterior g Elevations

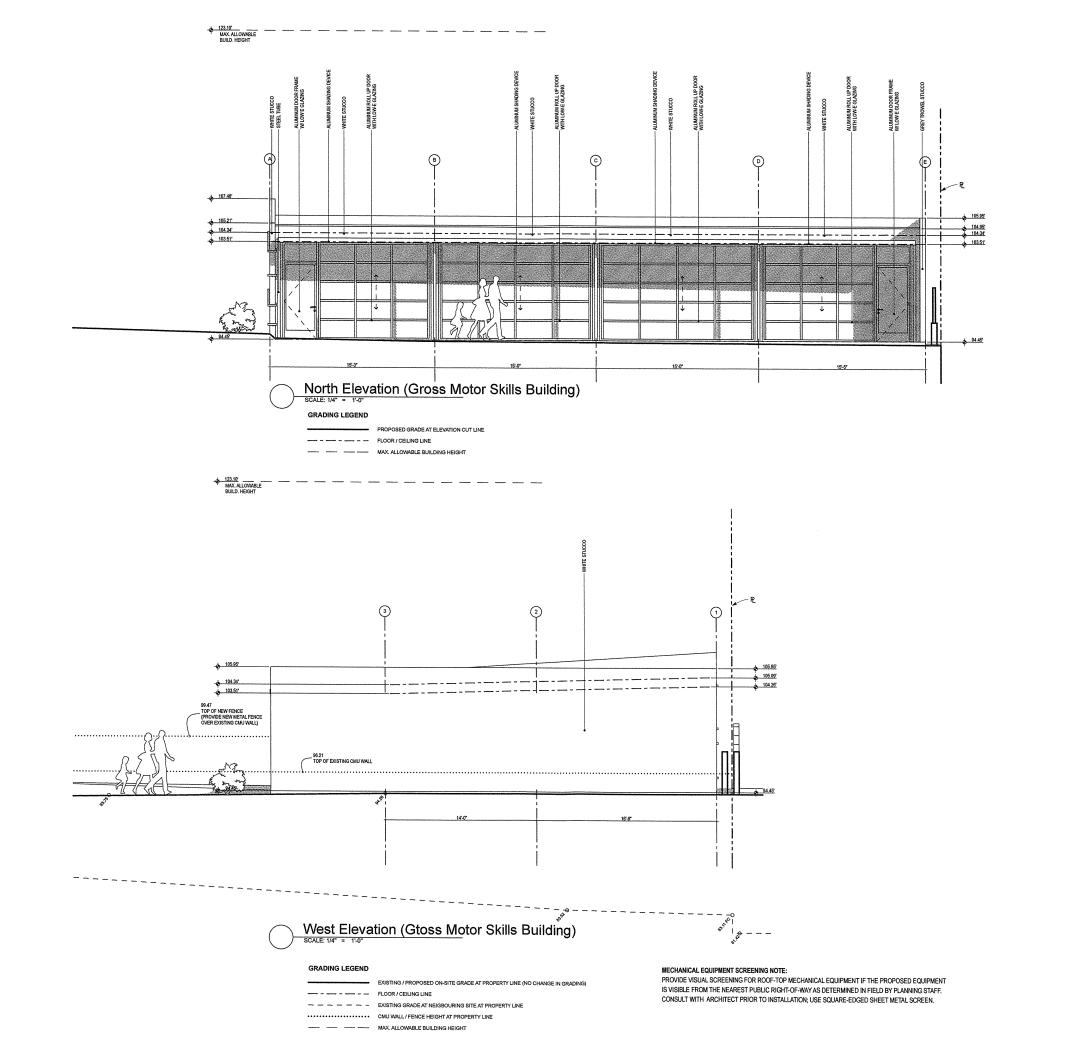
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Revisions:

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A-6.3

Exterior Exteriors

Revisions:

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